



**NORTHEAST
GEOTECHNICAL, INC.**
Delivering Practical Engineering Solutions

March 14, 2025

Project No. O542.00

Mr. Arthur Redding
New England Appliance Group (NEAG)
126 Grove Street
Franklin, MA 02038

SUBJECT: In-Situ Soil Permeability Testing Summary Report
Stormwater Management Basins for NEAG
126 Grove Street
Franklin, MA

Dear Arthur:

Northeast Geotechnical, Inc. has prepared this report summarizing the results of our in-situ soil permeability testing performed in support of the proposed stormwater management basins pursuant to our proposal to you dated January 31, 2025. Our objective has been to perform soil evaluation and in-situ permeability testing in the two stormwater management basins at the site. This report is subject to the attached Limitations and Service Constraints attached as Appendix A.

BACKGROUND

Our understanding of the existing site conditions and proposed project is based on our recent correspondences, our time on-site while logging the test pits and performing infiltration testing, and our review of the following referenced project plan. The “Drainage Plan” prepared by Guerriere & Halnon, Inc. (G&H), sheet 11 of 17, dated November 5, 2024 shows two existing stormwater management basins numbers one and two. The plan was annotated by G&H showing locations of proposed test pits and in-situ permeability testing locations.

Stormwater management basin number one is located on the western end of the site between the front of the building and Grove Street to the west. The basin is located to the north of the southerly entrance drive into the site. The bottom of the basin is shown on the referenced plan at about Elevation (El) 274 feet. Based on our visual observations while on-site, the basin appears to actually extend about a foot or two below that referenced elevation.

Stormwater management basin number two is located in the southeastern portion of the site at the toe of a slope leading from paved truck dock areas. This basin is also located to the south of the building where truck shipping and receiving activities take place. The bottom of the basin is shown on the referenced plan at about El 252 feet. The basin is connected to a bordering vegetated wetland located to the west. An earthen berm surrounds the basin and separates it from an adjoining wetlands.

SUBSURFACE EXPLORATORY TEST PITS

Northeast Geotechnical coordinated, observed, and logged a subsurface exploration program consisting of five exploratory test pits (TP-101 through TP-105) within the two stormwater management basins on March 4, 2025. Test pits TP-101 through TP-103 were performed in stormwater management basin number one. Test pits TP-104 and TP-105 were performed in stormwater management basin number two. In addition, two test pits (TP-11 and TP-12) were previously conducted in stormwater management basin two on July 26, 2024 as part of our previous studies. We have included the results of TP-11 and TP-12 in this report.

The test pit locations were survey located at the site by G&H in advance of our field work. The approximate test pit locations are shown on the Subsurface Exploration Location Plan, which is attached to this report as Figure No. 1. The existing ground surface elevations depicted on the individual test pit logs were estimated from G&H's Drainage Plan. The elevations shown on the test pit logs and referenced throughout this report should be considered approximate.

Fossile Construction, Inc. of Hudson, Massachusetts excavated test pits TP-101 through TP-105 with a Caterpillar 308E2 CR excavator. The excavator is equipped with a $0.5\pm$ cubic yard capacity toothed bucket and has a $14\pm$ foot maximum reach. Canesi Brothers Construction Inc. excavated test pits TP-11 and TP-12 with a Case CX145C SR excavator. This excavator is equipped with a $1.5\pm$ cubic yard bucket and has an $18\pm$ foot reach.

The test pits were generally advanced through topsoil fill, existing granular fill and into underlying apparent natural granular soils. The apparent natural granular soils generally consisted of sandy silt, silty sand, silt and fine sand, sand, sand with gravel and/or sand and gravel. The soils exposed in the test pits were visually described in the field by a Northeast Geotechnical, Inc. licensed Massachusetts Soil Evaluator using the USDA soil textural classification system.

The depths to seasonal high groundwater in the test pits were estimated based on the presence and absence of redoximorphic features and/or groundwater to the depths explored in the test pits. The visual descriptions, estimates of seasonal high groundwater, approximate changes in soil strata, and other observations are shown on the test pit logs contained in Appendix B. Photographs of the test pits are included in the test pit photo log contained in Appendix C.

CONSTANT HEAD IN-SITU PERMEABILITY TESTING

A Northeast Geotechnical, Inc. licensed Massachusetts Soil Evaluator performed in-situ constant head permeability tests within the two proposed stormwater management areas. The testing was performed in four of the five test pits excavated on March 4, 2025. The purpose of the testing was to evaluate field saturated hydraulic conductivity rates.

The tests were performed using a Guelph Permeameter in accordance with the Massachusetts Department of Environmental Protection (MassDEP) Stormwater Handbook and ASTM D5126. The Guelph permeameter is generally capable of testing in soils with hydraulic conductivities ranging from 10^{-2} to 10^{-5} cm/sec (0.01 to 14.2 in/hour). One test indicated the hydraulic conductivity of the soil at the test location and depth is outside of these referenced ranges. The test result is still presented however the designer should consider these applicable ranges.

Results of the permeability testing and USDA soil textural classifications at the testing depths for the March 2025 test pits are presented in the following table:

Test Pit I.D.	Permeability Test Depth (feet) / Elevation (feet)	Soil Strata / USDA Soil Textural Classification	Soil Saturated Hydraulic Conductivity, K_{fs} cm/sec (in/hr) ¹
TP-102	1.2± (El 273.8±)	Natural Sandy Silt (Loam)	No measurable infiltration after 30 minutes
TP-103	2.3± (El 271.7±)	Natural Sand with Gravel (Sand)	1.5×10^{-3} (2.1 in/hr)
TP-104	2.3± (El 249.7±)	Natural Sandy Silt (Silt Loam)	1.3×10^{-3} (1.8 in/hr)
TP-105	2± (El 250±)	Natural Sand with Gravel (Sand)	2.1×10^{-2} (30.3 in/hr)

1. A factor of safety has not been applied to these reported soil saturated hydraulic conductivities.

LABORATORY TESTING

Northeast Geotechnical submitted selected representative soil samples collected from the test pits to Thielsch Engineering of Cranston, Rhode Island for laboratory testing to assess basic geotechnical engineering characteristics of the soils. The laboratory testing consisted of four combined sieve and hydrometer tests with USDA soil textural classifications performed on samples of the natural granular soils collected from test pits TP-102 through TP-105. Additionally, a combined sieve and hydrometer test with USDA soil textural classification was performed on a sample of the natural granular soil collected from test pit TP-11 as part of our previous studies. The laboratory test results are attached to this report in Appendix D.

GENERAL SUBSURFACE CONDITIONS

The general subsurface conditions within the two stormwater management basins were assessed based upon the results of the subsurface exploration program, laboratory test results and published USGS surficial geology information. The following subsections describe the encountered subsurface conditions in each of the two basins.

4.1 Stormwater Management Basin One

The following subsections describe the subsurface conditions encountered in test pits TP-101 through TP-103 performed in stormwater management basin one.

4.1.1 Topsoil Fill

Topsoil fill was encountered at the ground surface of the three test pits and was observed to be approximately 6± to 10± inches thick. The general USDA soil textural description of the topsoil fill is loam.

4.1.2 Existing Fill

Existing fill was observed beneath the topsoil fill in the test pits, extending to depths of approximately 14± to 43± inches below the ground surface. The general USDA soil textural description of the existing fill is loamy sand as well as sandy loam.

4.1.3 Natural Sandy Silt and Silty Sand

Natural sandy silt was observed overlying in turn, natural silty sand and then natural sand and gravel in test pit TP-101. The natural sandy silt was encountered below the existing fill at a depth of approximately 43± inches below ground surface. Natural silty sand was encountered below the sandy silt and was observed to extend to approximately 66± inches below ground surface. Natural sandy silt was also observed in test pit TP-102 underlying the existing fill at a depth of approximately 14± inches and extended to a depth of approximately 50± inches below the ground surface.

The general USDA soil textural description of the sandy silt is loam. The general USDA soil textural description of the silty sand is loamy sand.

4.1.4 Natural Sand and Gravel

Natural sand and gravel was encountered in each of the test pits performed in stormwater management basin number one. Each of the test pits performed in this basin terminated in the natural sand and gravel. In test pits TP-101 and TP-102 the natural sand and gravel was encountered below the natural silty sand layer and below the natural sandy silt layer respectively. The depths to the surface of the natural sand and gravel layer were 66± and 50± inches below ground surface in test pits TP-101 and TP-102 respectively.

Natural sand and gravel was encountered below the existing fill layer in test pit TP-103. No silty sand or sandy silt was observed in test pit TP-103. The natural sand and gravel was observed at a depth of approximately 23± inches below ground surface and extended to the depth explored which was 60± inches below ground surface. The general USDA soil textural descriptions of the natural sand and gravel is sand.

4.1.5 Groundwater

Weeping and standing groundwater was encountered in each of the three test pits performed in stormwater management basin one as shown on the test pit logs. The depths of estimated seasonal high groundwater in the test pits were based on the presence of redoximorphic features and observed groundwater conditions. Signs indicative of seasonal high groundwater were observed in select test pits at depths ranging from approximately 14± to 43± inches below the existing ground surface, corresponding to elevations ranging from approximately 271.4± to 273.8± feet. Refer to the individual test pit logs in Appendix B for additional information.

4.2 Stormwater Management Basin Two

The following subsections describe the subsurface conditions encountered in test pits TP-11, TP-12, TP-104 and TP-105 performed in stormwater management basin two.

4.2.1 Topsoil Fill

Topsoil fill was encountered at the ground surface of the four test pits and was observed to be approximately 4± to 20± inches thick. The general USDA soil textural description of the topsoil fill is sandy loam.

4.2.2 Subsoil

Natural subsoil was encountered below the topsoil fill in test pits TP-11 and TP-12 at depths of approximately 4± and 9± inches below ground surface respectively. This subsoil was observed to extend to depths of approximately 15± and 27± below ground surface in TP-11 and TP-12 respectively. The general USDA soil textural description of the subsoil is loamy sand.

4.2.3 Natural Sandy Silt

Natural sandy silt was encountered below the topsoil fill in test pit TP-104 at a depth of approximately 17± inches below ground surface. This deposit was observed extending approximately 86± inches below ground surface. The general USDA soil textural description of sandy silt is silt loam.

4.2.4 Natural Sand

Natural sand was encountered below the natural sandy silt in test pit TP-104 at a depth of approximately 86± inches below ground surface. This deposit was observed extending to the depth explored which was approximately 119± inches below ground surface. Natural sand was also encountered below the subsoil in TP-11 and TP-12, extending to depths of approximately 48± and 83± inches respectively. The general USDA soil textural description of the natural sand is sand.

4.2.5 Natural Silty Sand

Natural silty sand was encountered below the natural sand in test pit TP-11 at a depth of approximately 48± to 57± inches below ground surface. The general USDA soil textural description of the natural silty sand is loamy sand.

4.2.6 Natural Fine Sand and Silt

Natural fine sand and silt was encountered below the natural sand in test pit TP-12 at a depth of approximately 83± inches below ground surface. This deposit was observed extending to the depth explored which was approximately 136± inches below ground surface. The general USDA soil textural description of the natural fine sand and silt is sandy loam.

4.2.7 Natural Sand with Gravel

Natural sand with gravel was encountered below the natural silty sand in test pit TP-11 at a depth of approximately 57± inches below the ground surface, and below the topsoil fill in test pit TP-105 at a depth of approximately 20± inches below ground surface. This deposit was observed extending to the depths explored which was approximately 109± to 110± inches below ground surface. The general USDA soil textural description of the natural sand with gravel is sand.

4.2.8 Groundwater

Groundwater was not encountered to the depths explored in the three of the four test pits performed in stormwater management basin number two. Estimated seasonal high groundwater levels are therefore below the depths explored in test pits TP-12, TP-104 and TP-105. The depths of these test pits range from approximately 110± to 136± inches below ground surface. These depths correspond to elevations ranging from 240.7± to 242.8± feet.

Weeping and standing groundwater was encountered in test pit TP-11 performed in stormwater management basin two as shown on the test pit log. The depth of estimated seasonal high groundwater in the test pit was based on the presence of redoximorphic features at a depth of approximately 53± inches below ground surface. It is our opinion that the redoximorphic features may represent a periodic, temporary perched condition. The depth of the redoximorphic features corresponds to approximate elevation 247.7± feet. Refer to the individual test pit logs in Appendix B for additional information.

Thank you very much for the opportunity to assist you with this project. If you have any questions regarding this report or if we can be of further assistance, please call either Glenn Olson, P.E. at 508-274-0887 or via email at golson@northeastgeotechnical.com or call Christian Rice, P.E. at 508-847-1348 or via email at crice@northeastgeotechnical.com.

Sincerely,

NORTHEAST GEOTECHNICAL, INC.



Glenn A. Olson, P.E.
Principal Engineer

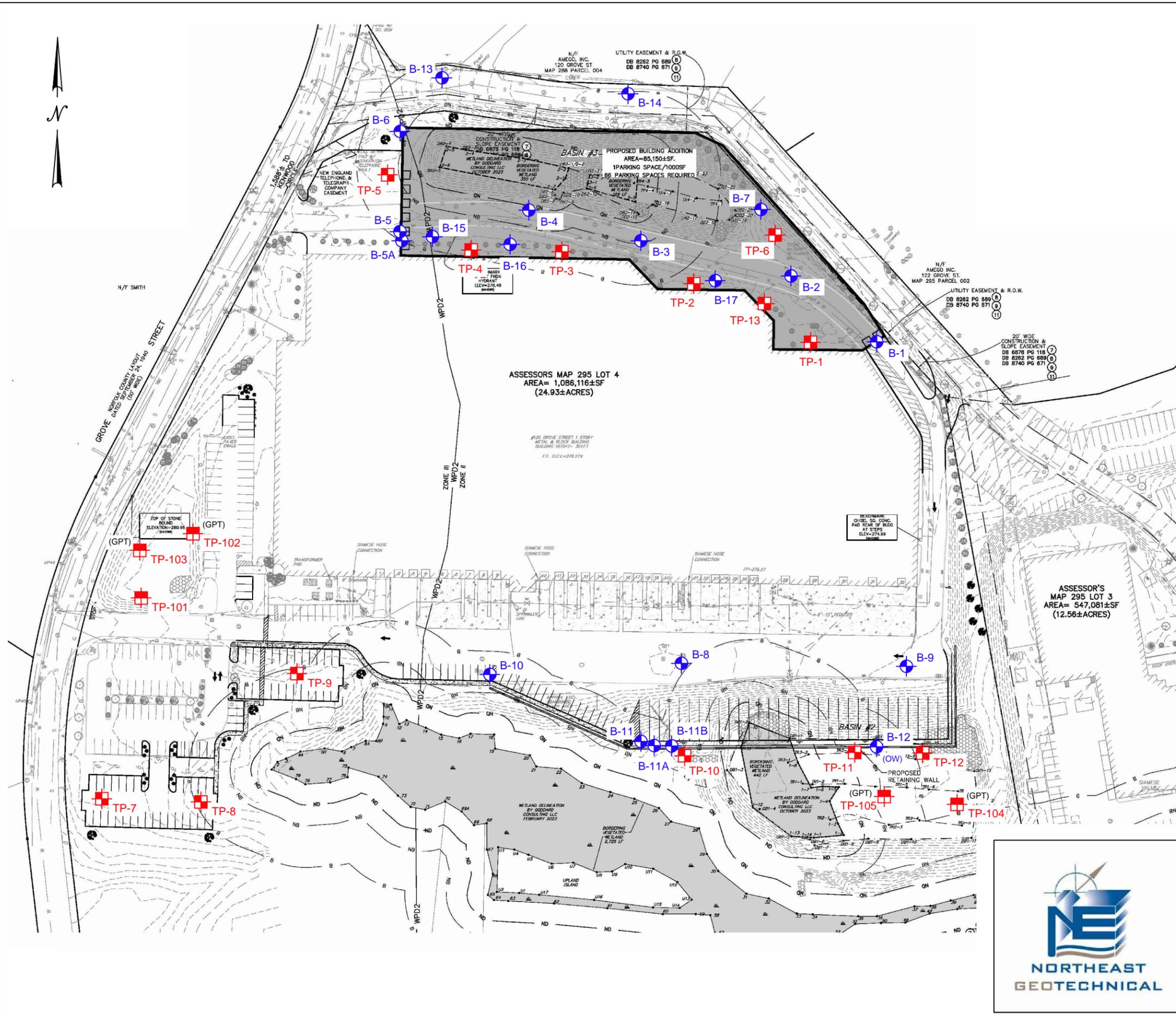


Christian B. Rice, P.E.
Project Manager

Copy: Michael Hassett, Guerriere & Halnon, Inc.

Attachments: Figure 1 – Subsurface Exploration Location Plan
Appendix A – Limitations and Service Constraints
Appendix B – Test Pit Logs
Appendix C – Test Pit Photo Log
Appendix D – Soil Laboratory Test Results

FIGURE



- NOTES:
1. BASE MAP DEVELOPED FROM PLAN TITLED "EXHIBIT PLAN", SHEET No. 1 OF 1, DATED AUGUST 19, 2024, BY GUERRIERE & HALNON, INC. (G&H).
 2. TEST PIT TP-13 AND TEST BORING B-5A, B-11A, B-11B, B-15, B-16, AND B-17 LOCATIONS ESTABLISHED AT THE SITE BY NORTHEAST GEOTECHNICAL, INC. PERSONNEL USING TAPED MEASUREMENTS AND LINE OF SIGHT FROM EXISTING SITE AND BUILDING FEATURES. REMAINING TEST PIT AND TEST BORING LOCATIONS SURVEY LOCATED AT THE SITE BY G&H. TEST BORING B-11 DRILLED APPROXIMATELY 4+/- FEET EAST OF SURVEY-LOCATED STAKE DUE TO EXISTING TREE IN VICINITY OF STAKE. THE TEST PIT AND TEST BORING LOCATIONS SHOWN ON THIS PLAN SHOULD BE CONSIDERED ACCURATE TO THE DEGREE IMPLIED BY THE METHODS USED.
 3. TEST BORINGS AND TEST PITS OBSERVED AND LOGGED BY NORTHEAST GEOTECHNICAL, INC. PERSONNEL.

- LEGEND:
- TEST BORINGS PERFORMED BY SOIL X CORP. OF LEOMINSTER, MA ON AUGUST 5 THROUGH 9, 2024.
 - INDICATES OBSERVATION WELL INSTALLED.
 - TEST PITS EXCAVATED BY CANESI BROS. CONSTRUCTION, INC. OF FRANKLIN, MA ON JULY 25 AND 26, 2024.
 - TEST PITS EXCAVATED BY FOSSILE CONSTRUCTION OF HUDSON, MA ON MARCH 4, 2025.
 - IN-SITU PERMEABILITY TEST PERFORMED WITH THE GUELPH PERMEAMETER IN TEST PIT.



NORTHEAST GEOTECHNICAL, INC.		
NEW ENGLAND APPLIANCE GROUP WAREHOUSE ADDITION		
126 GROVE STREET		FRANKLIN, MA
SUBSURFACE EXPLORATION LOCATION PLAN		
Project No.: O542.00	Drawn By: CBR	Reviewed By: G. OLSON, P.E.
Date: 3/5/2025	Scale: 1"=120'	Figure No.: 1

APPENDIX A

Limitations and Service Constraints

LIMITATIONS AND SERVICE CONSTRAINTS

Geotechnical Engineering Consulting Services

The opinions, conclusions and recommendations presented in this report are based upon the scope of services, information obtained through the performance of the services, and the schedule as agreed upon by Northeast Geotechnical, Inc. and the party for whom this report was originally prepared. This report is an instrument of professional service and was prepared in accordance with the generally accepted standards and level of skill and care under similar conditions and circumstances established by the geotechnical consulting industry. No representation, warranty, or guarantee, express or implied, is intended or given. To the extent that Northeast Geotechnical, Inc. relied upon any information prepared by other parties not under contract to Northeast Geotechnical, Inc., Northeast Geotechnical, Inc. makes no representation as to the accuracy or completeness of such information. This report is expressly for the sole and exclusive use of the party for whom this report was originally prepared and/or other specifically named parties have the right to make use of and rely upon this report. Reuse of this report or any portion thereof for other than its intended purpose, or if modified, or if used by third parties, shall be at the user's sole risk.

Furthermore, nothing contained in this document shall relieve any other party of its responsibility to abide by contract documents and applicable laws, codes, regulations, or standards.

Subsurface Explorations and Testing

Results of any observations, subsurface exploration or testing, and any findings presented in this report apply solely to conditions existing at the time when Northeast Geotechnical, Inc.'s exploratory work was performed. It must be recognized that any such observations and exploratory or testing activities are inherently limited and do not represent a conclusive or complete characterization. Conditions in other parts of the project site may vary from those at the locations where data were collected and conditions can change with time. Northeast Geotechnical, Inc.'s ability to interpret exploratory and test results is related to the availability of the data and the extent of the exploratory and testing activities.

The findings, conclusions and recommendations submitted in this report are based, in part, on data obtained from subsurface borings, test pits, and specific, discrete sampling locations. The nature and extent of variation between these test locations, which may be widely spaced, may not become evident until construction. If variations are subsequently encountered, it will be necessary to re-evaluate the conclusions and recommendations of this report.

Correlations and descriptions of subsurface conditions presented in boring logs, test pit logs, subsurface profiles, and other materials are approximate only. Subsurface conditions may vary significantly from those encountered in borings and sampling locations and transitions between subsurface materials may be gradual or highly variable.

Conditions at the time water level measurements and other subsurface observations were made are presented in the boring logs or other sampling forms. These field data have been reviewed and interpretations provided in this report. However, groundwater levels may be variable and may fluctuate due to variation in precipitation, temperature, and other factors. Therefore, groundwater levels at the site at any time may be different than stated in this report.

Review

In the event that any change in the nature, design, or location of the proposed structure(s) is planned, the conclusions and recommendations in this report shall not be considered valid unless the changes are reviewed and the conclusions and recommendations of this report are modified or verified in writing.

Northeast Geotechnical, Inc. should be provided the opportunity for a general review of final design plans and specifications to assess that our recommendations have been properly interpreted and included in the design and construction documents.

Construction

To verify conditions presented in this report and modify recommendations based on field conditions encountered in the field, Northeast Geotechnical, Inc. should be retained to provide geotechnical engineering services during the construction phase of the project. This is to observe compliance with design concepts, specifications, and recommendations contained in this report, and to verify and refine our recommendations as necessary in the event that subsurface conditions differ from those anticipated prior to the start of construction.

APPENDIX B

Test Pit Logs

NORTHEAST GEOTECHNICAL, INC.

TEST PIT LOG

Project: New England Appliance Group
Warehouse Addition
126 Grove Street
Franklin, MA

Test Pit/Deep Observation Hole Number: TP-11

Subcontractor: Canesi Bros Construction Inc.
 Operator: Andrew
 Equipment: Case CX145C SR Excavator
 Capacity/Reach: 1.5± yd³ toothed bucket / 18± ft

Date/Weather: 7-26-2024 / Clear, 60s to 80s °F
 Northeast Geotechnical Observer: Christian Rice, P.E.
 Test Pit Location: See Subsurface Exploration Location Plan
 Ground Surface Elevation: 252.1± feet

Page: 1 of 1
 File No. O542.00
 Reviewed By: Glenn Olson, P.E.

Depth (in.)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments		Soil Structure	Soil Consistence (Moist)	Other
			Depth (in.)	Color	Percent		Gravel	Cobbles & Stones			
0 - 4±	Topsoil Fill (A _p)	10YR/3/3	---	---	---	Sandy Loam	0%	0%	Granular	Very Friable	Frequent roots
4 - 15±	Subsoil (B _w)	10YR/4/4	---	---	---	Loamy Sand	15±%	0%	Granular	Firm	Trace roots
15 - 48±	Natural Sand (C1)	2.5Y/6/3	---	---	---	Sand	9±%	5±%	Single Grain	Friable	
48 - 57±	Natural Silty Sand (C2)	10YR/5/1	53±	7.5YR/5/6	5±	Loamy Sand	0%	0%	Granular	Firm	
57 - 109±	Natural Sand with Gravel (C3)	2.5Y/6/3	---	---	---	Sand	30±%	10±%	Single Grain	Firm	

Groundwater Observed: Yes Depth Weeping from Pit: 104± inches Depth Standing Water in Hole: 104± inches

Estimated Depth (Elevation) to Seasonal High Groundwater: 53± inches (EI 247.7± feet)

Notes:

- 1) Test pit dimensions: 5± feet (north/south) x 12± feet (east/west).
- 2) Grab sample GS-1 collected from approximately 24± to 48± inches (2± to 4± feet) below ground surface.
- 3) Test pit terminated at 109± inches (9.1± feet) below ground surface.

NORTHEAST GEOTECHNICAL, INC.

TEST PIT LOG

Project: New England Appliance Group
Warehouse Addition
126 Grove Street
Franklin, MA

Test Pit/Deep Observation Hole Number: TP-12

Subcontractor: Canesi Bros Construction Inc.
 Operator: Andrew
 Equipment: Case CX145C SR Excavator
 Capacity/Reach: 1.5± yd³ toothed bucket / 18± ft

Date/Weather: 7-26-2024 / Clear, 60s to 80s °F
 Northeast Geotechnical Observer: Christian Rice, P.E.
 Test Pit Location: See Subsurface Exploration Location Plan
 Ground Surface Elevation: 252.0± feet

Page: 1 of 1
 File No. O542.00
 Reviewed By: Glenn Olson, P.E.

Depth (in.)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments		Soil Structure	Soil Consistence (Moist)	Other
			Depth (in.)	Color	Percent		Gravel	Cobbles & Stones			
0 - 9±	Topsoil Fill (A _p)	10YR/3/3	---	---	---	Sandy Loam	5±%	0%	Granular	Very Friable	Frequent roots
9 - 27±	Subsoil (B _w)	10YR/4/4	---	---	---	Loamy Sand	15±%	10±%	Granular	Firm	Trace roots
27 - 83±	Natural Sand (C1)	2.5Y/5/2	---	---	---	Sandy Loam	15±%	10±%	Sand	Friable	Occasional 24± inch boulders
83 - 136±	Natural Fine Sand and Silt (C2)	2.5Y/6/1	---	---	---	Sandy Loam	0%	0%	Granular	Firm	

Groundwater Observed: No Depth Weeping from Pit: N/A Depth Standing Water in Hole: N/A

Estimated Depth (Elevation) to Seasonal High Groundwater: >136± inches (<El 240.7± feet)

Notes:

- 1) Test pit dimensions: 4± feet (north/south) x 10± feet (east/west).
- 2) Grab sample GS-1 collected from approximately 30± to 60± inches (2.5± to 5± feet) below ground surface.
- 3) Soils observed to be more moist below approximately 102± inches (8.5± feet) below ground surface.
- 4) Test pit terminated upon soils caving in at approximately 136± inches (11.3± feet) below ground surface.

NORTHEAST GEOTECHNICAL, INC.

TEST PIT LOG

Project: New England Appliance Group
Warehouse Addition
126 Grove Street
Franklin, MA

Test Pit/Deep Observation Hole Number: TP-101

Subcontractor: Fossile Construction Date/Weather: 3-4-2025 / Clear to Overcast, 20s to 40s °F Page: 1 of 1
 Operator: Neil Fossile Northeast Geotechnical Observer: Christian Rice, P.E. File No. O542.00
 Equipment: Caterpillar 308E2 CR Excavator Test Pit Location: See Subsurface Exploration Location Plan Reviewed By: Glenn Olson, P.E.
 Capacity/Reach: 0.5± yd³ toothed bucket / 14± ft Ground Surface Elevation: 275± feet

Depth (in.)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments		Soil Structure	Soil Consistence (Moist)	Other
			Depth (in.)	Color	Percent		Gravel	Cobbles & Stones			
0 - 9±	Topsoil Fill (A _p)	10YR/2/1	---	---	---	Loam	0%	0%	Granular	Friable	Occasional roots
9 - 43±	Existing Fill	10YR/5/3	---	---	---	Loamy Sand	25±%	10±%	Granular	Firm	
43 - 50±	Natural Sandy Silt (C1)	GLEY1/6/N	43±	7.5YR/6/8	40±	Loam	0%	0%	Massive	Firm	
50 - 66±	Natural Silty Sand (C2)	2.5Y/5/2	50±	7.5YR/6/8	20±	Loamy Sand	10±%	0%	Granular	Firm	
66 - 80±	Natural Sand & Gravel (C3)	2.5Y/5/3	66±	7.5YR/6/8	20±	Sand	30±%	15±%	Single Grain	Friable	

Groundwater Observed: Yes Depth Weeping from Pit: 43± inches Depth Standing Water in Hole: 64± inches

Estimated Depth (Elevation) to Seasonal High Groundwater: 43± inches (EI 271.4± feet)

Notes:

- 1) Test pit dimensions: 3± feet (north/south) x 8± feet (east/west).
- 2) Grab sample GS-1 collected from approximately 43± to 50± inches (3.6± to 4.2± feet) below ground surface.
- 3) Test pit terminated upon soils caving in at approximately 80± inches (6.7± feet) below ground surface.

NORTHEAST GEOTECHNICAL, INC.

TEST PIT LOG

Project: New England Appliance Group
Warehouse Addition
126 Grove Street
Franklin, MA

Test Pit/Deep Observation Hole Number: TP-102

Subcontractor: Fossile Construction
 Operator: Neil Fossile
 Equipment: Caterpillar 308E2 CR Excavator
 Capacity/Reach: 0.5± yd³ toothed bucket / 14± ft

Date/Weather: 3-4-2025 / Clear to Overcast, 20s to 40s °F
 Northeast Geotechnical Observer: Christian Rice, P.E.
 Test Pit Location: See Subsurface Exploration Location Plan
 Ground Surface Elevation: 275± feet

Page: 1 of 1
 File No. O542.00
 Reviewed By: Glenn Olson, P.E.

Depth (in.)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments		Soil Structure	Soil Consistence (Moist)	Other
			Depth (in.)	Color	Percent		Gravel	Cobbles & Stones			
0 - 6±	Topsoil Fill (A _p)	10YR/2/1	---	---	---	Loam	0%	0%	Granular	Friable	Occasional roots
6 - 14±	Existing Fill	2.5Y/5/3	---	---	---	Sandy Loam	15±%	5±%	Granular	Firm	
14 - 50±	Natural Sandy Silt (C1)	GLE1/6/10Y	14±	10YR/5/8	30±	Loam	<5±%	0%	Massive	Very Firm	
50 - 63±	Natural Sand & Gravel (C2)	2.5Y/5/3	50±	10YR/5/8	20±	Sand	30±%	10±%	Single Grain	Firm	

Groundwater Observed: Yes Depth Weeping from Pit: 50± inches Depth Standing Water in Hole: 54± inches

Estimated Depth (Elevation) to Seasonal High Groundwater: 14± inches (El 273.8± feet)

Notes:

- 1) Test pit dimensions: 7.5± feet (north/south) x 4.5± feet (east/west).
- 2) Grab sample GS-1 collected from approximately 14± to 36± inches (1.2± to 3± feet) below ground surface.
- 3) Constant head permeability test performed at approximately 14± inches (1.2± feet) below ground surface.
- 4) Test pit terminated at 63± inches (5.3± feet) below ground surface.

NORTHEAST GEOTECHNICAL, INC.

TEST PIT LOG

Project: New England Appliance Group
Warehouse Addition
126 Grove Street
Franklin, MA

Test Pit/Deep Observation Hole Number: TP-103

Subcontractor: Fossile Construction
 Operator: Neil Fossile
 Equipment: Caterpillar 308E2 CR Excavator
 Capacity/Reach: 0.5± yd³ toothed bucket / 14± ft

Date/Weather: 3-4-2025 / Clear to Overcast, 20s to 40s °F
 Northeast Geotechnical Observer: Christian Rice, P.E.
 Test Pit Location: See Subsurface Exploration Location Plan
 Ground Surface Elevation: 274± feet

Page: 1 of 1
 File No. O542.00
 Reviewed By: Glenn Olson, P.E.

Depth (in.)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments		Soil Structure	Soil Consistence (Moist)	Other
			Depth (in.)	Color	Percent		Gravel	Cobbles & Stones			
0 - 10±	Topsoil Fill (A _p)	10YR/2/1	---	---	---	Loam	0%	0%	Granular	Friable	Frequent roots
10 - 23±	Existing Fill	10YR/5/3	---	---	---	Loamy Sand	15±%	10±%	Granular	Firm	Trace roots
23 - 60±	Natural Sand with Gravel (C)	10YR/6/4	23±	7.5YR/5/6	25±	Sand	20±%	15±%	Single Grain	Friable	

Groundwater Observed: Yes Depth Weeping from Pit: 50± inches Depth Standing Water in Hole: 56± inches

Estimated Depth (Elevation) to Seasonal High Groundwater: 23± inches (EI 272.1± feet)

Notes:

- 1) Test pit dimensions: 4± feet (north/south) x 7± feet (east/west).
- 2) Grab sample GS-1 collected from approximately 28± to 40± inches (2.3± to 3.3± feet) below ground surface.
- 3) Constant head permeability test performed at approximately 28± inches (2.3± feet) below ground surface.
- 4) Test pit terminated at 60± inches (5± feet) below ground surface.

NORTHEAST GEOTECHNICAL, INC.

TEST PIT LOG

Project: New England Appliance Group
Warehouse Addition
126 Grove Street
Franklin, MA

Test Pit/Deep Observation Hole Number: TP-104

Subcontractor: Fossile Construction
 Operator: Neil Fossile
 Equipment: Caterpillar 308E2 CR Excavator
 Capacity/Reach: 0.5± yd³ toothed bucket / 14± ft

Date/Weather: 3-4-2025 / Clear to Overcast, 20s to 40s °F
 Northeast Geotechnical Observer: Christian Rice, P.E.
 Test Pit Location: See Subsurface Exploration Location Plan
 Ground Surface Elevation: 252± feet

Page: 1 of 1
 File No. O542.00
 Reviewed By: Glenn Olson, P.E.

Depth (in.)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments		Soil Structure	Soil Consistence (Moist)	Other
			Depth (in.)	Color	Percent		Gravel	Cobbles & Stones			
0 - 17±	Topsoil Fill (A _p)	10YR/2/1	---	---	---	Sandy Loam	5±%	0%	Granular	Friable	Occasional roots, trace brick/glass
17 - 86±	Natural Sandy Silt (C1)	2.5Y/5/2	---	---	---	Silt Loam	0%	0%	Massive	Firm	
86 - 119±	Natural Sand (C2)	2.5Y/5/2	---	---	---	Sand	0%	0%	Single Grain	Friable	

Groundwater Observed: No Depth Weeping from Pit: N/A Depth Standing Water in Hole: N/A

Estimated Depth (Elevation) to Seasonal High Groundwater: >110± inches (<El 242.8± feet)

Notes:

- 1) Test pit dimensions: 6± feet (north/south) x 8.5± feet (east/west).
- 2) Grab sample GS-1 collected from approximately 27± to 42± inches (2.3± to 3.5± feet) below ground surface.
- 3) Constant head permeability test performed at approximately 27± inches (2.3± feet) below ground surface.
- 4) Test pit terminated at 119± inches (9.9± feet) below ground surface.

NORTHEAST GEOTECHNICAL, INC.

TEST PIT LOG

Project: New England Appliance Group
Warehouse Addition
126 Grove Street
Franklin, MA

Test Pit/Deep Observation Hole Number: TP-105

Subcontractor: Fossile Construction Date/Weather: 3-4-2025 / Clear to Overcast, 20s to 40s °F Page: 1 of 1
 Operator: Neil Fossile Northeast Geotechnical Observer: Christian Rice, P.E. File No. O542.00
 Equipment: Caterpillar 308E2 CR Excavator Test Pit Location: See Subsurface Exploration Location Plan Reviewed By: Glenn Olson, P.E.
 Capacity/Reach: 0.5± yd³ toothed bucket / 14± ft Ground Surface Elevation: 252± feet

Depth (in.)	Soil Horizon/Layer	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features (mottles)			Soil Texture (USDA)	Coarse Fragments		Soil Structure	Soil Consistence (Moist)	Other
			Depth (in.)	Color	Percent		Gravel	Cobbles & Stones			
0 - 20±	Topsoil Fill (A _p)	10YR/3/1	---	---	---	Sandy Loam	15±%	5±%	Granular	Friable	Occasional roots
20 - 110±	Natural Sand with Gravel (C)	2.5Y/6/3	---	---	---	Sand	20±%	5±%	Single Grain	Friable	6± inch thick seam of sandy silt observed at south end of test pit from 5± to 5.5± feet bgs.

Groundwater Observed: No Depth Weeping from Pit: N/A Depth Standing Water in Hole: N/A

Estimated Depth (Elevation) to Seasonal High Groundwater: >119± inches (<El 242.1± feet)

Notes:

- 1) Test pit dimensions: 8.5± feet (north/south) x 4± feet (east/west).
- 2) Grab sample GS-1 collected from approximately 24± to 42± inches (2± to 3.5± feet) below ground surface.
- 3) Constant head permeability test performed at approximately 24± inches (2± feet) below ground surface.
- 4) Test pit terminated at 110± inches (9.2± feet) below ground surface.

APPENDIX C

Test Pit Photo Log

NORTHEAST GEOTECHNICAL, INC.



Photograph #1

Description of Photograph:

Test Pit TP-101

Photograph Taken By:

Christian Rice, dated 3/4/2025



Photograph #2

Description of Photograph:

Test Pit TP-102

Photograph Taken By:

Christian Rice, dated 3/4/2025

NORTHEAST GEOTECHNICAL, INC.



Photograph #3

Description of Photograph:

Test Pit TP-103

Photograph Taken By:

Christian Rice, dated 3/4/2025



Photograph #4

Description of Photograph:

Test Pit TP-104

Photograph Taken By:

Christian Rice, dated 3/4/2025

NORTHEAST GEOTECHNICAL, INC.



Photograph #5

Description of Photograph:

Test Pit TP-105

Photograph Taken By:

Christian Rice, dated 3/4/2025



Photograph #6

Description of Photograph:

Test Pit TP-11

Photograph Taken By:

Christian Rice, dated 7/26/2024

NORTHEAST GEOTECHNICAL, INC.



Photograph #7

Description of Photograph:

Test Pit TP-12

Photograph Taken By:

Christian Rice, dated 7/26/2024

APPENDIX D

Soil Laboratory Test Results



195 Frances Avenue
 Cranston RI, 02910
 Phone: (401)-467-6454
 Fax: (401)-467-2398
cts.thielsch.com
Let's Build a Solid Foundation

Client Information:
Northeast Geotechnical
North Attleborough, MA
(508) 598-3510
 Project Contact: Glenn Olson
 Collected By: Client

Project Information:
NEAG Warehouse Addition
126 Grove St, Franklin, MA
 Project Number: O542.00
 Summary Page: 1 of 1
 Report Date: 03.12.25

LABORATORY TESTING DATA SHEET, Report No.: 7425-C-113

Material Source	Sample ID	Depth (ft)	Laboratory No.	Identification Tests										Proctor / CBR / Permeability Tests							Laboratory Log and Soil Description		
				As Rcvd Moisture Content %	LL %	PL %	OD LL	Gravel %	Sand %	Fines %	Org. %	pH	9 _d MAX (pcf) W _{opt} (%)	9 _d MAX (pcf) W _{opt} (Corr.)	Dry unit wt. (pcf)	Test Moisture Content %	Target Test Setup as % of Proctor	CBR @ 0.1"	CBR @ 0.2"	Permeability cm/sec			
				D2216	D4318			D6913			D2974	D4792	D1557										
TP-102	GS-1	1.2-3	25-S-776					0.1	37.7	62.2											Light Grey sandy silt // loam		
TP-103	GS-1	2.3-3.3	25-S-777					21.3	69.3	9.4											Brown poorly graded sand with silt and gravel // sand		
TP-104	GS-1	2.3-3.5	25-S-778					0.0	14.3	85.7											Grey silt // silt loam		
TP-105	GS-1	2-3.5	25-S-779					17.0	79.3	3.7											Brown poorly graded sand with gravel // sand		

Date Received: 03.05.25

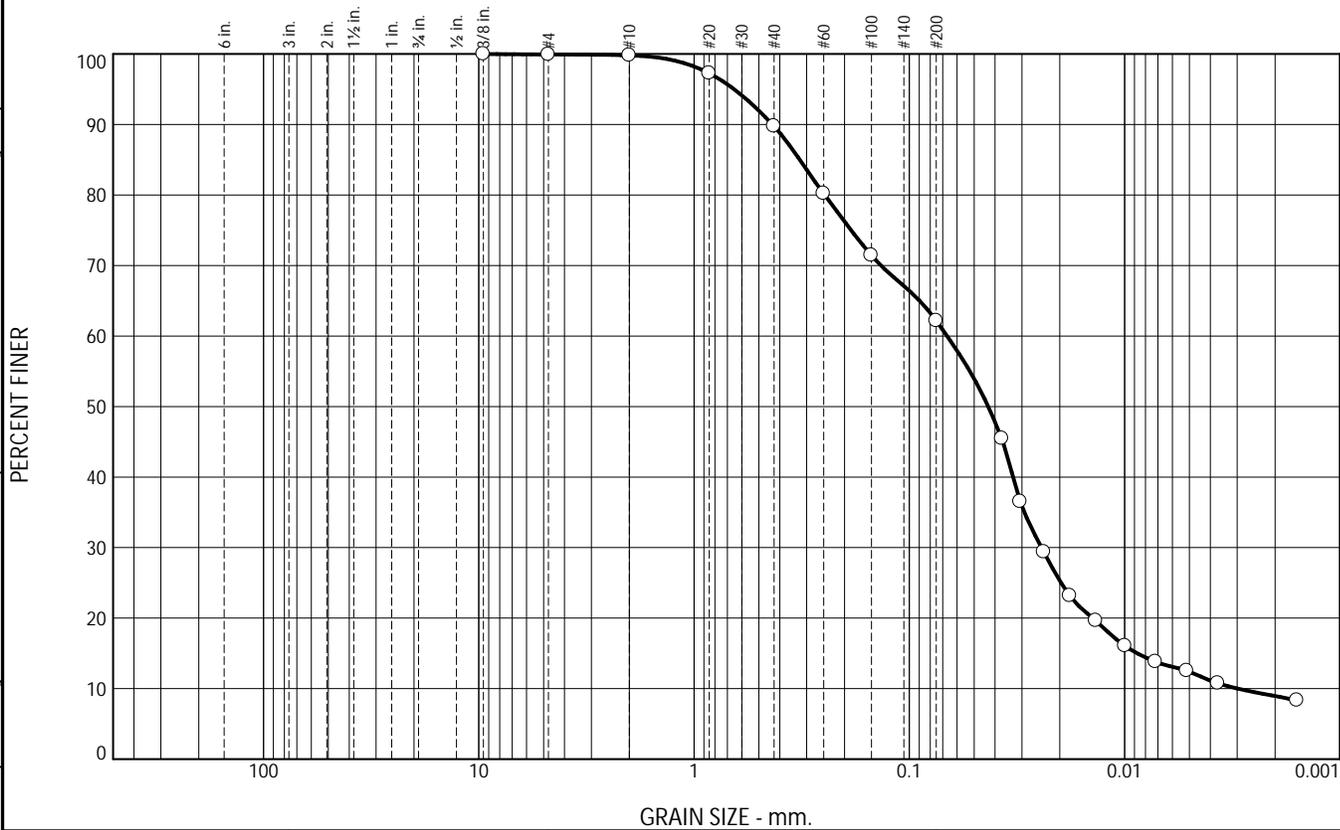
Reviewed By: 

Date Reviewed: 03.12.25

This report only relates to items inspect and/or tested. No warranty, expressed or implied, is made.
 This report shall not be reproduced, except in full, without prior written approval from the Agency, as defined in ASTM E329.

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Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.1	0.1	10.1	27.5	53.3	8.9

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3/8"	100.0		
#4	99.9		
#10	99.8		
#20	97.3		
#40	89.7		
#60	80.2		
#100	71.4		
#200	62.2		
0.0372 mm.	45.4		
0.0305 mm.	36.5		
0.0237 mm.	29.4		
0.0180 mm.	23.1		
0.0136 mm.	19.6		
0.0100 mm.	16.0		
0.0072 mm.	13.8		
0.0051 mm.	12.5		
0.0037 mm.	10.7		
0.0016 mm.	8.3		

* (no specification provided)

Soil Description

Light Grey sandy silt // loam

PL= NP	<u>Atterberg Limits</u>	PI= NP
	LL= NV	

<u>Coefficients</u>		
D ₉₀ = 0.4326	D ₈₅ = 0.3228	D ₆₀ = 0.0668
D ₅₀ = 0.0430	D ₃₀ = 0.0243	D ₁₅ = 0.0087
D ₁₀ = 0.0030	C _u = 22.39	C _c = 2.97

USCS= ML	<u>Classification</u>	AASHTO= A-4(0)
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Remarks

Source of Sample: Test Pit Depth: 1.2-3.0'
 Sample Number: TP-102 / GS-1

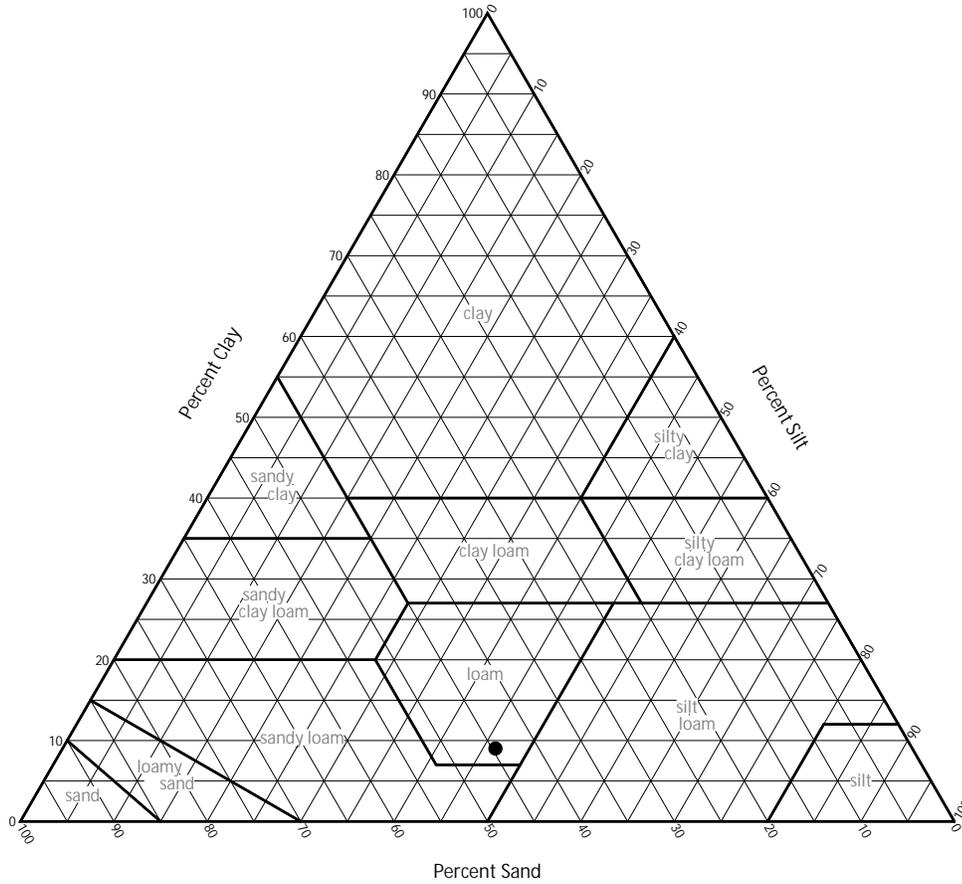
Date: 3/10/25

Thielsch Engineering Inc. Cranston, RI	Client: Northeast Geotechnical, Inc. Project: NEAG Warehouse Addition 126 Grove St, Franklin, MA Project No: O542.00
Fig. 25-S-776	

Tested By: AB/SBR Checked By: Rebecca Roth

These results are for the exclusive use of the client for whom they were obtained. This report only relates to items inspected and/or tested. No warranty, expressed or implied, is made.

USDA Soil Classification



SOIL DATA							
	Source	Sample No.	Depth	Percentages From Material Passing a #10 Sieve			Classification
				Sand	Silt	Clay	
●	Test Pit	TP-102 / GS-1	1.2-3.0'	44.6	46.5	8.9	Loam

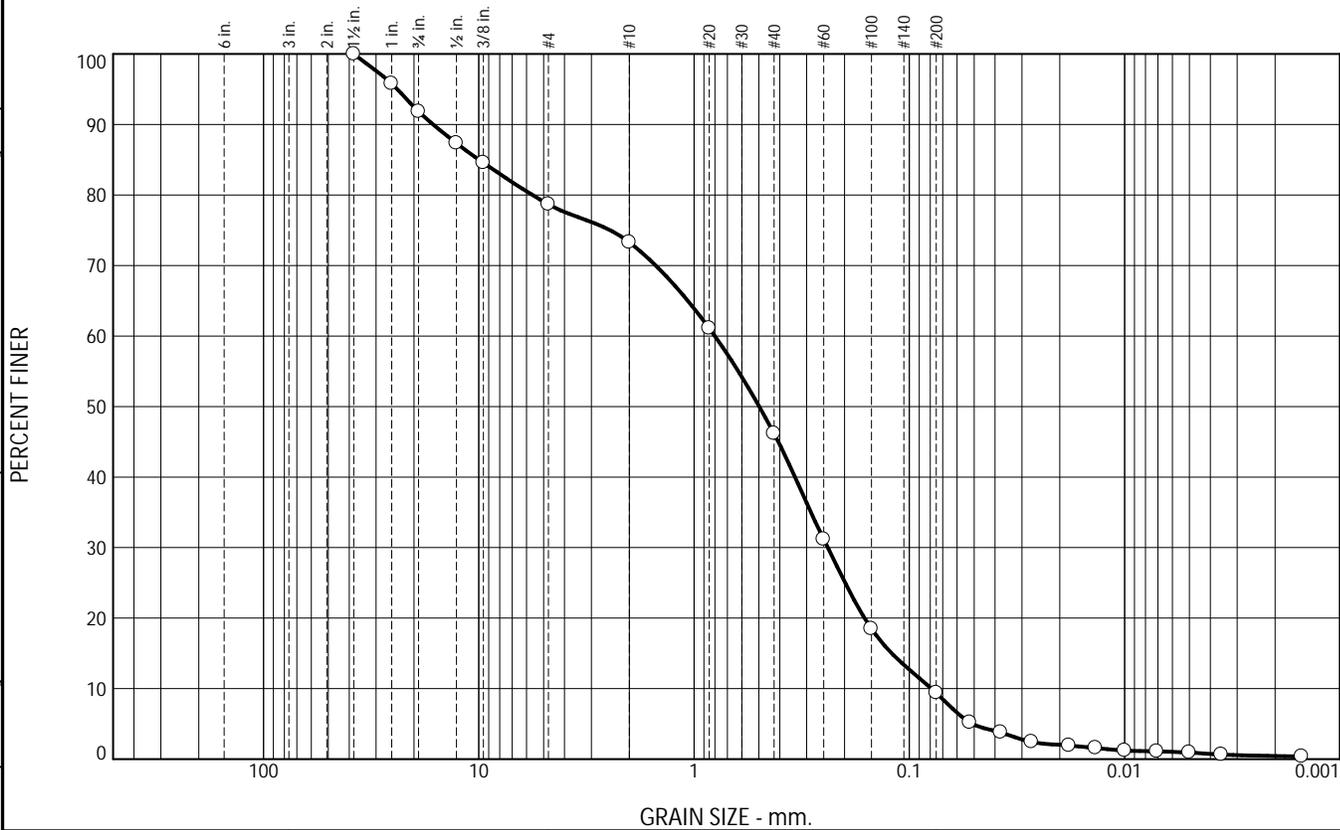
<p style="font-size: 1.2em; margin: 0;">Thielsch Engineering Inc.</p> <p style="margin: 0;">Cranston, RI</p>	<p>Client: Northeast Geotechnical, Inc.</p> <p>Project: NEAG Warehouse Addition 126 Grove St, Franklin, MA</p> <p>Project No.: O542.00</p>
--	--

Fig. 25-US-776

Checked By: RR

These results are for the exclusive use of the client for whom they were obtained. This report only relates to items inspected and/or tested. No warranty, expressed or implied, is made.

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	8.2	13.1	5.4	27.1	36.8	9.0	0.4

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1 1/2"	100.0		
1"	95.8		
3/4"	91.8		
1/2"	87.4		
3/8"	84.6		
#4	78.7		
#10	73.3		
#20	61.1		
#40	46.2		
#60	31.2		
#100	18.5		
#200	9.4		
0.0524 mm.	5.2		
0.0376 mm.	3.8		
0.0270 mm.	2.4		
0.0181 mm.	1.9		
0.0136 mm.	1.6		
0.0100 mm.	1.2		
0.0071 mm.	1.1		
0.0050 mm.	0.9		
0.0036 mm.	0.6		
0.0015 mm.	0.4		

* (no specification provided)

Soil Description

Brown poorly graded sand with silt and gravel // sand

PL= NP	<u>Atterberg Limits</u> LL= NV	PI= NP
--------	-----------------------------------	--------

<u>Coefficients</u>		
D ₉₀ = 16.2971	D ₈₅ = 9.9731	D ₆₀ = 0.8006
D ₅₀ = 0.4982	D ₃₀ = 0.2392	D ₁₅ = 0.1208
D ₁₀ = 0.0789	C _u = 10.14	C _c = 0.91

USCS= SP-SM	<u>Classification</u> AASHTO= A-1-b
-------------	--

Remarks

Source of Sample: Test Pit Depth: 2.3-3.3'
 Sample Number: TP-103 / GS-1

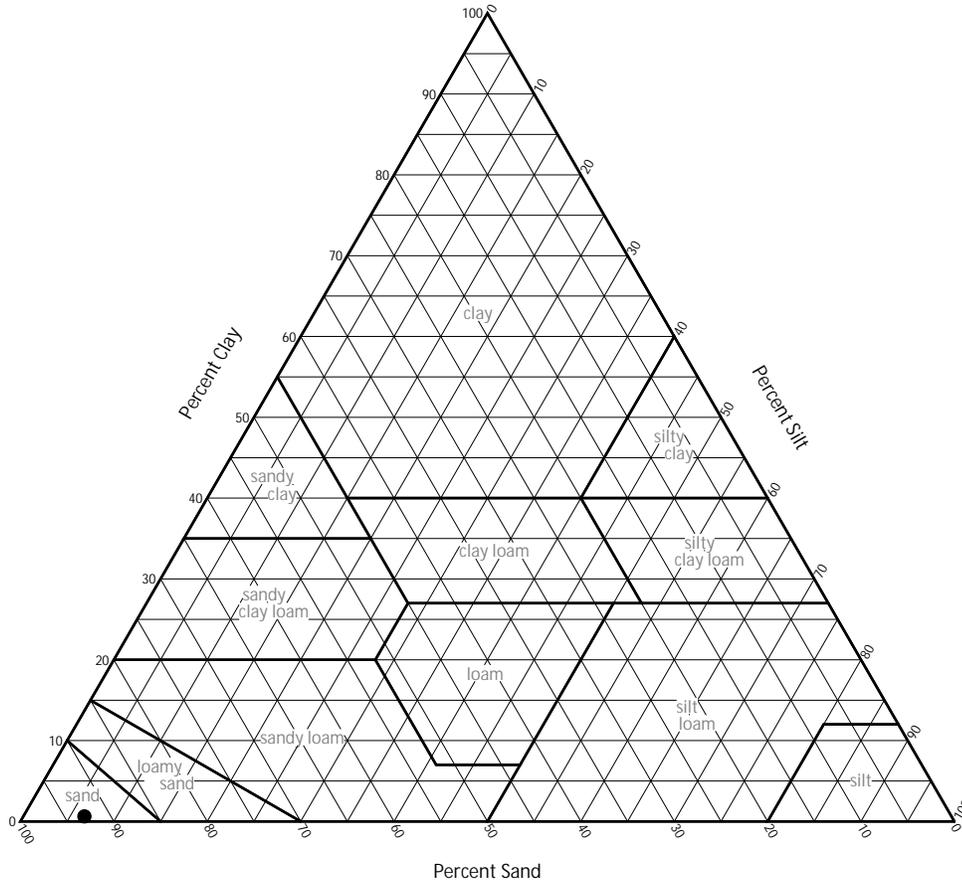
Date: 3/10/25

Thielsch Engineering Inc. Cranston, RI	Client: Northeast Geotechnical, Inc. Project: NEAG Warehouse Addition 126 Grove St, Franklin, MA Project No: O542.00
Fig. 25-S-777	

Tested By: AB/SBR Checked By: Rebecca Roth

These results are for the exclusive use of the client for whom they were obtained. This report only relates to items inspected and/or tested. No warranty, expressed or implied, is made.

USDA Soil Classification



SOIL DATA							
	Source	Sample No.	Depth	Percentages From Material Passing a #10 Sieve			Classification
				Sand	Silt	Clay	
●	Test Pit	TP-103 / GS-1	2.3-3.3'	92.8	6.7	0.5	Sand

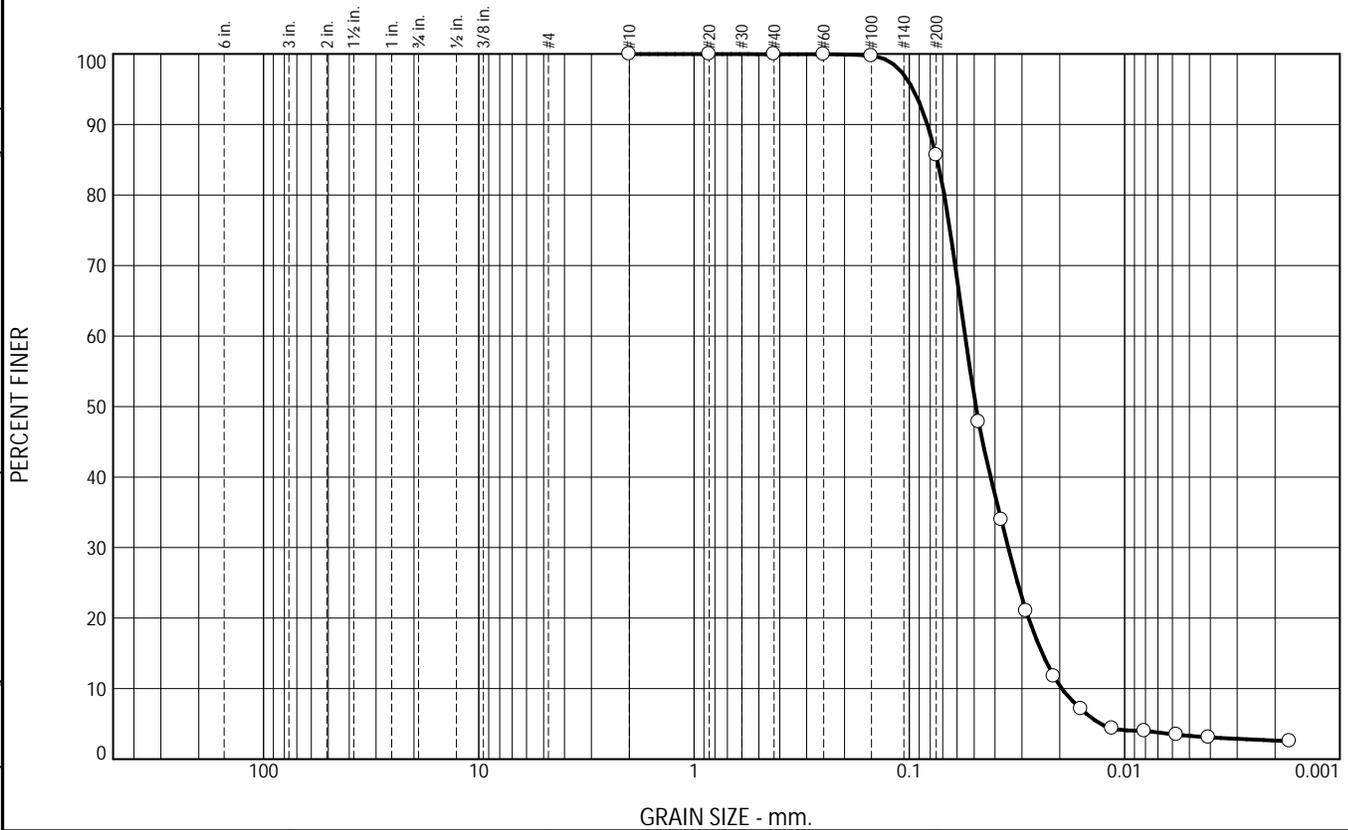
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--	--

Fig. 25-US-777

Checked By: RR

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Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.0	14.3	83.1	2.6

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
#10	100.0		
#20	100.0		
#40	100.0		
#60	100.0		
#100	99.7		
#200	85.7		
0.0477 mm.	47.8		
0.0374 mm.	34.0		
0.0287 mm.	21.0		
0.0213 mm.	11.8		
0.0160 mm.	7.1		
0.0114 mm.	4.3		
0.0081 mm.	4.0		
0.0057 mm.	3.5		
0.0041 mm.	3.1		
0.0017 mm.	2.5		

* (no specification provided)

Soil Description

Grey silt // silt loam

Atterberg Limits

PL= NP LL= NV PI= NP

Coefficients

D₉₀= 0.0824 D₈₅= 0.0741 D₆₀= 0.0551
D₅₀= 0.0491 D₃₀= 0.0347 D₁₅= 0.0241
D₁₀= 0.0196 C_u= 2.81 C_c= 1.12

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Sample visually classified as non-plastic. Sample could not be rolled to 1/4".

Source of Sample: Test Pit Depth: 2.3-3.5'
Sample Number: TP-104 / GS-1

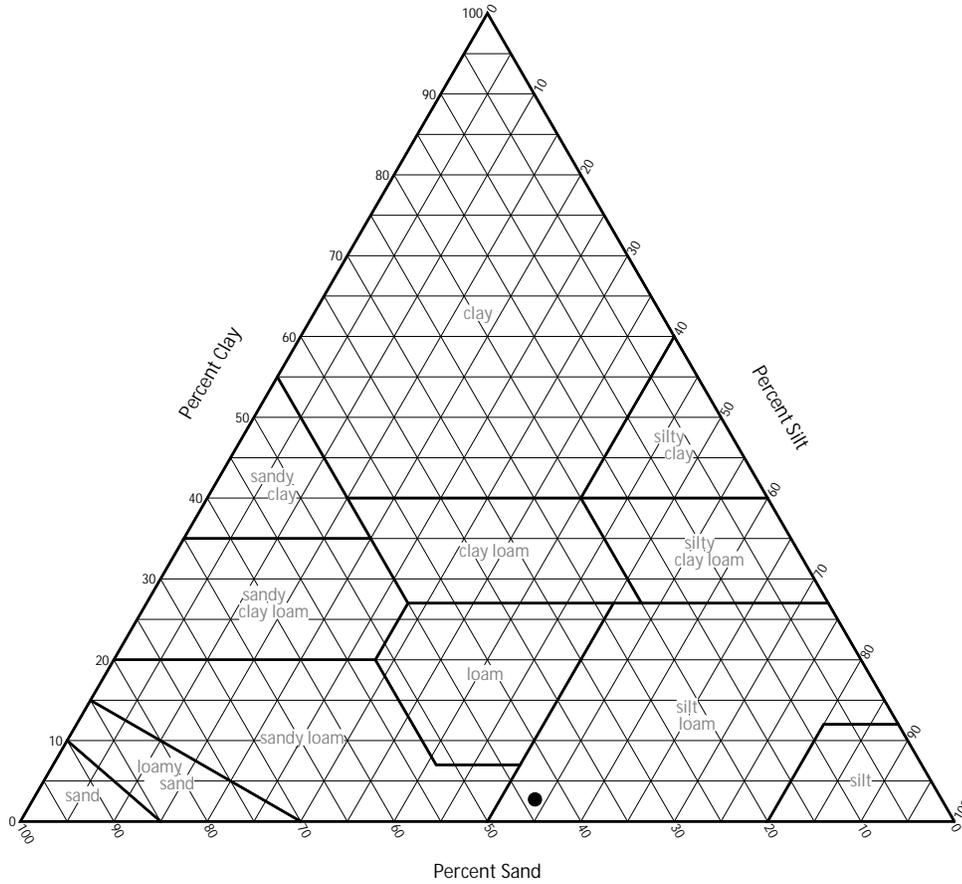
Date: 3/10/25

Thielsch Engineering Inc. Cranston, RI	Client: Northeast Geotechnical, Inc. Project: NEAG Warehouse Addition 126 Grove St, Franklin, MA Project No: O542.00
Fig. 25-S-778	

Tested By: AB Checked By: Rebecca Roth

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USDA Soil Classification



SOIL DATA							
	Source	Sample No.	Depth	Percentages From Material Passing a #10 Sieve			Classification
				Sand	Silt	Clay	
●	Test Pit	TP-104 / GS-1	2.3-3.5'	43.5	53.9	2.6	Silt loam

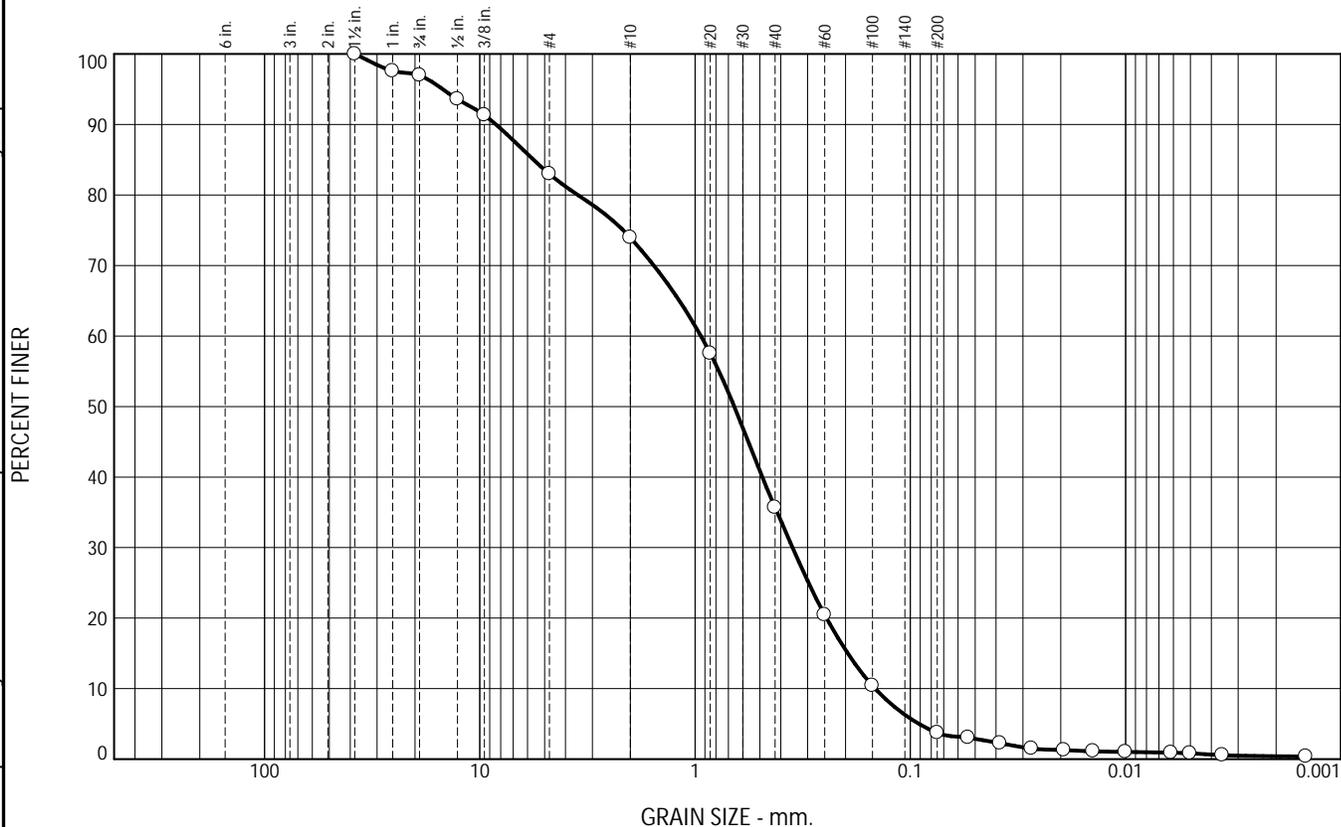
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--	--

Fig. 25-US-778

Checked By: RR

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Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	3.0	14.0	9.0	38.3	32.0	3.3	0.4

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1 1/2"	100.0		
1"	97.6		
3/4"	97.0		
1/2"	93.6		
3/8"	91.3		
#4	83.0		
#10	74.0		
#20	57.6		
#40	35.7		
#60	20.5		
#100	10.4		
#200	3.7		
0.0539 mm.	3.0		
0.0384 mm.	2.3		
0.0273 mm.	1.5		
0.0194 mm.	1.3		
0.0142 mm.	1.1		
0.0100 mm.	1.0		
0.0061 mm.	0.9		
0.0050 mm.	0.8		
0.0036 mm.	0.6		
0.0015 mm.	0.4		

* (no specification provided)

Soil Description

Brown poorly graded sand with gravel // sand

PL= NP	<u>Atterberg Limits</u> LL= NV	PI= NP
--------	-----------------------------------	--------

<u>Coefficients</u>		
D ₉₀ = 8.4098	D ₈₅ = 5.6249	D ₆₀ = 0.9396
D ₅₀ = 0.6577	D ₃₀ = 0.3524	D ₁₅ = 0.1954
D ₁₀ = 0.1458	C _u = 6.45	C _c = 0.91

USCS= SP	<u>Classification</u> AASHTO= A-1-b
----------	--

Remarks

Source of Sample: Test Pit Depth: 2-3.5.0'
 Sample Number: TP-105 / GS-1

Date: 3/12/25

Thielsch Engineering Inc.

Cranston, RI

Client: Northeast Geotechnical, Inc.
 Project: NEAG Warehouse Addition
 126 Grove St, Franklin, MA

Project No: O542.00

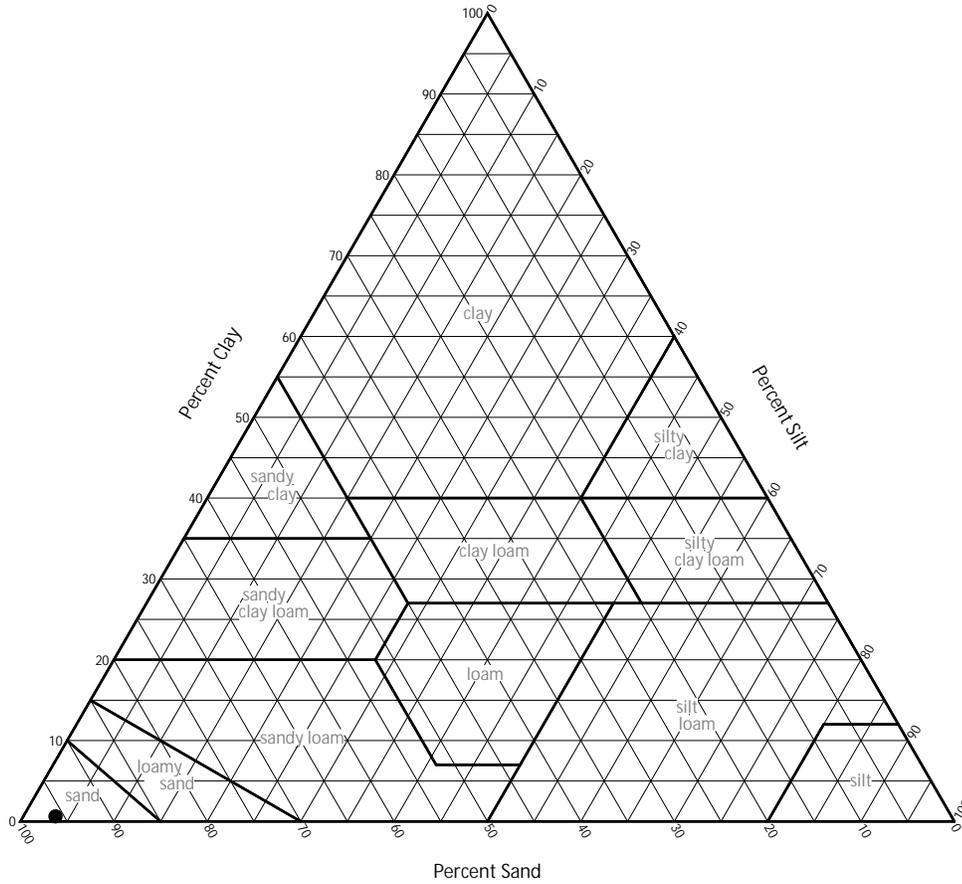
Fig. 25-S-779

Tested By: AB

Checked By: Rebecca Roth

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USDA Soil Classification



SOIL DATA							
	Source	Sample No.	Depth	Percentages From Material Passing a #10 Sieve			Classification
				Sand	Silt	Clay	
●	Test Pit	TP-105 / GS-1	2-3.5.0'	95.9	3.5	0.5	Sand

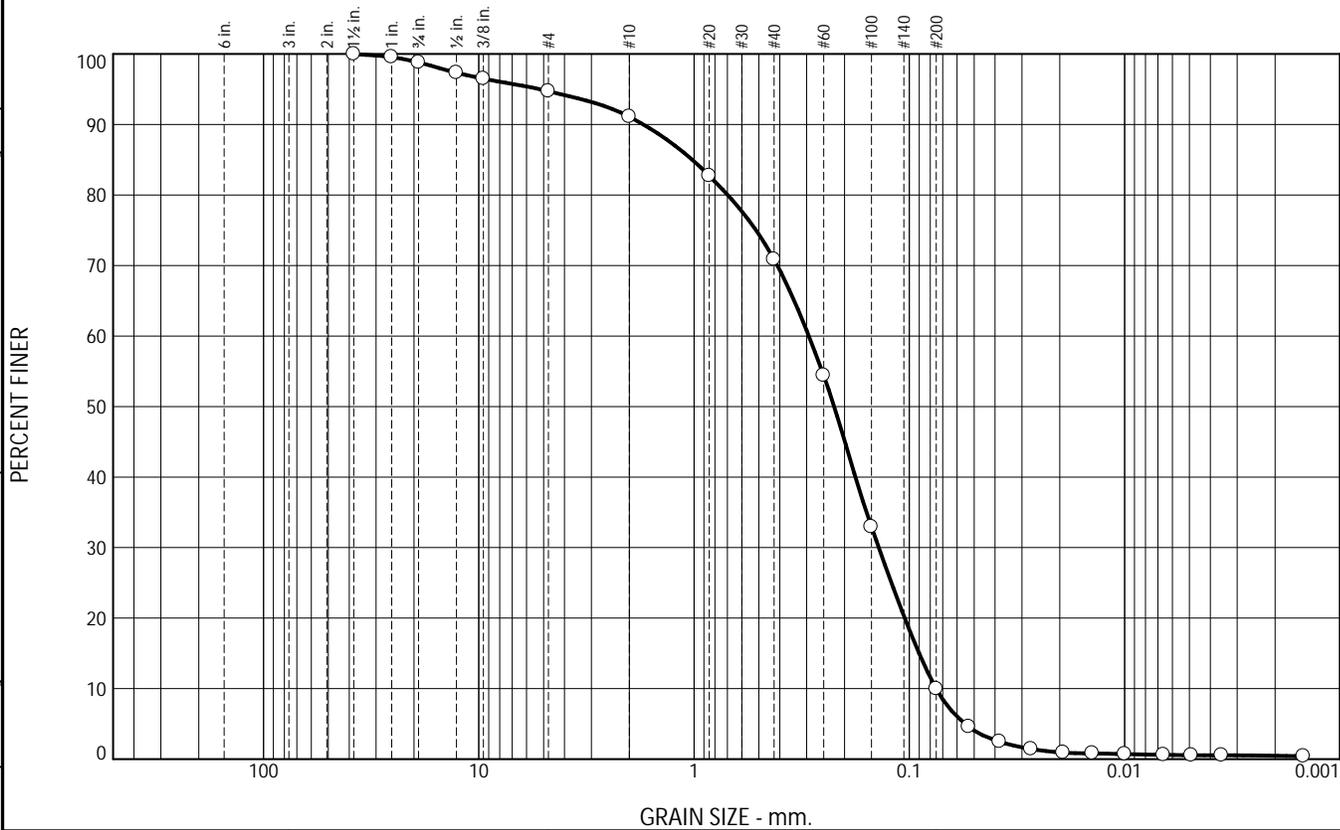
<p style="font-size: 1.2em; margin: 0;">Thielsch Engineering Inc.</p> <p style="margin: 0;">Cranston, RI</p>	<p>Client: Northeast Geotechnical, Inc.</p> <p>Project: NEAG Warehouse Addition 126 Grove St, Franklin, MA</p> <p>Project No.: O542.00</p>
--	--

Fig. 25-US-779

Checked By: RR

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Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	1.2	4.1	3.6	20.2	60.9	9.5	0.5

SIEVE SIZE OR DIAMETER	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
1 1/2"	100.0		
1"	99.6		
3/4"	98.8		
1/2"	97.3		
3/8"	96.5		
#4	94.7		
#10	91.1		
#20	82.7		
#40	70.9		
#60	54.4		
#100	32.9		
#200	10.0		
0.0529 mm.	4.6		
0.0381 mm.	2.5		
0.0272 mm.	1.4		
0.0193 mm.	0.9		
0.0141 mm.	0.8		
0.0100 mm.	0.7		
0.0066 mm.	0.6		
0.0049 mm.	0.5		
0.0035 mm.	0.5		
0.0015 mm.	0.4		

Soil Description

Light Brown poorly graded sand with silt // Sand

Atterberg Limits

PL= NP LL= NV PI= NP

Coefficients

D₉₀= 1.7224 D₈₅= 1.0197 D₆₀= 0.2931
 D₅₀= 0.2236 D₃₀= 0.1387 D₁₅= 0.0901
 D₁₀= 0.0751 C_u= 3.90 C_c= 0.87

Classification

USCS= SP-SM AASHTO= A-3

Remarks

* (no specification provided)

Source of Sample: Test Pits Depth: 2-4'
 Sample Number: TP-11 / GS-1

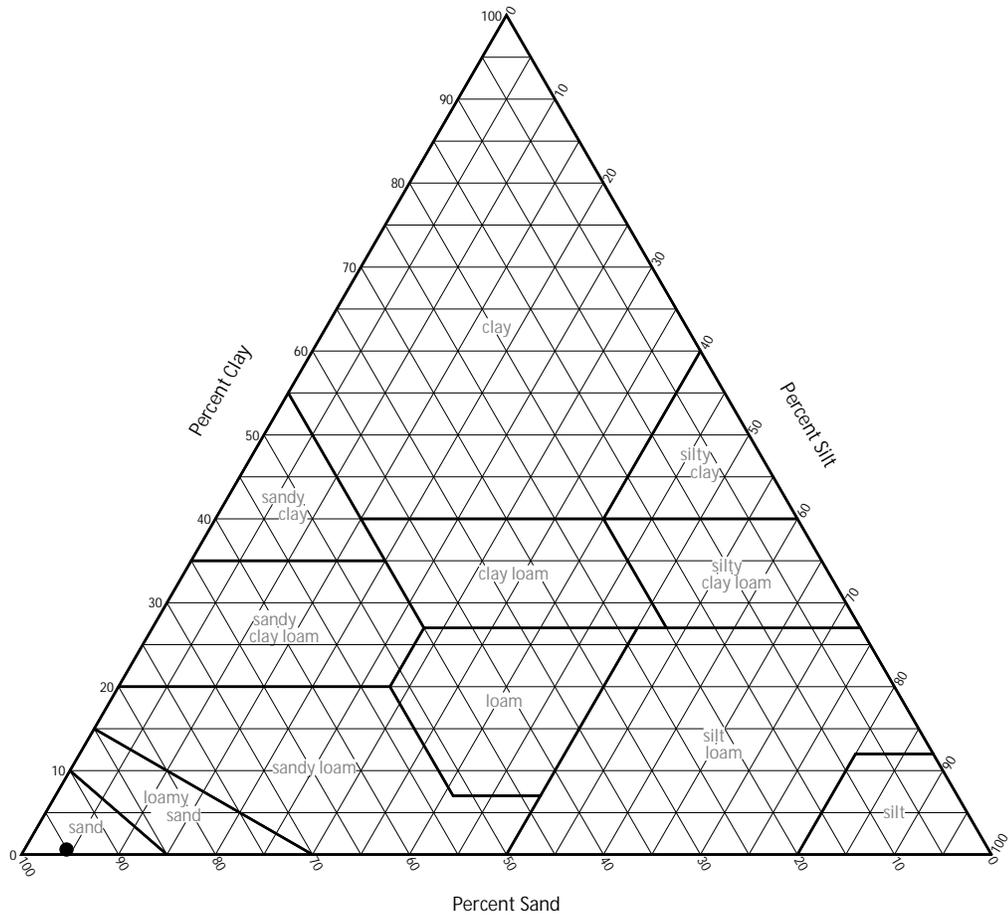
Date: 08.02.24

Thielsch Engineering Inc. Cranston, RI	Client: Northeast Geotechnical, Inc. Project: New England Appliance Group Warehouse Addition Franklin, MA Project No: O542.00
Fig. 24-S-2891	

Tested By: RB / MCS Checked By: Rebecca Roth

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USDA Soil Classification



SOIL DATA							
	Source	Sample No.	Depth	Percentages From Material Passing a #10 Sieve			Classification
				Sand	Silt	Clay	
●	Test Pits	TP-11 / GS-1	2-4'	95.0	4.5	0.5	Sand

Thielsch Engineering Inc.
Cranston, RI

Client: Northeast Geotechnical, Inc.
Project: New England Appliance Group Warehouse Addition
Franklin, MA
Project No.: O542.00

Fig. USDA-2891