



**ALLEN & MAJOR
ASSOCIATES, INC.**

DRAINAGE REPORT

40B Multi-Family Site Development
444 East Central Street Franklin, MA



APPLICANT:

TAG Central LLC
1070 E Indiantown Rd, Suite 308
Jupiter, FL 33477

PREPARED BY:

Allen & Major Associates, Inc.
Engineer of Record: Carlton M. Quinn
100 Commerce Way, Suite 5
Woburn, Massachusetts 01801



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**SECTION 1.0 -
DRAINAGE REPORT**



Introduction

The purpose of this drainage report is to provide an overview of the proposed stormwater management system (SMS) for the proposed Multifamily project located at 444 East Central Street in Franklin. The report will show by means of narrative, calculations and exhibits that the proposed stormwater management system will meet or exceed the Massachusetts Department of Environmental Protection (MassDEP) stormwater standards, and the Franklin Stormwater Management Regulations.

The proposed site improvements include the development of 4 residential buildings and a clubhouse. The four residential buildings are composed of 264 units, with 66 (25%) of the units designated as affordable. The proposed site includes 431 parking stalls, parking garages, sidewalks, waste enclosures, drive aisles, and amenity areas. This site will be serviced by water, sewer, and electrical, which will be coordinated with the Town of Franklin.

The proposed SMS incorporates [structural and non-structural] Best Management Practices (BMPs) to provide stormwater peak flow mitigation, quality treatment, and conveyance.

The SMS includes underground infiltration systems, water quality structures, and deep sump catch basins.

Site Categorization for Stormwater Regulations

The proposed site improvements at 444 East Central Street are considered a new development under the DEP Stormwater Management Standards due to the net increase in impervious area. A new development project is required to meet all the Stormwater Management Standards listed within the MA DEP Stormwater Handbook.

Site Location and Access

The parcel identified as 284-066-000-000 is a single lot with 200± feet of frontage on East Central Street entirely within the Town of Franklin. The lot area of the parcel is 14.6± acres. The parcel is located approximately 0.7 miles from the bordering town of Wrentham.

The parcel is abutted by residential neighborhoods on Red Gate Lane & Northern Spy Road to the east and south. Commercial developments abut the site to the west.

The site is accessed by an existing curb cut along East Central Street.

Existing Site Conditions

The site is currently owned and occupied by Stobart Properties LLC. A portion of the site is developed and contains a wood frame nursery dwelling, greenhouses, sheds, and accessory buildings with associated parking and drive aisles. The principal gross building area is approximately 11,500 sf. A large portion of the site is undisturbed, consisting of tree cover, wetlands, and a stream.



The surface drainage flows were analyzed at four Study Points. Study Point #1 summarizes on-site flows to a perennial stream at the northern section of the parcel. The flows for this study point are generated from the “front” of the site, including the principal nursery building and surrounding area as well as a large area abutting the property eastward. These flows are captured via catch basins and routed through an underground pipe system to discharge to the perennial stream. Study Point #2 summarizes on-site flows to a perennial stream south of the perennial stream at Study Point #1. The flows for this study point are generated from the surrounding developed area and discharge directly into the stream. Study Point #3 summarizes on site flows directed to the offsite detention abutting the parcel to the northwest. Study Point #4 summarizes on site flows directed to the southernmost perennial stream.

Watershed

The subject property is located within the Charles Watershed. The Charles River flows 80 miles from Hopkinton, Mass. to Boston Harbor. The entire Charles River drains rain and melted snow from a watershed area of 310 square miles. Two hundred and sixty-eight square miles of that watershed area drain over the Watertown Dam into the Lower Charles River. The remaining 42 square miles drain directly into the Lower Charles River. The river flows through the municipalities of Milford, Bellingham, Franklin, Medway, Millis, Medfield, Sherborn, Dover, Natick, Wellesley, Needham, Dedham, Newton, Waltham, Watertown, Cambridge and Boston, MA and from there into Boston Harbor.

Source: United States Environmental Protection Agency (EPA)

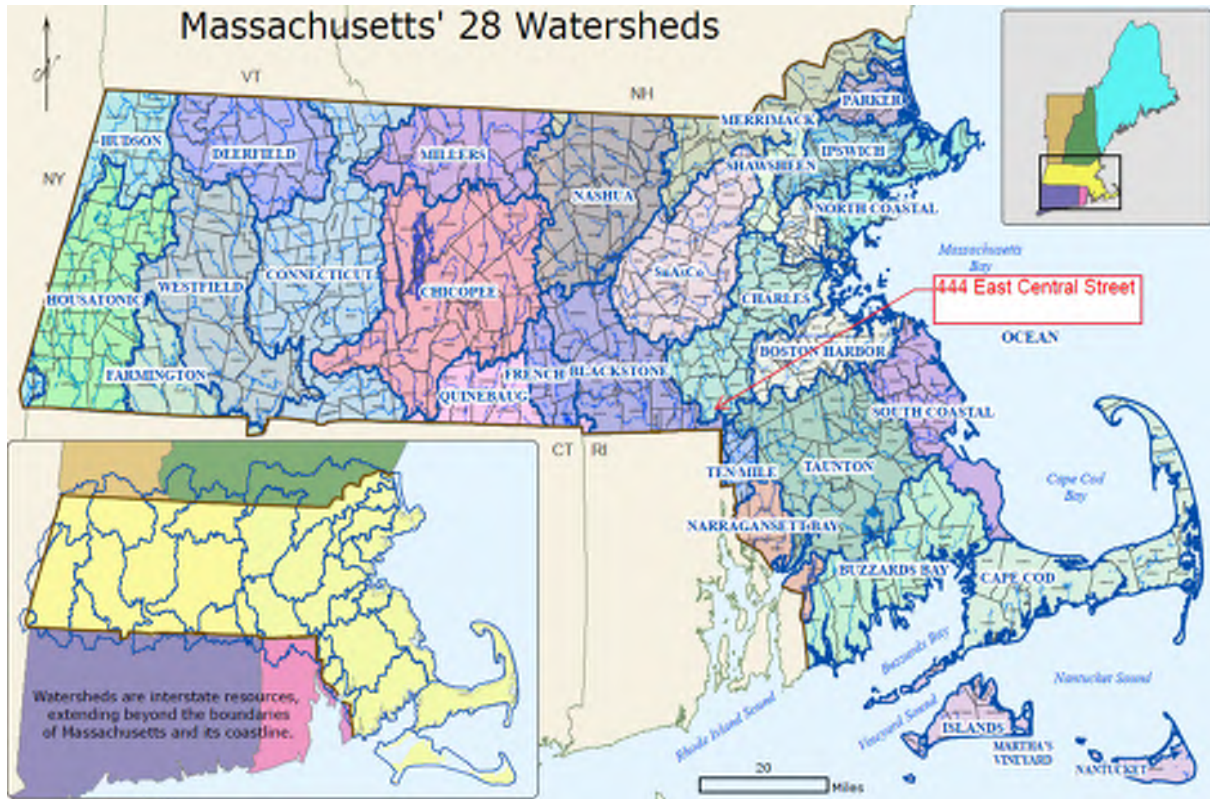


Exhibit 1: Watershed Map

Image Source: Mass.gov

Existing Soil Conditions

The on-site soils were identified using the USDA Natural Resources Conservation Services (NRCS) Soil Survey for Norfolk County. The site soil types, and corresponding Hydrologic Soil Groups (HSG) include:

- SCS 10 – Scarboro & Birdsall soils, 0 to 3 percent slopes, HSG A/D
- SCS 52 – Freetown muck, 0 to 1 percent slopes, HSG B/D
- SCS 254A – Merrimac Fine Sandy Loam, 0 to 3 percent slopes, HSG A
- SCS 254B – Merrimac Fine Sandy Loam, 3 to 8 percent slopes, HSG A
- SCS 260B – Sudbury Fine Sandy Loam, 2 to 8 percent slopes, HSG B
- SCS 420B – Canton Fine Sandy Loam, 3 to 8 percent slopes, HSG B

A copy of the NRCS Custom Soil Resource Report is included in the appendix of this report.

An exfiltration rate for the sandy loam has been determined to 1.02 inches per hour based upon Table 2.3.3 1982 Rawls Rate, Volume 3: Documenting Compliance with the Massachusetts's Stormwater Handbook.



FEMA Floodplain/Environmental Due Diligence

There are portions of the site located within the FEMA Zone "A" Special Flood Hazard Area Subject to Inundation by the 1% Annual Chance Flood (100-year floodplain). Zone "A" represents that there are no base flood elevations determined. There are also portions of the site encroached by Zone "X" Other Flood areas subject to 0.2% annual flood. The official Flood Insurance Rate Map (FIRM) effective date July 17, 2012, community panel 309 of 340, map number 25021C0309E. See Section 3 of this report for a copy of the FEMA FIRM.

Environmentally Sensitive Zones

The Commonwealth of Massachusetts asserts control over numerous protected and regulated areas including: Areas of Critical Environmental Concern (ACEC); Outstanding Resource Waters (ORWs); Priority and Protected Habitat for rare and endangered species, and areas protected under the Wetlands Protection Act. According to the MassGIS online map viewer MassMapper, the subject property is not located within AEC, ORW, or Priority and Protected Habitat for rare and endangered species areas. The subject is located within a few DEP wetlands. See Section 3 of this report for a copy of the wetland exhibit.

Drainage Analysis Methodology

A peak rate of runoff will be determined using techniques and data found in the following:

1. Urban Hydrology for Small Watersheds – Technical Release 55 by the United States Department of Agriculture Soils Conservation Service, June 1986. Runoff curve numbers and 24-hour precipitation values were obtained from this reference.
2. HydroCAD © Stormwater Modeling System by HydroCAD Software Solutions LLC, version 10.00-24. The HydroCAD program was used to generate the runoff hydrographs for the watershed areas, to determine discharge/ stage/storage characteristics for the stormwater BMPs, to perform drainage routing and to combine the results of the runoff hydrographs. HydroCAD uses the TR-20 methodology of the SCS Unit Hydrograph procedure (SCS-UH).

Proposed Conditions – Peak Rate of Runoff

The stormwater runoff analysis of the existing and proposed conditions includes an estimate of the peak rate of runoff from various rainfall events. Peak runoff rates were developed using TR55 Urban Hydrology for Small Watersheds, developed by the U.S. Department of Commerce, Engineering Division and the HydroCAD computer program. Further, the analysis has been prepared in accordance with the MassDEP and the city requirements and standard engineering practices. The peak rate of runoff has been estimated for each watershed during the 2, 10, and 100-year storm events.

The proposed stormwater management system for the site consists of deep sump catch basins, proprietary separators, and subsurface infiltration structures. These systems have



been designed in accordance with the MA DEP Stormwater Management Policy to recharge groundwater and reduce rate of runoff from the parcel.

Infiltration System 1,3,4,5 discharges to Study Point #4. Infiltration System 2 discharges Study Point #2.

The stormwater runoff model indicates that the proposed site development reduces the rate of runoff during all storm events at the identified points of analysis. The following tables provide a summary of the estimated peak rate, in Cubic Feet per Second (CFS) at each of the four (4) Study Points for each of the design storm events. The HydroCAD worksheets are included in Section 4 and 5 of this report.

STUDY POINT FLOW (CFS) SUMMARY

STUDY POINT #1			
	2-Year	10-Year	100-Year
Existing Flow (CFS)	2.63	4.10	7.59
Proposed Flow (CFS)	0.83	2.24	6.88
Reduction %	68%	45%	9%

STUDY POINT #2			
	2-Year	10-Year	100-Year
Existing Flow (CFS)	3.97	6.28	11.39
Proposed Flow (CFS)	0.78	2.08	7.42
Reduction %	80%	67%	35%

STUDY POINT #3			
	2-Year	10-Year	100-Year
Existing Flow (CFS)	7.78	12.22	22.52
Proposed Flow (CFS)	3.08	5.77	18.24
Reduction %	60%	53%	19%



STUDY POINT VOLUME (CF) SUMMARY

STUDY POINT #1			
	2-Year	10-Year	100-Year
Existing Flow (CF)	9,929	17,097	31,620
Proposed Flow (CF)	1,145	3,647	19,352
Reduction %	88%	79%	39%

STUDY POINT #2			
	2-Year	10-Year	100-Year
Existing Flow (CF)	16,062	27,695	51,386
Proposed Flow (CF)	1,796	4,931	22,678
Reduction %	89%	82%	56%

STUDY POINT #3			
	2-Year	10-Year	100-Year
Existing Flow (CF)	33,678	58,725	126,094
Proposed Flow (CF)	11,850	24,162	79,294
Reduction %	65%	59%	37%

MASSDEP Stormwater Performance Standards

The MA DEP Stormwater Management Policy was developed to improve water quality by implementing performance standards for stormwater management. The intent is to implement the stormwater management standards through the review of Notice of Intent filings by the issuing authority (Conservation Commission or DEP). The following section outlines how the proposed Stormwater Management System meets the standards set forth by the Policy.

BMP's implemented in the design include –

- Deep Sump Catch Basins
- Proprietary Separators
- Subsurface Structures

Stormwater Best Management Practices (BMP's) have been incorporated into the design of the project to mitigate the anticipated pollutant loading. An Operations and Maintenance Plan has been developed for the project, which addresses the long-term maintenance requirements of the proposed system.



Temporary erosion and sedimentation controls will be incorporated into the construction phase of the project. These temporary controls may include straw bale and/or silt fence barriers, inlet sediment traps, slope stabilization, and stabilized construction entrances.

The Massachusetts Department of Environmental Protection has established ten (10) Stormwater Management Standards. A project that meets or exceeds the standards is presumed to satisfy the regulatory requirements regarding stormwater management. The Standards are enumerated below as well as descriptions and supporting calculations as to how the Project will comply with the Standards:

1. *No new stormwater conveyances (e.g. outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.*

The proposed development will not introduce any new outfalls with direct discharge to a wetland area or waters of the Commonwealth of Massachusetts. All discharges will be treated for water quality and the rate will not be increased over existing conditions.

2. *Stormwater management systems shall be designed so that post-development peak discharge rates do not exceed pre-development peak discharge rates. This Standard may be waived for discharges to land subject to coastal storm flowage as defined in 310 CMR 10.04.*

The proposed development has been designed so that the post-development peak discharge rates do not exceed the predevelopment peak discharge rates. A summary of the existing and proposed discharge rates is included within this document.

3. *Loss of annual recharge to groundwater shall be eliminated or minimized through the use of infiltration measures including environmentally sensitive site design, low impact development techniques, stormwater best management practices, and good operation and maintenance. At a minimum, the annual recharge from the post-development site shall approximate the annual recharge from pre-development conditions based on soil type. This Standard is met when the stormwater management system is designed to infiltrate the required recharge volume as determined in accordance with the Massachusetts Stormwater Handbook.*

The existing annual recharge for the site has been approximated in the proposed condition. There are proposed subsurface infiltration systems designed to meet this requirement. Stormwater runoff generated from the impervious areas of the proposed development are routed through these infiltration BMPs. The proposed Recharge Volume is based on the Static Method per the MA DEP Stormwater Management Standards, Volume 3, Chapter 1.



4. *Stormwater management systems shall be designed to remove 80% of the average annual post-construction load of Total Suspended Solids (TSS). This standard is met when:*
- *Suitable practices for source control and pollution prevention are identified in a long-term pollution prevention plan, and thereafter are implemented and maintained;*
 - *Structural stormwater best management practices are sized to capture the required water quality volume determined in accordance with the Massachusetts Stormwater Handbook; and*
 - *Pretreatment is provided in accordance with the Massachusetts Stormwater Handbook.*

The proposed stormwater management system has been designed so that the 44% TSS removal standard will be met for each drainage area. Standard #4 is met when structural stormwater best management practices are sized to capture and treat the required water quality volume and pretreatment is provided in accordance with the Massachusetts Stormwater Handbook. Standard #4 also requires that suitable source control measures are identified in the Long-Term Pollution Prevention Plan. The 44% TSS removal standard will be met using some combination of the following: deep-sump, hooded catch basins, subsurface infiltration systems with Isolator Rows and proprietary separators.

The water quality volume for the site development will be captured and treated using proprietary separators and infiltration systems equipped with Isolator Rows. All systems have been sized to meet the water quality flow rate for the 0.5" storm event.

5. *For land uses with higher potential pollutant loads, source control and pollution prevention shall be implemented in accordance with the Massachusetts Stormwater Handbook to eliminate or reduce the discharge of stormwater runoff from such land uses to the maximum extent practicable. If through source control and/or pollution prevention all land uses with higher potential pollutant loads cannot be completely protected from exposure to rain, snow, snow melt, and stormwater runoff, the proponent shall use the specific structural stormwater BMPs determined by the Department to be suitable for such uses as provided in the Massachusetts Stormwater Handbook. Stormwater discharges from land uses with higher potential pollutant loads shall also comply with the requirements of the Massachusetts Clean Waters*



Act, M.G.L. c. 21, §§ 26-53 and the regulations promulgated thereunder at 314 CMR 3.00, 314 CMR 4.00 and 314 CMR 5.00.

The site is considered a source of higher potential pollutant loads because [it has a parking lot with 1,000 vehicle trips per day or more. Shopping centers, malls, and large office parks.] Pretreatment and Source reduction is provided to the maximum extent practicable. The drainage system will be designed to treat 1" water quality volume utilizing the BMPs listed in Table LUHPPL, within the Massachusetts Stormwater Handbook, Volume 1: Overview of the Massachusetts Stormwater Standards, Chapter 1, Page 14. This requirement only applies to stormwater discharges that come into contact with the actual area or activity on the site that may generate the higher potential pollutant load.

6. *Stormwater discharges within the Zone II or Interim Wellhead Protection Area of a public water supply, and stormwater discharges near or to any other critical area, require the use of the specific source control and pollution prevention measures and the specific structural stormwater best management practices determined by the Department to be suitable for managing discharges to such areas, as provided in the Massachusetts Stormwater Handbook. A discharge is near a critical area if there is a strong likelihood of a significant impact occurring to said area, taking into account site-specific factors. Stormwater discharges to Outstanding Resource Waters and Special Resource Waters shall be removed and set back from the receiving water or wetland and receive the highest and best practical method of treatment. A "storm water discharge" as defined in 314 CMR 3.04(2)(a)1 or (b) to an Outstanding Resource Water or Special Resource Water shall comply with 314 CMR 3.00 and 314 CMR 4.00. Stormwater discharges to a Zone I or Zone A are prohibited unless essential to the operation of a public water supply.*

The stormwater systems have been sized to properly treat stormwater discharges to critical areas.

7. *A redevelopment project is required to meet the following Stormwater Management Standards only to the maximum extent practicable: Standard 2, Standard 3, and the pretreatment and structural best management practice requirements of Standards 4, 5, and 6. Existing stormwater discharges shall comply with Standard 1 only to the maximum extent practicable. A redevelopment project shall also comply with all other requirements of the Stormwater Management Standards and improve existing conditions.*



The proposed project is not considered a re-development project under the Stormwater Management Handbook guidelines as there is an increase in the amount of impervious area.

8. *A plan to control construction-related impacts including erosion, sedimentation and other pollutant sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plan) shall be developed and implemented.*

A plan to control construction-related impacts, including erosion, sedimentation and other pollutant sources during construction has been developed. A detailed Erosion and Sedimentation Control Plan is included in the Permit Drawings. The proponent will prepare and submit a Stormwater Pollution Prevention Plan (SWPPP) prior to commencement of construction activities that will result in the disturbance of one acre of land or more.

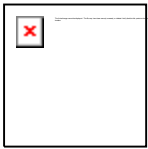
9. *A long-term operation and maintenance plan shall be developed and implemented to ensure that stormwater management systems function as designed.*

A Long-Term Operation & Maintenance (O&M) Plan has been developed for the proposed stormwater management system and is included within this document. See Section 2.0 of this report.

10. *All illicit discharges to the stormwater management system are prohibited.*

There are no expected illicit discharges to the stormwater management system. The applicant will submit the Illicit Discharge Compliance Statement prior to the discharge of stormwater runoff to the post-construction stormwater best management practices and prior to the issuance of a Certificate of Compliance.

See the next page for the MassDEP Stormwater Checklist.



Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the [Massachusetts Stormwater Handbook](#). The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.¹ This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

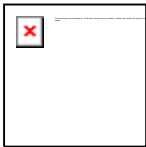
In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

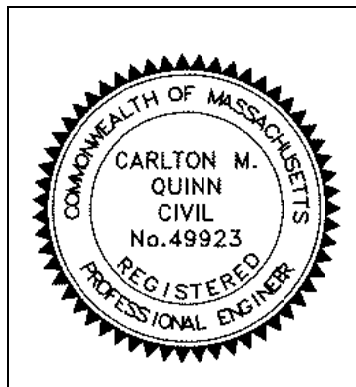
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature

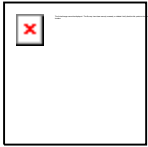


 7.21.25
Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

- ☒ New development
- ☐ Redevelopment
- ☐ Mix of New Development and Redevelopment



Checklist for Stormwater Report

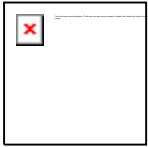
Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

- ☐ No disturbance to any Wetland Resource Areas
- ☐ Site Design Practices (e.g. clustered development, reduced frontage setbacks)
- ☐ Reduced Impervious Area (Redevelopment Only)
- ☐ Minimizing disturbance to existing trees and shrubs
- ☐ LID Site Design Credit Requested:
 - ☐ Credit 1
 - ☐ Credit 2
 - ☐ Credit 3
- ☐ Use of “country drainage” versus curb and gutter conveyance and pipe
- ☐ Bioretention Cells (includes Rain Gardens)
- ☐ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
- ☐ Treebox Filter
- ☐ Water Quality Swale
- ☐ Grass Channel
- ☐ Green Roof
- ☒ Other (describe): Underground Infiltration Systems

Standard 1: No New Untreated Discharges

- ☒ No new untreated discharges
- ☒ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- ☒ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Checklist for Stormwater Report

Checklist (continued)

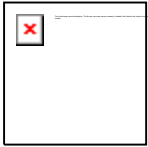
Standard 2: Peak Rate Attenuation

- ☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- ☐ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
- ☒ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

- ☒ Soil Analysis provided.
- ☒ Required Recharge Volume calculation provided.
- ☐ Required Recharge volume reduced through use of the LID site Design Credits.
- ☒ Sizing the infiltration, BMPs is based on the following method: Check the method used.
 - ☒ Static
 - ☐ Simple Dynamic
 - ☐ Dynamic Field¹
- ☒ Runoff from all impervious areas at the site discharging to the infiltration BMP.
- ☐ Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
- ☒ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
- ☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:
 - ☐ Site is comprised solely of C and D soils and/or bedrock at the land surface
 - ☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
 - ☐ Solid Waste Landfill pursuant to 310 CMR 19.000
 - ☐ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- ☒ Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
- ☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Checklist for Stormwater Report

Checklist (continued)

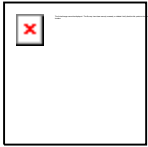
Standard 3: Recharge (continued)

- ☒ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
- ☐ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
 - Provisions for storing materials and waste products inside or under cover;
 - Vehicle washing controls;
 - Requirements for routine inspections and maintenance of stormwater BMPs;
 - Spill prevention and response plans;
 - Provisions for maintenance of lawns, gardens, and other landscaped areas;
 - Requirements for storage and use of fertilizers, herbicides, and pesticides;
 - Pet waste management provisions;
 - Provisions for operation and management of septic systems;
 - Provisions for solid waste management;
 - Snow disposal and plowing plans relative to Wetland Resource Areas;
 - Winter Road Salt and/or Sand Use and Storage restrictions;
 - Street sweeping schedules;
 - Provisions for prevention of illicit discharges to the stormwater management system;
 - Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
 - Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
 - List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- ☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
 - ☒ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - ☐ is within the Zone II or Interim Wellhead Protection Area
 - ☐ is near or to other critical areas
 - ☐ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - ☒ involves runoff from land uses with higher potential pollutant loads.
 - ☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.
 - ☒ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

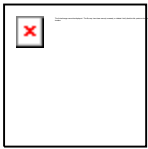
- ☒ The BMP is sized (and calculations provided) based on:
 - ☒ The ½" or 1" Water Quality Volume or
 - ☐ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- ☒ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- ☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- ☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- ☒ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- ☐ The NPDES Multi-Sector General Permit does **not** cover the land use.
- ☒ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- ☐ All exposure has been eliminated.
- ☒ All exposure has **not** been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- ☒ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

- ☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- ☐ Critical areas and BMPs are identified in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

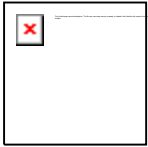
Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

- ☐ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:
 - ☐ Limited Project
 - ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
 - ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
 - ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
 - ☐ Bike Path and/or Foot Path
 - ☐ Redevelopment Project
 - ☐ Redevelopment portion of mix of new and redevelopment.
- ☐ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
- ☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
 - Construction Period Operation and Maintenance Plan;
 - Names of Persons or Entity Responsible for Plan Compliance;
 - Construction Period Pollution Prevention Measures;
 - Erosion and Sedimentation Control Plan Drawings;
 - Detail drawings and specifications for erosion control BMPs, including sizing calculations;
 - Vegetation Planning;
 - Site Development Plan;
 - Construction Sequencing Plan;
 - Sequencing of Erosion and Sedimentation Controls;
 - Operation and Maintenance of Erosion and Sedimentation Controls;
 - Inspection Schedule;
 - Maintenance Schedule;
 - Inspection and Maintenance Log Form.
- ☒ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- ☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has **not** been included in the Stormwater Report but will be submitted **before** land disturbance begins.
- ☐ The project is **not** covered by a NPDES Construction General Permit.
- ☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- ☒ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

- ☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - ☒ Name of the stormwater management system owners;
 - ☒ Party responsible for operation and maintenance;
 - ☒ Schedule for implementation of routine and non-routine maintenance tasks;
 - ☒ Plan showing the location of all stormwater BMPs maintenance access areas;
 - ☒ Description and delineation of public safety features;
 - ☒ Estimated operation and maintenance budget; and
 - ☒ Operation and Maintenance Log Form.
- ☐ The responsible party is **not** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

- ☒ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- ☒ An Illicit Discharge Compliance Statement is attached;
- ☐ NO Illicit Discharge Compliance Statement is attached but will be submitted **prior to** the discharge of any stormwater to post-construction BMPs.



SECTION 2.0 - OPERATION & MAINTENANCE PLAN



Introduction

In accordance with the standards set forth by the Stormwater Management Policy issued by the Massachusetts Department of Environmental Protection (MassDEP), Allen & Major Associates, Inc. has prepared the following Operations & Maintenance (O&M) Plan for the existing development at 444 East Central Street, Franklin, MA.

The plan is broken down into three major sections. The first section describes construction-related erosion and sedimentation controls (Demolition & Construction Maintenance Plan). The second section describes the long-term pollution prevention measures (Long Term Pollution Prevention Plan). The third section is a post-construction operation and maintenance plan designed to address the long-term maintenance needs of the stormwater management system (Long-Term Maintenance Plan – Facilities Description).

Notification Procedures for Change of Responsibility for O&M

The Stormwater Management System (SMS) for this project is owned by TAG Central LLC. The owner shall be legally responsible for the long-term operation and maintenance of this SMS as outlined in this Operation and Maintenance Plan.

The owner shall submit an annual summary report and the completed Operation & Maintenance Schedule & Checklist to the Conservation Commission (via email or print copy), highlighting inspection and maintenance activities including performances of BMPs. Should ownership of the SMS change, the owner will continue to be responsible until the succeeding owner shall notify the Commission that the succeeding owner has assumed such responsibility. Upon subsequent transfers, the responsibility shall continue to be that of transferring owner until the transferee owner notifies the Commission of its assumption of responsibility.

In the event the SMS will serve multiple lots/owners, such as the subdivision of the existing parcel or creation of lease areas, the owner(s) shall establish an association or other legally enforceable arrangements under which the association or a single party shall have legal responsibility for the operation and maintenance of the entire SMS. The legal instrument creating such responsibility shall be recorded with the Registry of Deeds and promptly following its recording, a copy thereof shall be furnished to the Commission.



Contact Information

Stormwater Management System Owner: TAG Central LLC
1070 E Indiantown Rd
Jupiter, FL

Emergency Contact Information:

TAG Central LLC (Owner/Operator)	Phone: (561) 685-5336
Allen & Major Associates, Inc. (Site Civil Engineer)	Phone: (781) 935-6889
Franklin Department of Public Works	Phone: (508) 553-5500
Franklin Conservation Commission	Phone: (508) 520-4929
Franklin Fire Department (non-emergency line)	Phone: (508) 528-2323
MassDEP Emergency Response	Phone: (888) 304-1133
Clean Harbors Inc (24-Hour Line)	Phone: (800) 645-8265

Demolition & Construction Maintenance Plan

1. Call Digsafe: 1-888-344-7233
2. Contact the City at least three (3) days prior to start of demolition and/or construction activities.
3. Install Erosion Control measures as shown on the Plans prepared by A&M. The City shall review the installation of straw bales and silt fencing prior to the start of any site demolition work. Install Construction fencing if determined to be necessary at the commencement of construction.
4. Install construction entrances, straw bales, and silt fence at the locations shown on the Erosion Control Plan prepared by A&M.
5. Site access shall be achieved only from the designated construction entrances.
6. Cut and clear trees in construction areas only (within the limit of work; see plans).
7. Stockpiles of materials subject to erosion shall be stabilized with erosion control matting or temporary seeding whenever practicable, but in no case more than 14 days after the construction activity in that portion of the site has temporarily or permanently ceased.
8. Install silt sacks and straw bales around each drain inlet prior to any demolition and or construction activities.



9. All erosion control measures shall be inspected weekly and after every rainfall event. Records of these inspections shall be kept on-site for review.
10. All erosion control measures shall be maintained, repaired, or replaced as required or at the direction of the owner's engineer or the City.
11. Sediment accumulation up-gradient of the straw bales, silt fence, and stone check dams greater than 6" in depth shall be removed and disposed of in accordance with all applicable regulations.
12. If it appears that sediment is exiting the site, silt sacks shall be installed in all catch basins adjacent to the site. Sediment accumulation on all adjacent catch basin inlets shall be removed and the silt sack replaced if torn or damaged.
13. Install stone check dams on-site during construction as needed. Refer to the erosion control details. Temporary sediment basins combined with stone check dams shall be installed on-site during construction to control and collect runoff from upland areas of this site during demolition and construction activities.
14. The contractor shall comply with the Sedimentation and Erosion Control Notes as shown on the Site Development Plans and Specifications.
15. The stabilized construction entrances shall be inspected weekly and records of inspections kept. The entrances shall be maintained by adding additional clean, angular, durable stone to remove the soil from the construction vehicle's tires when exiting the site. If soil is still leaving the site via the construction vehicle tires, adjacent roadways shall be kept clean by street sweeping.
16. Dust pollution shall be controlled using on-site water trucks and/or an approved soil stabilization product.
17. During demolition and construction activities, Status Reports on compliance with this O&M Document shall be submitted weekly. The report shall document any deficiencies and corrective actions taken by the applicant.

Long-Term Pollution Prevention Plan

Standard #4 from the MassDEP Stormwater Management Handbook requires that a Long-Term Pollution Prevention Plan (LTPPP) be prepared and incorporated as part of the Operation and Maintenance Plan of the Stormwater Management System. The purpose of the LTPPP is to identify potential sources of pollution that may affect the quality of stormwater discharges, and to describe the implementation of practices to reduce the pollutants in stormwater discharges. The following items describe the source control and proper procedures of the LTPPP.



- Housekeeping

The existing development has been designed to maintain a high level of water quality treatment for all stormwater discharge to the wetland areas. An Operation and Maintenance (O&M) plan has been prepared and is included in this section of the report. The owner (or its designee) is responsible for adherence to the O&M plan in a strict and complete manner.

- Storing of Materials & Water Products

The trash and waste program for the site includes exterior dumpsters. There is a trash contractor used to pick up the waste material in the dumpsters. The stormwater drainage system has water quality inlets designed to capture trash and debris.

- Vehicle Washing

Outdoor vehicle washing has the potential to result in high loads of nutrients, metals, and hydrocarbons during dry weather conditions, as the detergent-rich water used to wash the grime off the vehicle enters the stormwater drainage system. The existing development does not include any designated vehicle washing areas, nor is it expected that any vehicle washing will take place on-site.

- Spill Prevention & Response

Sources of potential spill hazards include vehicle fluids, liquid fuels, pesticides, paints, solvents, and liquid cleaning products. The majority of the spill hazards would likely occur within the buildings and would not enter the stormwater drainage system. However, there are spill hazards from vehicle fluids or liquid fuels located outside of the buildings. These exterior spill hazards have the potential to enter the stormwater drainage system and are to be addressed as follows:

1. Spill hazards of pesticides, paints, and solvents shall be remediated using the Manufacturers' recommended spill cleanup protocol.
2. Vehicle fluids and liquid fuel spill shall be remediated according to the local and state regulations governing fuel spills.
3. The owner shall have the following equipment and materials on hand to address a spill clean-up: brooms, dust pans, mops, rags, gloves, absorptive material, sand, sawdust, plastic and metal trash containers.
4. All spills shall be cleaned up immediately after discovery.
5. Spills of toxic or hazardous material shall be reported, regardless of size, to the Massachusetts Department of Environmental Protection at (888) 304-1333.



6. Should a spill occur, the pollution prevention plan will be adjusted to include measures to prevent another spill of a similar nature. A description of the spill, along with the causes and cleanup measures will be included in the updated pollution prevention plan.

- Maintenance of Lawns, Gardens, and Other Landscaped Areas

It should be recognized that this is a general guideline towards achieving high quality and well-groomed landscaped areas. The grounds staff/landscape contractor must recognize the shortcomings of a general maintenance plan such as this, and modify and/or augment it based on weekly, monthly, and yearly observations. In order to assure the highest quality conditions, the staff must also recognize and appreciate the need to be aware of the constantly changing conditions of the landscaping and be able to respond to them on a proactive basis. No trees shall be planted over the drain lines or recharge area, and that only shallow rooted plants and shrubs will be allowed.

- o Fertilizer

Maintenance practices should be aimed at reducing environmental, mechanical and pest stresses to promote healthy and vigorous growth. When necessary, pest outbreaks should be treated with the most sensitive control measure available. Synthetic chemical controls should be used only as a last resort to organic and biological control methods. Fertilizer, synthetic chemical controls and pest management applications (when necessary) shall be performed only by licensed applicators in accordance with the manufacturer's label instructions when environmental conditions are conducive to controlled product application.

Only slow-release organic fertilizers should be used in the planting and mulch areas to limit the amount of nutrients that could enter downstream resource areas. Fertilization of the planting and mulch areas will be performed within manufacturers labeling instructions and shall not exceed an NPK ration of 1:1:1 (i.e. Triple 10 fertilizer mix), considered a low nitrogen mixture. Fertilizers approved for the use under this O&M Plan are as follows:

Type:	LESCO® 28-0-12 (Lawn Fertilizer)
	MERIT® 0.2 Plus Turf Fertilizer
	MOMENTUM™ Force Weed & Feed

- o Suggested Aeration Program

In-season aeration of lawn areas is good cultural practice, and is recommended whenever feasible. It should be accomplished with a solid thin tine aeration method to reduce disruption to the use of the area. The



depth of solid tine aeration is similar to core type, but should be performed when the soil is somewhat drier for a greater overall effect.

Depending on the intensity of use, it can be expected that all landscaped lawn areas will need aeration to reduce compaction at least once per year. The first operation should occur in late May following the spring season. Methods of reducing compaction will vary based on the nature of the compaction. Compaction on newly established landscaped areas is generally limited to the top 2-3" and can be alleviated using hollow core or thin tine aeration methods.

The spring aeration should consist of two passes at opposite directions with 1/4" hollow core tines penetrating 3-5" into the soil profile. Aeration should occur when the soil is moist but not saturated. The soil cores should be shattered in place and dragged or swept back into the turf to control thatch. If desired the cores may also be removed and the area top-dressed with sand or sandy loam. If the area drains on average too slowly, the topdressing should contain a higher percentage of sand. If it is draining on average too quickly, the top dressing should contain a higher percentage of soil and organic matter.

o Landscape Maintenance Program Practices:

▪ Lawn

1. Mow a minimum of once a week in spring, to a height of 2" to 2 1/2" high. Mowing should be frequent enough so that no more than 1/3 of grass blade is removed at each mowing. The top growth supports the roots; the shorter the grass is cut, the less the roots will grow. Short cutting also dries out the soil and encourages weeds to germinate.
2. Mow approximately once every two weeks from July 1st to August 15th depending on lawn growth.
3. Mow on a ten-day cycle in fall, when growth is stimulated by cooler nights and increased moisture.
4. Do not remove grass clippings after mowing.
5. Keep mower blades sharp to prevent ragged cuts on grass leaves, which cause a brownish appearance and increase the chance for disease to enter a leaf.

▪ Shrubs

1. Mulch not more than 3" depth with shredded pine or fir bark.



2. Hand prune annually, immediately after blooming, to remove 1/3 of the above-ground biomass (older stems). Stem removals are to occur within 6" of the ground to open up shrub and maintain two-year wood (the blooming wood).
 3. Hand-prune evergreen shrubs only as needed to remove dead and damaged wood and to maintain the naturalistic form of the shrub. Never mechanically shear evergreen shrubs.
- Trees
 1. Provide aftercare of new tree plantings for the first three years.
 2. Do not fertilize trees, it artificially stimulates them (unless tree health warrants).
 3. Water once a week for the first year; twice a month for the second; once a month for the third year.
 4. Prune trees on a four-year cycle.
 - Invasive Species
 1. Inform the Conservation Commission Agent prior to the removal of invasive species proposed either through hand work or through chemical removal.
 - Storage and Use of Herbicides and Pesticides

Integrated Pest Management is the combination of all methods (of pest control) which may prevent, reduce, suppress, eliminate, or repel an insect population. The main requirements necessary to support any pest population are food, shelter and water, and any upset of the balance of these will assist in controlling a pest population. Scientific pest management is the knowledgeable use of all pest control methods (sanitation, mechanical, chemical) to benefit mankind's health, welfare, comfort, property and food. A Pest Management Professional (PMP) should be retained who is licensed with the Commonwealth of Massachusetts Executive Office of Energy and Environmental Affairs, Department of Agricultural Resources.

The site manager will be provided with approved bulletin before entering into or renewing an agreement to apply pesticides for the control of indoor household or structural pests, refer to 333 CMR 13.08.

Before beginning each application, the applicator must post a Department approved notice on all of the entrances to the treated room or area. The applicator must leave such notices posted after the application. The notice will be posted at conspicuous point(s) of access to the area treated. The location and number of



signs will be determined by the configuration of the area to be treated based on the applicator's best judgment. It is intended to give sufficient notice so that no one comes into an area being treated unaware that the applicator is working and pesticides are being applied. However, if the contracting entity does not want the signs posted, he/she may sign a Department approved waiver indicating this.

The applicator or employer will provide to any person upon their request the following information on previously conducted applications:

1. Name and phone number of pest control company;
 2. Date and time of the application;
 3. Name and license number of the applicator;
 4. Target pests; and
 5. Name and EPA Registration Number of pesticide products applied.
- Pet Waste Management
The owner's landscape crew (or designee) shall remove any obvious pet waste that has been left behind by pet owners within the development. The pet waste shall be disposed of in accordance with local and state regulations.
 - Operations and Management of Septic Systems
There are no proposed septic systems within the limits of the project.
 - Management of Deicing Chemicals and Snow
Snow will be stockpiled on site until the accumulated snow becomes a hazard to the daily operations of the site. It will be the responsibility of the snow removal contractor to properly dispose of transported snow according to MassDEP, Bureau of Resource Protection – Snow Disposal Guideline #BRPG01-01, governing the proper disposal of snow. It will be the responsibility of the snow removal contractor to follow these guidelines and all applicable laws and regulations

The owner's maintenance staff (or its designee) will be responsible for the clearing of the sidewalk and building entrances. The owner may be required to use a de-icing agent such as potassium chloride to maintain a safe walking surface. If used, the de-icing agent for the walkways and building entrances will be kept within the storage rooms located within the building. If used, de-icing agents will not be stored outside. The owner's maintenance staff will limit the application of sand.

Long-Term Maintenance Plan – Facilities Description

A maintenance log will be kept (i.e. report) summarizing inspections, maintenance, and any corrective actions taken. The log will include the date on which each inspection or maintenance task was performed, a description of the inspection findings or maintenance completed, and the name of the inspector or maintenance personnel performing the task. If a maintenance task requires the clean-out of any sediments or debris, the location



where the sediment and debris was disposed after removal will be indicated. The log will be made accessible to department staff and a copy provided to the department upon request.

The following is a description of the Stormwater Management System for the project site.

Stormwater Collection System – On-Site:

The stormwater collection system is a series of inlets located at low points within the limits of the paved area. All of the proposed on-site catch basins incorporate a deep sump and hooded outlet. The catch basins are connected by a closed gravity pipe network that pass through proprietary separators prior to entering the underground infiltration systems or infiltration basins.

Structural Pretreatment BMPs: Regular maintenance of these BMPs is especially critical because they typically receive the highest concentration of suspended solids during the first flush of a storm event.

Deep Sump Catch Basins:

Deep sump catch basins, also known as oil and grease or hooded catch basins, are underground retention systems designed to remove trash, debris, and coarse sediment from stormwater runoff, and serve as temporary spill containment devices for floatables such as oils and greases.

Regular maintenance is essential. Deep sump catch basins remain effective by removing pollutants only if they are cleaned out frequently. One study found that once 50% of the sump volume is filled, the catch basin is not able to retain additional sediments.

Inspect or clean deep sump catch basins at least four times per year and at the end of the foliage and snow-removal seasons. Sediments must also be removed four times per year or whenever the depths of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin.

Clamshell buckets are typically used to remove sediment in Massachusetts. However, vacuum trucks are preferable, because they remove more trapped sediment and supernatant than clamshells. Vacuuming is also a speedier process and is less likely to snap the cast iron hood within the deep sump catch basin.

Always consider the safety of the staff cleaning deep sump catch basins. Cleaning a deep sump catch basin within a road with active traffic or even within a parking lot is dangerous, and a police detail may be necessary to safeguard workers.

Although catch basin debris often contains concentrations of oil and hazardous materials, such as petroleum hydrocarbons and metals, MassDEP classifies them as solid waste. Unless there is evidence that they have been contaminated by a spill



or other means, MassDEP does not routinely require catch basin cleanings to be tested before disposal. Contaminated catch basin cleanings must be evaluated in accordance with the Hazardous Waste Regulations, 310 CMR 30.000, and handled as hazardous waste.

In the absence of evidence of contamination, catch basin cleanings may be taken to a landfill or other facility permitted by MassDEP to accept solid waste, without any prior approval by MassDEP. However, some landfills require catch basin cleanings to be tested before they are accepted.

With prior MassDEP approval, catch basin cleanings may be used as grading and shaping materials at landfills undergoing closure (see Revised Guidelines for Determining Closure Activities at Inactive Unlined Landfill Sites) or as daily cover at active landfills. MassDEP also encourages the beneficial reuse of catch basin cleanings whenever possible. A Beneficial Reuse Determination is required for such use.

MassDEP regulations prohibit landfills from accepting materials that contain free-draining liquids. One way to remove liquids is to use a hydraulic lift truck during cleaning operations so that the material can be decanted at the site. After loading material from several catch basins into a truck, elevate the truck so that any free-draining liquid can flow back into the structure. If there is no free water in the truck, the material may be deemed to be sufficiently dry. Otherwise catch basin cleanings must undergo a Paint Filter Liquids Test. Go to www.Mass.gov/dep/recycle/laws/cafacts.doc for information on all of the MassDEP requirements pertaining to the disposal of catch basin cleanings.

Infiltration BMPs:

Subsurface Structures:

Subsurface structures are underground systems that capture runoff, and gradually infiltrate it into the groundwater through rock and gravel.

Because subsurface structures are installed underground, they are extremely difficult to maintain. Inspect inlets at least twice a year. Remove any debris that might clog the system. Include mosquito controls in the Operation and Maintenance Plan.

Inspect outlet from subsurface structures to adjacent resource area for signs of scour and sediment accumulation at least twice annually. Remove sediment accumulation and add rip rap as necessary to prevent scour.



Other BMPs and Accessories:

Outlet Structures:

Outlets of BMPs are devices that control the flow of stormwater out of the BMP to the conveyance system.

Inspect outlet structures twice per year. Remove any accumulated sediment and debris that could prevent flow at the outlet structure.

Vegetated Areas:

Inspect slopes and embankments early in the growing season to identify active or potential erosion problems. Replant bare areas or areas with sparse growth. Where rill erosion is evident, armor the area with an appropriate lining or divert the erosive flows to on-site areas able to withstand the concentrated flows.

Roadway and Parking Surfaces:

Clear accumulations of winter sand in parking lots and along roadways at least once a year, preferably in the spring. Accumulations on pavement may be removed by pavement sweeping. Accumulations of sand along road shoulders may be removed by grading excess sand to the pavement edge and removing it manually or by a front-end loader.

Other Maintenance Activity:

- Mosquito Control - Both above ground and underground stormwater BMPs have the potential to serve as mosquito breeding areas. Good design, proper operation and maintenance, and treatment with larvicides can minimize this potential. See the supplemental information for Mosquito Control in Stormwater Management Practices, and the Operation and Maintenance Plan Schedule for inspection schedule.
- Street Sweeping - Clear accumulations of winter sand in parking lots and along roadways at least once a year, preferably in the spring. Accumulations on pavement may be removed by pavement sweeping. Accumulations of sand along road shoulders may be removed by grading excess sand to the pavement edge and removing it manually or by a front-end loader.

Inspection and Maintenance Frequency and Corrective Measures

In accordance with MA DEP Stormwater Handbook: Volume 2, Chapter 2; the previously described BMPs will be inspected and the identified deficiencies will be corrected. Clean-out must include the removal and legal disposal of any accumulated sediments, trash, and debris. In any and all cases, operations, inspections, and maintenance activities shall utilize best practical measures to avoid and minimize impacts to wetland resource areas outside the footprint of the SMS.



Supplemental Information

- Operation & Maintenance Plan Schedule
- Massachusetts Stormwater Handbook, Chapter 5, Miscellaneous Stormwater Topics, Mosquito Control in Stormwater Management Practices.

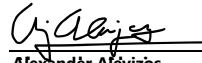
OPERATION AND MAINTENANCE PLAN SCHEDULE

Date: 7/21/2025



Project: 40B Multi-Family Site Development
Project Address: 444 East Central Street Fanklin, MA

Responsible for O&M Plan: TAG Central LLC
Address: 275 Regatta Drive, Jupiter, FL 33477
Phone: (561) 685-5336


Alexander Alévizos

Principal

All information within table is derived from Massachusetts Stormwater Handbook: Volume 2, Chapter 2

BMP CATEGORY	BMP OR MAINTENANCE ACTIVITY	SCHEDULE/FREQUENCY	NOTES	ESTIMATED ANNUAL MAINTENANCE COST	INSPECTION PERFORMED	
					DATE:	BY:
STRUCTURAL PRETREATMENT BMPs	DEEP SUMP CATCH BASIN	Four times per year (quarterly).	Inspect and clean catch basin units whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the basin.	\$1,000		
	PROPRIETARY SEPARATORS	In accordance with manufacturers requirements, but no less than twice a year following installation and once a year thereafter.	Remove sediment and other trapped pollutants at frequency or level specified by manufacturer.	\$2,000		
	GRAVEL TRENCH DRAIN	Inspect structure inlets at least twice a year. Remove debris that may clog the system as needed..	Observed trash and debris will removed at each inspection. Sediment will be removed as necessary.	\$750		
INFILTRATION BMPs	SUBSURFACE STRUCTURES	Inspect structure inlets at least twice a year. Remove debris that may clog the system as needed.	Because subsurface structures are installed underground, they are extremely difficult to maintain. Remove any debris that might clog the system.	\$500		
BMP ACCESSORIES	OUTLET STRUCTURES	Periodic cleaning of Outlet Control Structures as needed.	Clear trash and debris as necessary.	\$500		
OTHER MAINTENANCE ACTIVITY	MOSQUITO CONTROL	Inspect BMPs as needed to ensure the system's drainage time is less than the maximum 72 hour period.	Massachusetts stormwater handbook requires all stormwater practices that are designed to drain do so within 72 hours to reduce the number of mosquitos that mature to adults since the aquatic stage of a mosquito is 7-10 days.	\$100		
	SNOW STORAGE	Clear and remove snow to approved storage locations as necessary to ensure systems are working properly and are protected from meltwater pollutants.	Carefully select snow disposal sites before winter. Avoid dumping removed snow over catch basins, or in detention ponds, sediment forebays, rivers, wetlands, and flood plains. It is also prohibited to dump snow in the bioretention basins or gravel swales.	\$500		
	STREET SWEEPING	Clear accumulations of winter sand in parking lots and along roadways at least once a year, preferably in the spring.	Sweep, power broom or vacuum paved areas. Submit information that confirms that all street sweepings have been completed in accordance with state and local requirements	\$2,000		

Chapter 5 Miscellaneous Stormwater Topics

Mosquito Control in Stormwater Management Practices

Both aboveground and underground stormwater BMPs have the potential to serve as mosquito breeding areas. Good design, proper operation and maintenance and treatment with larvicides can minimize this potential.

EPA recommends that stormwater treatment practices dewater within 3 days (72 hours) to reduce the number of mosquitoes that mature to adults, since the aquatic stage of many mosquito species is 7 to 10 days. Massachusetts has had a 72-hour dewatering rule in its Stormwater Management Standards since 1996. The 2008 technical specifications for BMPs set forth in Volume 2, Chapter 2 of the Massachusetts Stormwater Handbook also concur with this practice by requiring that all stormwater practices designed to drain do so within 72 hours.

Some stormwater practices are designed to include permanent wet pools. These practices – if maintained properly – can limit mosquito breeding by providing habitat for mosquito predators. Additional measures that can be taken to reduce mosquito populations include increasing water circulation, attracting mosquito predators by adding suitable habitat, and applying larvicides.

The Massachusetts State Reclamation and Mosquito Control Board (SRMCB), through the Massachusetts Mosquito Control Districts, can undertake further mosquito control actions specifically for the purpose of mosquito control pursuant to Massachusetts General Law Chapter 252. The Mosquito Control Board, <http://www.mass.gov/agr/mosquito/>, describes mosquito control methods and is in the process of developing guidance documents that describe Best Management Practices for mosquito control projects.

The SRMCB and Mosquito Control Districts are not responsible for operating and maintaining stormwater BMPs to reduce mosquito populations. The owners of property that construct the stormwater BMPs or municipalities that “accept” them through local subdivision approval are responsible for their maintenance.¹ The SRMCB is composed of officials from MassDEP, Department of Agricultural Resources, and Department of Conservation and Recreation. The nine (9) Mosquito Control Districts overseen by the SRMCB are located throughout Massachusetts, covering 176 municipalities.

Construction Period Best Management Practices for Mosquito Control

To minimize mosquito breeding during construction, it is essential that the following actions be taken to minimize the creation of standing pools by taking the following actions:

- **Minimize Land Disturbance:** Minimizing land disturbance reduces the likelihood of mosquito breeding by reducing silt in runoff that will cause construction period controls to clog and retain standing pools of water for more than 72 hours.
- **Catch Basin inlets:** Inspect and refresh filter fabric, hay bales, filter socks or stone dams on a regular basis to ensure that any stormwater ponded at the inlet drains within 8 hours after precipitation stops. Shorter periods may be necessary to avoid hydroplaning in roads

¹ MassDEP and MassHighway understand that the numerous stormwater BMPs along state highways pose a unique challenge. To address this challenge, the 2004 MassHighway Stormwater Handbook will provide additional information on appropriate operation and maintenance practices for mosquito control when the Handbook is revised to reflect the 2008 changes to the Stormwater Management Standards..

caused by water ponded at the catch basin inlet. Treat catch basin sumps with larvicides such as *Bacillus sphaericus* (Bs) using a licensed pesticide applicator.

- **Check Dams:** If temporary check dams are used during the construction period to lag peak rate of runoff or pond runoff for exfiltration, inspect and repair the check dams on a regular basis to ensure that any stormwater ponded behind the check dam drains within 72 hours.
- **Design construction period sediment traps** to dewater within 72 hours after precipitation. Because these traps are subject to high silt loads and tend to clog, treat them with the larvicide Bs after it rains from June through October, until the first frost occurs.
- **Construction period open conveyances:** When temporary manmade ditches are used for channelizing construction period runoff, inspect them on a regular basis to remove any accumulated sediment to restore flow capacity to the temporary ditch.
- **Revegetating Disturbed Surfaces:** Revegetating disturbed surfaces reduces sediment in runoff that will cause construction period controls to clog and retain standing pools of water for greater than 72 hours.
- **Sediment fences/hay bale barriers:** When inspections find standing pools of water beyond the 24-hour period after a storm, take action to restore barrier to its normal function.

Post-Construction Stormwater Treatment Practices

- Mosquito control begins with the environmentally sensitive site design. Environmentally sensitive site design that minimizes impervious surfaces reduces the amount of stormwater runoff. Disconnecting runoff using the LID Site Design credits outlined in the Massachusetts Stormwater Handbook reduces the amount of stormwater that must be conveyed to a treatment practice. Utilizing green roofs minimizes runoff from smaller storms. Storage media must be designed to dewater within 72 hours after precipitation.
- Mosquito control continues with the selection of structural stormwater BMPs that are unlikely to become breeding grounds for mosquitoes, such as:
 - **Bioretention Areas/Rain Gardens/Sand Filter:** These practices tend not to result in mosquito breeding. If any level spreaders, weirs or sediment forebays are used as part of the design, inspect them and correct them as necessary to prevent standing pools of water for more than 72 hours.
 - **Infiltration Trenches:** This practice tends not to result in mosquito breeding. If any level spreaders, weirs, or sediment forebays are used as part of the design, inspect them and correct them as necessary to prevent standing pools of water for more than 72 hours.
- Another mosquito control strategy is to select BMPs that can become habitats for mosquito predators, such as:
 - **Constructed Stormwater Wetlands:** Habitat features can be incorporated in constructed stormwater wetlands to attract dragonflies, amphibians, turtles, birds, bats, and other natural predators of mosquitoes.
 - **Wet Basins:** Wet basins can be designed to incorporate fish habitat features, such as deep pools. Introduce fish in consultation with Massachusetts Division of Fisheries and Wildlife. Vegetation within wet basins designed as fish habitat must be properly managed to ensure that vegetation does not overtake the habitat. Proper design to ensure that no low circulation or “dead” zones are created may reduce the potential for mosquito breeding. Introducing bubblers may increase water circulation in the wet basin.

Effective mosquito controls require proponents to design structural BMPs to prevent ponding and facilitate maintenance and, if necessary, the application of larvicides. Examples of such design practices include the following:

- **Basins:** Provide perimeter access around wet basins, extended dry detention basins and dry detention basins for both larviciding and routine maintenance. Control vegetation to ensure that access pathways stay open.
- **BMPs without a permanent pool of water:** All structural BMPs that do not rely on a permanent pool of water must drain and completely dewater within 72 hours after precipitation. This includes dry detention basins, extended dry detention basins, infiltration basins, and dry water quality swales. Use underdrains at extended dry detention basins to drain the small pools that form due to accumulation of silts. Wallace indicates that extended dry extended detention basins may breed more mosquitoes than wet basins. It is, therefore, imperative to design outlets from extended dry detention basins to completely dewater within the 72-hour period.
- **Energy Dissipators and Flow Spreaders:** Currier and Moeller, 2000 indicate that shallow recesses in energy dissipators and flow spreaders trap water where mosquitoes breed. Set the riprap in grout to reduce the shallow recesses and minimize mosquito breeding.
- **Outlet control structures:** Debris trapped in small orifices or on trash racks of outlet control structures such as multiple stage outlet risers may clog the orifices or the trash rack, causing a standing pool of water. Optimize the orifice size or trash rack mesh size to provide required peak rate attenuation/water quality detention/retention time while minimizing clogging.
- **Rain Barrels and Cisterns:** Seal lids to reduce the likelihood of mosquitoes laying eggs in standing water. Install mosquito netting over inlets. The cistern system should be designed to ensure that all collected water is drained into it within 72 hours.
- **Subsurface Structures, Deep Sump Catch Basins, Oil Grit Separators, and Leaching Catch Basins:** Seal all manhole covers to reduce likelihood of mosquitoes laying eggs in standing water. Install mosquito netting over the outlet (CALTRANS 2004).

The Operation and Maintenance Plan should provide for mosquito prevention and control.

- **Check dams:** Inspect permanent check dams on the schedule set forth in the O&M Plan. Inspect check dams 72 hours after storms for standing water ponding behind the dam. Take corrective action if standing water is found.
- **Cisterns:** Apply *Bs* larvicide in the cistern if any evidence of mosquitoes is found. The Operation and Maintenance Plan shall specify how often larvicides should be applied to waters in the cistern.
- **Water quality swales:** Remove and properly dispose of any accumulated sediment as scheduled in the Operation and Maintenance Plan.
- **Larvicide Treatment:** The Operation and Maintenance Plan must include measures to minimize mosquito breeding, including larviciding.
- The party identified in the Operation and Maintenance Plan as responsible for maintenance shall see that larvicides are applied as necessary to the following stormwater treatment practices: catch basins, oil/grit separators, wet basins, wet water quality swales, dry extended detention basins, infiltration basins, and constructed stormwater wetlands. The Operation and Maintenance Plan must ensure that all larvicides are applied by a licensed pesticide applicator and in compliance with all pesticide label requirements.
- The Operation and Maintenance Plan should identify the appropriate larvicide and the time and method of application. For example, *Bacillus sphaericus* (*Bs*), the preferred

larvicide for stormwater BMPs, should be hand-broadcast.² Alternatively, Altosid, a Methopren product, may be used. Because some practices are designed to dewater between storms, such as dry extended detention and infiltration basins, the Operation and Maintenance Plan should provide that larviciding must be conducted during or immediately after wet weather, when the detention or infiltration basin has a standing pool of water, unless a product is used that can withstand extended dry periods.

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² *Bacillus thuringiensis israelensis* or *Bti* is usually applied by helicopter to wetlands and floodplains

Roads and Stormwater BMPs

In general, the stormwater BMPs used for land development projects can also be used for new roadways and roadway improvement projects. However, for improvement of existing roads, there are often constraints that limit the choice of BMP. These constraints derive from the linear configuration of the road, the limited area within the existing right-of-way, the structural and safety requirements attendant to good roadway design, and the long-term maintainability of the roadway drainage systems. The MassHighway Handbook provides strategies for dealing with the constraints associated with providing stormwater BMPs for roadway redevelopment projects.

Roadway design can minimize impacts caused by stormwater. Reducing roadway width reduces the total and peak volume of runoff. Designing a road with country drainage (no road shoulders or curbs) disconnects roadway runoff. Disconnection of roadway runoff is eligible for the Low Impact Site Design Credit provided the drainage is disconnected in accordance with specifications outlined in Volume 3.

Like other parties, municipalities that work within wetlands jurisdictional areas and adjacent buffer zones must design and implement structural stormwater best management practices in accordance with the Stormwater Management Standards and the Stormwater Management Handbook. In addition, in municipalities and areas where state agencies operate stormwater systems, the DPWs (or other town or state agencies) must meet the “good housekeeping” requirement of the municipality’s or agency’s MS4 permit.

MassHighway has taken stormwater management one step further by working with MassDEP to develop the MassHighway Storm Water Handbook for Highways and Bridges. The purpose of the MassHighway Handbook is to provide guidance for persons involved in the design, permitting, review and implementation of state highway projects, especially those involving existing roadways where physical constraints often limit the stormwater management options available. These constraints, like those common to redevelopment sites, may make it difficult to comply precisely with the requirements of the Stormwater Management Standards and the Massachusetts Stormwater Handbook.³ In response to these constraints, MassDEP and MHD developed specific design, permitting, review and implementation practices that meet the unique challenges of providing environmental protection for existing state roads. The information in the MassHighway Handbook may also aid in the planning and design of projects to build new highways and to add lanes to existing highways, since they may face similar difficulties in meeting the requirements of the Stormwater Management Standards.

Although it is very useful, the MassHighway Handbook does not allow MassHighway projects to proceed without individual review and approval by the issuing authority when subject to the Wetlands Protection Act Regulations, 310 CMR 10.00, or the 401 Water Quality Certification Regulations, 314 CMR 9.00. For example, MassHighway must provide a Conservation Commission with a project-specific Operation and Maintenance Plan in accordance with Standard 9 that documents how the project’s post-construction BMPs will be operated and maintained.⁴

³ The 2004 MassHighway Handbook outlines standardized methods for dealing with these constraints as they apply to highway redevelopment projects. MassDEP and MassHighway intend to work together to provide guidance for add a lane projects when the 2004 Handbook is revised to reflect the 2008 changes to the Stormwater Management Standards.

⁴ The general permit for municipal separate storm sewer systems (the MS4 Permit) requires MassHighway to develop and implement procedures for the proper operation and maintenance of stormwater BMPs. To

Some municipalities have asked if the MassHighway Handbook governs municipal road projects. The answer is no.⁵ The MassHighway Handbook was developed in response to the unique problems and challenges arising out of the management of the state highway system. Like other project proponents, cities and towns planning road or other projects in areas subject to jurisdiction under the Wetlands Protection Act must design and implement LID, non-structural and structural best management practices in accordance with the Stormwater Management Standards and the Massachusetts Stormwater Handbook.

avoid duplication of effort, MassHighway may be able rely on the same procedures to fulfill the operation and maintenance requirements of Standard 9 and the MS 4 Permit.

⁵ Although the MassHighway Handbook does not govern municipal road projects, cities and towns may find some of the information presented in the Handbook useful.



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Massachusetts Department of Environmental Protection Bureau of Water Resources Snow Disposal Guidance

Effective Date: December 23, 2019

Applicability: Applies to all federal, state, regional and local agencies, as well as to private businesses.

Supersedes: Bureau of Resource Protection (BRP) Snow Disposal Guideline No. BRPG97-1 issued December 12, 1997 and BRPG01-01 issued March 8, 2001; Bureau of Water Resources (BWR) snow disposal guidance issued December 21, 2015 and December 12, 2018.

Approved by: Kathleen Baskin, Assistant Commissioner, Bureau of Water Resources

PURPOSE: To provide guidelines to all government agencies and private businesses regarding snow disposal site selection, site preparation and maintenance, and emergency snow disposal options that are protective of wetlands, drinking water, and water bodies, and are acceptable to the Massachusetts Department of Environmental Protection (MassDEP), Bureau of Water Resources.

APPLICABILITY: These Guidelines are issued by MassDEP's Bureau of Water Resources on behalf of all Bureau Programs (including Drinking Water Supply, Wetlands and Waterways, Wastewater Management, and Watershed Planning and Permitting). They apply to all federal agencies, state agencies, state authorities, municipal agencies and private businesses disposing of snow in the Commonwealth of Massachusetts.

INTRODUCTION

Finding a place to dispose of collected snow poses a challenge to municipalities and businesses as they clear roads, parking lots, bridges, and sidewalks. While MassDEP is aware of the threats to public safety caused by snow, collected snow that is contaminated with road salt, sand, litter, and automotive pollutants such as oil also threatens public health and the environment.

As snow melts, road salt, sand, litter, and other pollutants are transported into surface water or through the soil where they may eventually reach the groundwater. Road salt and other pollutants can contaminate water supplies and are toxic to aquatic life at certain levels. Sand washed into

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waterbodies can create sand bars or fill in wetlands and ponds, impacting aquatic life, causing flooding, and affecting our use of these resources.

There are several steps that communities can take to minimize the impacts of snow disposal on public health and the environment. These steps will help communities avoid the costs of a contaminated water supply, degraded waterbodies, and flooding. Everything that occurs on the land has the potential to impact the Commonwealth's water resources. Given the authority of local government over the use of the land, municipal officials and staff have a critically important role to play in protecting our water resources.

The purpose of these guidelines is to help federal agencies, state agencies, state authorities, municipalities and businesses select, prepare, and maintain appropriate snow disposal sites before the snow begins to accumulate through the winter. Following these guidelines and obtaining the necessary approvals may also help municipalities in cases when seeking reimbursement for snow disposal costs from the Federal Emergency Management Agency is possible.

RECOMMENDED GUIDELINES

These snow disposal guidelines address: (1) site selection; (2) site preparation and maintenance; and (3) emergency snow disposal.

1. SITE SELECTION

The key to selecting effective snow disposal sites is to locate them adjacent to or on pervious surfaces in upland areas or upland locations on impervious surfaces away from water resources and drinking water wells. At these locations, the snow meltwater can filter into the soil, leaving behind sand and debris which can be removed in the spring. The following conditions should be followed:

- Within water supply Zone A and Zone II, avoid storage or disposal of snow and ice containing deicing chemicals that has been collected from streets located outside these zones. Municipalities may have a water supply protection land use control that prohibits the disposal of snow and ice containing deicing chemicals from outside the Zone A and Zone II, subject to the Massachusetts Drinking Water Regulations at 310 CMR 22.20C and 310 CMR 22.21(2).
- Avoid storage or disposal of snow or ice in Interim Wellhead Protection Areas (IWPA) of public water supply wells, and within 75 feet of a private well, where road salt may contaminate water supplies.
- Avoid dumping snow into any waterbody, including rivers, the ocean, reservoirs, ponds, or wetlands. In addition to water quality impacts and flooding, snow disposed of in open water can cause navigational hazards when it freezes into ice blocks.
- Avoid dumping snow on MassDEP-designated high and medium-yield aquifers where it may contaminate groundwater.
- Avoid dumping snow in sanitary landfills and gravel pits. Snow meltwater will create more contaminated leachate in landfills posing a greater risk to groundwater, and in gravel pits, there is little opportunity for pollutants to be filtered out of the meltwater because groundwater is close to the land surface.

- Avoid disposing of snow on top of storm drain catch basins or in stormwater drainage systems including detention basins, swales or ditches. Snow combined with sand and debris may block a stormwater drainage system, causing localized flooding. A high volume of sand, sediment, and litter released from melting snow also may be quickly transported through the system into surface water.

Recommended Site Selection Procedures

It is important that the municipal Department of Public Works or Highway Department, Conservation Commission, and Board of Health work together to select appropriate snow disposal sites. The following steps should be taken:

- Estimate how much snow disposal capacity may be needed for the season so that an adequate number of disposal sites can be selected and prepared.
- Identify sites that could potentially be used for snow disposal, such as municipal open space (e.g., parking lots or parks).
- Select sites located in upland locations that are not likely to impact sensitive environmental resources first.
- If more storage space is still needed, prioritize the sites with the least environmental impact (using the site selection criteria, and local or MassGIS maps as a guide).

Snow Disposal Mapping Assistance

MassDEP has an online mapping tool to assist in identifying possible locations to potentially dispose of snow. MassDEP encourages municipalities to use this tool to identify possible snow disposal options. The tool identifies wetland resource areas, public drinking water supplies and other sensitive locations where snow should not be disposed. The tool may be accessed through the Internet at the following web address:

<https://maps.env.state.ma.us/dep/arcgis/js/templates/PSE/>.

2. SITE PREPARATION AND MAINTENANCE

In addition to carefully selecting disposal sites before the winter begins, it is important to prepare and maintain these sites to maximize their effectiveness. The following maintenance measures should be undertaken for all snow disposal sites:

- A silt fence or equivalent barrier should be placed securely on the downgradient side of the snow disposal site.
- Wherever possible maintain a 50-foot vegetated buffer between the disposal site and adjacent waterbodies to filter pollutants from the meltwater.
- Clear debris from the site prior to using the site for snow disposal.
- Clear debris from the site and properly dispose of it at the end of the snow season, and no later than May 15.

3. SNOW DISPOSAL APPROVALS

Proper snow disposal may be undertaken through one of the following approval procedures:

- Routine snow disposal – Minimal, if any, administrative review is required in these cases when upland and pervious snow disposal locations or upland locations on impervious surfaces that have functioning and maintained stormwater management systems have been identified, mapped, and used for snow disposal following ordinary snowfalls. Use of upland and pervious snow disposal sites avoids wetland resource areas and allows snow meltwater to recharge groundwater and will help filter pollutants, sand, and other debris. This process will address the majority of snow removal efforts until an entity exhausts all available upland snow disposal sites. The location and mapping of snow disposal sites will help facilitate each entity's routine snow management efforts.
- Emergency Certifications – If an entity demonstrates that there is no remaining capacity at upland snow disposal locations, local conservation commissions may issue an Emergency Certification under the Massachusetts Wetlands Protection regulations to authorize snow disposal in buffer zones to wetlands, certain open water areas, and certain wetland resource areas (i.e. within flood plains). Emergency Certifications can only be issued at the request of a public agency or by order of a public agency for the protection of the health or safety of citizens, and are limited to those activities necessary to abate the emergency. See 310 CMR 10.06(1)-(4). Use the following guidelines in these emergency situations:
 - Dispose of snow in open water with adequate flow and mixing to prevent ice dams from forming.
 - Do not dispose of snow in salt marshes, vegetated wetlands, certified vernal pools, shellfish beds, mudflats, drinking water reservoirs and their tributaries, Zone IIs or IWPA's of public water supply wells, Outstanding Resource Waters, or Areas of Critical Environmental Concern.
 - Do not dispose of snow where trucks may cause shoreline damage or erosion.
 - Consult with the municipal Conservation Commission to ensure that snow disposal in open water complies with local ordinances and bylaws.
- Severe Weather Emergency Declarations – In the event of a large-scale severe weather event, MassDEP may issue a broader Emergency Declaration under the Wetlands Protection Act which allows federal agencies, state agencies, state authorities, municipalities, and businesses greater flexibility in snow disposal practices. Emergency Declarations typically authorize greater snow disposal options while protecting especially sensitive resources such as public drinking water supplies, vernal pools, land containing shellfish, FEMA designated floodways, coastal dunes, and salt marsh. In the event of severe winter storm emergencies, the snow disposal site maps created by municipalities will enable MassDEP and the Massachusetts Emergency Management Agency (MEMA) in helping communities identify appropriate snow disposal locations.

If upland disposal sites have been exhausted, the Emergency Declaration issued by MassDEP allows for snow disposal near water bodies. In these situations, a buffer of at

least 50 feet, preferably vegetated, should still be maintained between the site and the waterbody. Furthermore, it is essential that the other guidelines for preparing and maintaining snow disposal sites be followed to minimize the threat to adjacent waterbodies.

Under extraordinary conditions, when all land-based snow disposal options are exhausted, the Emergency Declaration issued by MassDEP may allow disposal of snow in certain waterbodies under certain conditions. ***A federal agency, state agency, state authority, municipality or business seeking to dispose of snow in a waterbody should take the following steps:***

- Call the emergency contact phone number [(888) 304-1133]] and notify the MEMA of the municipality's intent.
- MEMA will ask for some information about where the requested disposal will take place.
- MEMA will confirm that the disposal is consistent with MassDEP's Severe Weather Emergency Declaration and these guidelines and is therefore approved.

During declared statewide snow emergency events, MassDEP's website will also highlight the emergency contact phone number [(888) 304-1133]] for authorizations and inquiries. For further non-emergency information about this Guidance you may contact your MassDEP Regional Office Service Center:

Northeast Regional Office, Wilmington, 978-694-3246

Southeast Regional Office, Lakeville, 508-946-2714

Central Regional Office, Worcester, 508-792-7650

Western Regional Office, Springfield, 413-755-2114



StormTech SC-800 Specifications

Cumulative Storage Volumes Per Chamber

Assumes 40% Stone Porosity. Calculations are Based Upon a 6" (150 mm) Stone Base Under Chambers.

Depth of Water in System Inches (mm)	Cumulative Chamber Storage ft³ (m³)	Total System Cumulative Storage ft³ (m³)
45 (1143)	↑ 50.62 (1.433)	81.08 (2.296)
44 (1118)	50.62 (1.433)	79.96 (2.264)
43 (1092)	50.62 (1.433)	78.83 (2.232)
42 (1067)	Stone Cover 50.62 (1.433)	77.70 (2.200)
41 (1041)	↓ 50.62 (1.433)	76.57 (2.168)
40 (1016)	50.62 (1.433)	75.44 (2.136)
39 (991)	50.62 (1.433)	74.31 (2.104)
38 (965)	50.55 (1.431)	73.14 (2.071)
37 (940)	50.35 (1.426)	71.90 (2.036)
36 (914)	50.07 (1.418)	70.60 (1.999)
35 (889)	49.56 (1.403)	69.17 (1.959)
34 (864)	48.82 (1.382)	67.60 (1.914)
33 (838)	47.93 (1.357)	65.94 (1.867)
32 (813)	46.91 (1.328)	64.20 (1.818)
31 (787)	45.79 (1.297)	62.40 (1.767)
30 (762)	44.58 (1.262)	60.55 (1.715)
29 (737)	43.28 (1.226)	58.65 (1.661)
28 (711)	41.91 (1.187)	56.70 (1.606)
27 (686)	40.47 (1.146)	54.71 (1.549)
26 (660)	38.96 (1.103)	52.68 (1.492)
25 (635)	37.40 (1.059)	50.61 (1.433)
24 (610)	35.78 (1.013)	48.51 (1.374)
23 (584)	34.10 (0.966)	46.38 (1.313)
22 (559)	32.38 (0.917)	44.22 (1.252)
21 (533)	30.61 (0.867)	42.03 (1.190)
20 (508)	28.80 (0.816)	39.82 (1.128)
19 (483)	26.95 (0.763)	37.58 (1.064)
18 (457)	25.06 (0.710)	35.32 (1.000)
17 (432)	23.13 (0.655)	33.04 (0.936)
16 (406)	21.17 (0.599)	30.74 (0.870)
15 (381)	19.17 (0.543)	28.42 (0.805)
14 (356)	17.14 (0.485)	26.08 (0.739)
13 (330)	15.09 (0.427)	23.72 (0.672)
12 (305)	13.00 (0.368)	21.34 (0.604)
11 (279)	10.89 (0.308)	18.95 (0.537)
10 (254)	8.76 (0.248)	16.54 (0.468)
9 (229)	6.60 (0.187)	14.12 (0.400)
8 (203)	4.42 (0.125)	11.69 (0.331)
7 (178)	2.22 (0.063)	9.24 (0.262)
6 (152)	↑ 0 (0)	6.78 (0.192)
5 (127)	0 (0)	5.65 (0.160)
4 (102)	Stone Foundation 0 (0)	4.52 (0.128)
3 (76)	↓ 0 (0)	3.39 (0.096)
2 (51)	0 (0)	2.26 (0.064)
1 (25)	0 (0)	1.13 (0.032)

Note: Add 1.13 ft³ (0.032 m³) of storage for each additional inch (25 mm) of stone foundation.

ADS StormTech products, manufactured in accordance with ASTM F2418 or ASTM F2922, comply with all requirements in the Build America, Buy America (BABA) Act.

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Storage Volume Per Chamber ft³ (m³)

	Bare Chamber Storage ft³ (m³)	Chamber and Stone Foundation Depth in. (mm)		
		6 (150)	12 (300)	18 (450)
SC-800 Chamber	50.6 (1.43)	81.0 (2.29)	87.8 (2.48)	94.6 (2.6)

Note: Assumes 6" (150 mm) stone above chambers, 6" (150 mm) row spacing and 40% stone porosity.

Amount of Stone Per Chamber

English Tons (yds³)	Stone Foundation Depth		
	6"	12"	18"
SC-800	3.9 (2.8)	4.8 (3.4)	5.7 (4.1)
Metric Kilograms (m³)	150 mm	300 mm	450 mm
SC-800	3580 (2.2)	4380 (2.6)	5170 (3.1)

Note: Assumes 6" (150 mm) of stone above and between chambers.

Volume Excavation Per Chamber yd³ (m³)

	Stone Foundation Depth		
	6" (150 mm)	12" (300 mm)	18" (450 mm)
SC-800	5.6 (4.3)	6.3 (4.8)	6.9 (5.3)

Note: Assumes 6" (150 mm) of row separation and 15" (375 mm) of cover. The volume of excavation will vary as depth of cover increases.

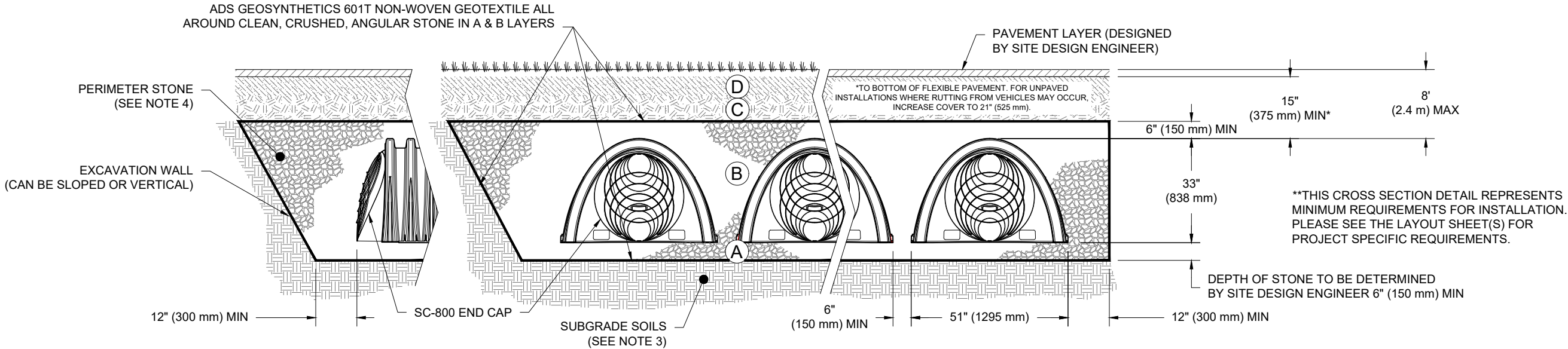
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ACCEPTABLE FILL MATERIALS: STORMTECH SC-800 CHAMBER SYSTEMS

MATERIAL LOCATION		DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	FINAL FILL: FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER.	ANY SOIL/ROCK MATERIALS, NATIVE SOILS, OR PER ENGINEER'S PLANS. CHECK PLANS FOR PAVEMENT SUBGRADE REQUIREMENTS.	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS.
C	INITIAL FILL: FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE ('B' LAYER) TO 15" (375 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER.	GRANULAR WELL-GRADED SOIL/AGGREGATE MIXTURES, <35% FINES OR PROCESSED AGGREGATE. MOST PAVEMENT SUBBASE MATERIALS CAN BE USED IN LIEU OF THIS LAYER.	AASHTO M145 ¹ A-1, A-2-4, A-3 OR AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGIN COMPACTIONS AFTER 12" (300 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 6" (150 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. ROLLER GROSS VEHICLE WEIGHT NOT TO EXCEED 12,000 lbs (53 kN). DYNAMIC FORCE NOT TO EXCEED 20,000 lbs (89 kN).
B	EMBEDMENT STONE: FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE.	CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE ⁵	AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57	NO COMPACTION REQUIRED.
A	FOUNDATION STONE: FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	CLEAN, CRUSHED, ANGULAR STONE OR RECYCLED CONCRETE ⁵	AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE. ^{2,3}

PLEASE NOTE:

1. THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
2. STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 6" (150 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
3. WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.
4. ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.
5. WHERE RECYCLED CONCRETE IS USED IN LAYERS 'A' OR 'B' THE MATERIAL SHOULD ALSO MEET THE ACCEPTABILITY CRITERIA OUTLINED IN TECHNICAL NOTE 6.20 "RECYCLED CONCRETE STRUCTURAL BACKFILL".



NOTES:

1. CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
2. SC-800 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
3. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS.
4. PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
5. REQUIREMENTS FOR HANDLING AND INSTALLATION:
 - TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS.
 - TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 2".
 - TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT AS DEFINED IN SECTION 6.2.8 OF ASTM F2418 SHALL BE GREATER THAN OR EQUAL TO 550 LBS/FT/%. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.

STANDARD CROSS SECTION

SC-800 CHAMBER

DATE: 01/10/24

DRAWN: JLM

CHECKED: JLM

PROJECT #:

DESCRIPTION

DATE

DRWN

CHKD

StormTech®

Chamber System

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4640 TRUEMAN BLVD

HILLIARD, OH 43026

ADS

1

SHEET

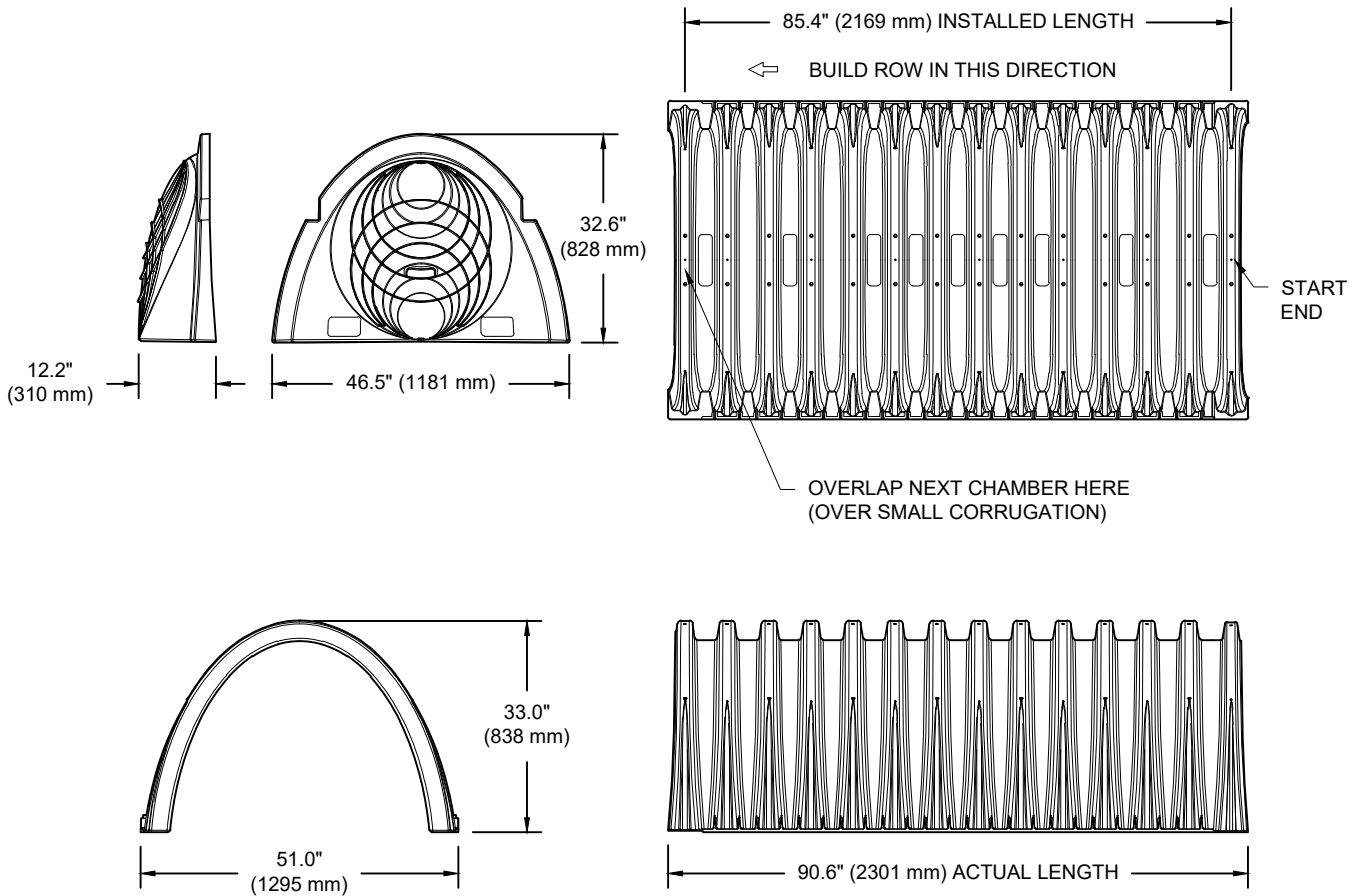
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SC-800 TECHNICAL SPECIFICATION

NTS



NOMINAL CHAMBER SPECIFICATIONS

SIZE (W X H X INSTALLED LENGTH)

CHAMBER STORAGE

MINIMUM INSTALLED STORAGE*

WEIGHT

51.0" X 33.0" X 85.4"

50.6 CUBIC FEET

81.0 CUBIC FEET

81.8 lbs.

(1295 mm X 838 mm X 2169 mm)

(1.43 m³)

(2.29 m³)

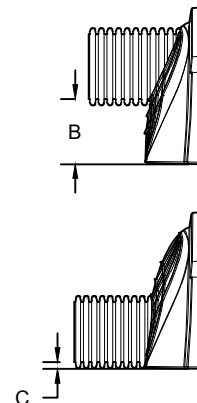
(37.1 kg)

*ASSUMES 6" (152 mm) STONE ABOVE, BELOW, AND BETWEEN CHAMBERS

PRE-CORED HOLES AT BOTTOM OF END CAP FOR PART NUMBERS ENDING WITH "B"

PRE-CORED HOLES AT TOP OF END CAP FOR PART NUMBERS ENDING WITH "T"

PART #	STUB	B	C
SC800EPE06TPC	6" (150 mm)	21.4" (544 mm)	---
SC800EPE06BPC		---	0.9" (23 mm)
SC800EPE08TPC		19.2" (488 mm)	---
SC800EPE08BPC	8" (200 mm)	---	1.0" (25 mm)
SC800EPE10TPC		17.0" (432 mm)	---
SC800EPE10BPC	10" (250 mm)	---	1.2" (30 mm)
SC800EPE12TPC		14.4" (366 mm)	---
SC800EPE12BPC	12" (300 mm)	---	1.6" (41 mm)
SC800EPE15TPC		11.3" (287 mm)	---
SC800EPE15BPC	15" (375 mm)	---	1.7" (43 mm)
SC800EPE18TPC		8.0" (203 mm)	---
SC800EPE18BPC	18" (450 mm)	---	2.0" (51 mm)
SC800EPE24BPC		---	2.3" (58 mm)
SC800EPE	NONE	SOLID END CAP	



NOTE: ALL DIMENSIONS ARE NOMINAL

TECHNICAL SPECIFICATIONS

SC-800 CHAMBER

DRAWN: JLM
CHECKED: JLM

DATE: 01/12/24
PROJECT #:

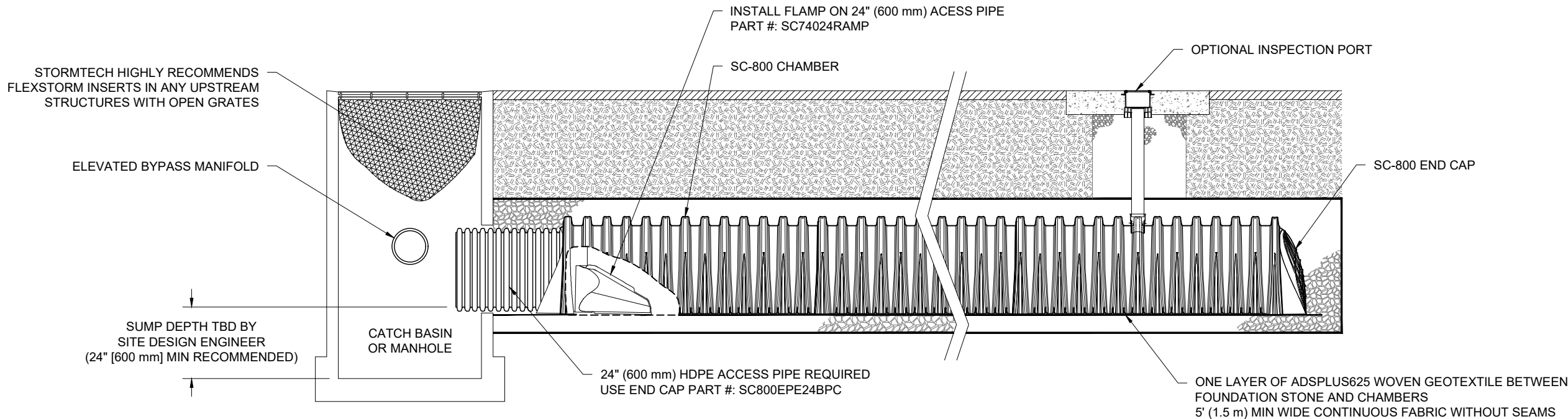
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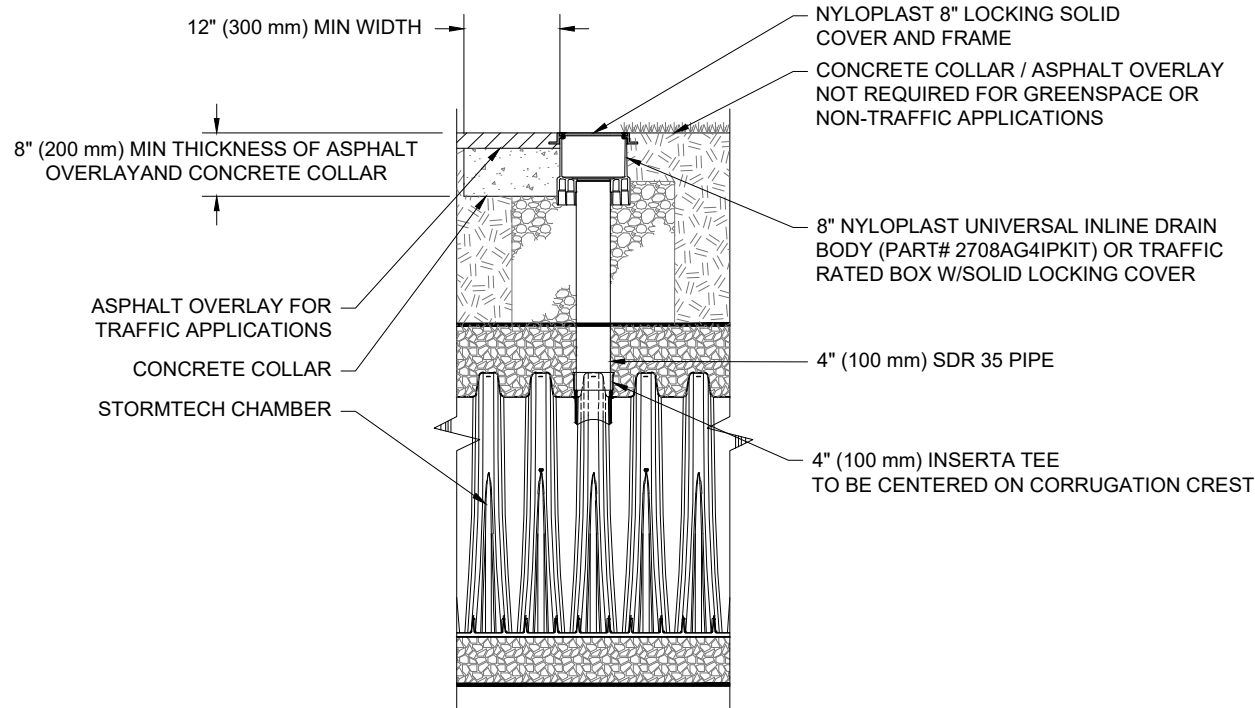
SC-800 ISOLATOR ROW PLUS DETAIL
NTS

INSPECTION & MAINTENANCE

- STEP 1) INSPECT ISOLATOR ROW PLUS FOR SEDIMENT
- A. INSPECTION PORTS (IF PRESENT)
- A.1. REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
- A.2. REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
- A.3. USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
- A.4. LOWER A CAMERA INTO ISOLATOR ROW PLUS FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
- A.5. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- B. ALL ISOLATOR PLUS ROWS
- B.1. REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW PLUS
- B.2. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW PLUS THROUGH OUTLET PIPE
- i) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
- ii) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
- B.3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- STEP 2) CLEAN OUT ISOLATOR ROW PLUS USING THE JETVAC PROCESS
- A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45" (1.1 m) OR MORE IS PREFERRED
- B. APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
- C. VACUUM STRUCTURE SUMP AS REQUIRED
- STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.
- STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

NOTES

- INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
- CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.



NOTE:
INSPECTION PORTS MAY BE CONNECTED THROUGH ANY CHAMBER CORRUGATION CREST.

4" PVC INSPECTION PORT DETAIL
(SC SERIES CHAMBER)
NTS

ISOLATOR ROW PLUS
SC-800 CHAMBER

DATE: 11/14/23
PROJECT #: JLM
DRAWN: JLM
CHECKED: CJD

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Isolator[®] Row Plus

O&M Manual



The Isolator[®] Row Plus

Introduction

An important component of any Stormwater Pollution Prevention Plan is inspection and maintenance. The StormTech Isolator Row Plus is a technique to inexpensively enhance Total Suspended Solids (TSS), Total Phosphorus (TP), Total Petroleum Hydrocarbons (TPH) and Total Nitrogen (TN) removal with easy access for inspection and maintenance.

The Isolator Row Plus

The Isolator Row Plus is a row of StormTech chambers, either SC-160, SC-310, SC-310-3, SC-740, DC-780, SC-800, MC-3500, MC-4500 or MC-7200 models, are lined with filter fabric and connected to a closely located manhole for easy access. The fabric lined chambers provide for sediment settling and filtration as stormwater rises in the Isolator Row Plus and passes through the filter fabric. The open bottom chambers allow stormwater to flow vertically out of the chambers. Sediments are captured in the Isolator Row Plus protecting the adjacent stone and chambers storage areas from sediment accumulation.

ADS Isolator Row and Plus fabric are placed between the stone and the Isolator Row Plus chambers. The woven geotextile provides a media for stormwater filtration, a durable surface for maintenance, prevents scour of the underlying stone and remains intact during high pressure jetting.

The Isolator Row Plus is designed to capture the “first flush” runoff and offers the versatility to be sized on a volume basis or a flow-rate basis. An upstream manhole provides access to the Isolator Row Plus and includes a high/low concept such that stormwater flow rates or volumes that exceed the capacity of the Isolator Row Plus bypass through a manifold to the other chambers. This is achieved with an elevated bypass manifold or a high-flow weir. This creates a differential between the Isolator Row Plus row of chambers and the manifold to the rest of the system, thus allowing for settlement time in the Isolator Row Plus. After Stormwater flows through the Isolator Row Plus and into the rest of the chamber system it is either exfiltrated into the soils below or passed at a controlled rate through an outlet manifold and outlet control structure.

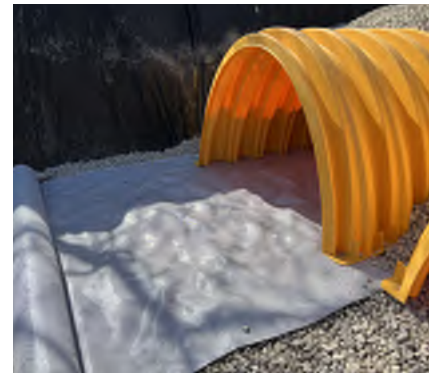
The Isolator Row Plus Flamp[™] is a flared end ramp apparatus attached to the inlet pipe on the inside of the chamber end cap. The FLAMP provides a smooth transition from pipe invert to fabric bottom. It is configured to improve chamber function performance by enhancing outflow of solid debris that would otherwise collect at the chamber's end, or more difficult to remove and require confined space entry into the chamber area. It also serves to improve the fluid and solid flow into the access pipe during maintenance and cleaning and to guide cleaning and inspection equipment back into the inlet pipe when complete.

The Isolator Row Plus may be part of a treatment train system. The treatment train design and pretreatment device selection by the design engineer is often driven by regulatory requirements. Whether pretreatment is used or not, StormTech recommend using the Isolator Row Plus to minimize maintenance requirements and maintenance costs.

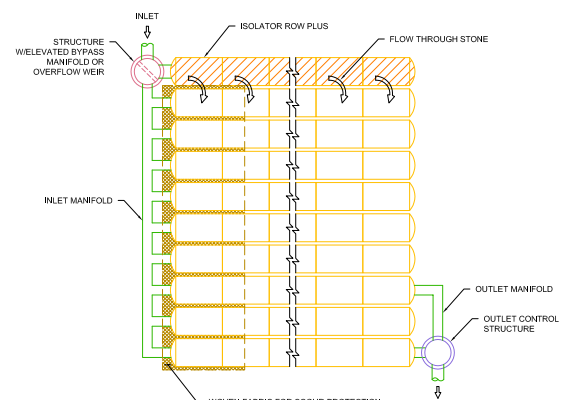
Note: See the StormTech Design Manual for detailed information on designing inlets for a StormTech system, including the Isolator Row Plus.



Looking down the Isolator Row Plus from the manhole opening, ADS Plus Fabric is shown between the chamber and stone base.



StormTech Isolator Row Plus with Overflow Structure (not to scale)



Isolator Row Plus Inspection/Maintenance

Inspection

The frequency of inspection and maintenance varies by location. A routine inspection schedule needs to be established for each individual location based upon site specific variables. The type of land use (i.e. industrial, commercial, residential), anticipated pollutant load, percent imperviousness, climate, etc. all play a critical role in determining the actual frequency of inspection and maintenance practices.

At a minimum, StormTech recommends annual inspections. Initially, the Isolator Row Plus should be inspected every 6 months for the first year of operation. For subsequent years, the inspection should be adjusted based upon previous observation of sediment deposition.

The Isolator Row Plus incorporates a combination of standard manhole(s) and strategically located inspection ports (as needed). The inspection ports allow for easy access to the system from the surface, eliminating the need to perform a confined space entry for inspection purposes.

If upon visual inspection it is found that sediment has accumulated, a stadia rod should be inserted to determine the depth of sediment. When the average depth of sediment exceeds 3" (75 mm) throughout the length of the Isolator Row Plus, clean-out should be performed.

Maintenance

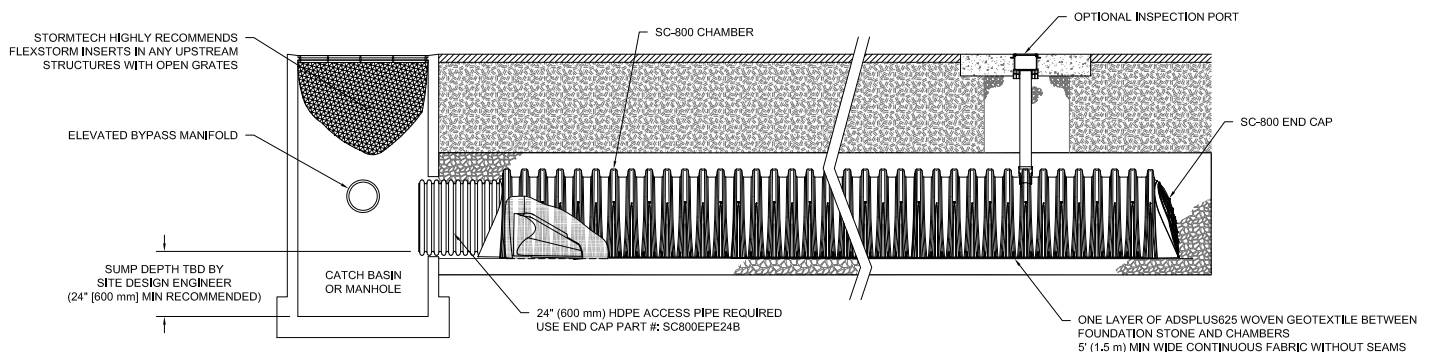
The Isolator Row Plus was designed to reduce the cost of periodic maintenance. By "isolating" sediments to just one row, costs are dramatically reduced by eliminating the need to clean out each row of the entire storage bed. If inspection indicates the potential need for maintenance, access is provided

via a manhole(s) located on the end(s) of the row for cleanout. If entry into the manhole is required, please follow local and OSHA rules for a confined space entry.

Maintenance is accomplished with the JetVac process. The JetVac process utilizes a high pressure water nozzle to propel itself down the Isolator Row Plus while scouring and suspending sediments. As the nozzle is retrieved, the captured pollutants are flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Selection of an appropriate JetVac nozzle will improve maintenance efficiency. Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear facing jets with an effective spread of at least 45" are best. StormTech recommends a maximum nozzle pressure of 2000 psi be utilized during cleaning. JetVac reels can vary in length. For ease of maintenance, ADS recommends Isolator Row Plus lengths up to 200' (61 m). **The JetVac process shall only be performed on StormTech Isolator Row Plus that have ADS Plus Fabric (as specified by StormTech) over their angular base stone.**



StormTech Isolator Row Plus (not to scale)



Isolator Row Plus Step By Step Maintenance Procedures

Step 1

Inspect Isolator Row Plus for sediment.

- A) Inspection ports (if present)
 - i. Remove lid from floor box frame
 - ii. Remove cap from inspection riser
 - iii. Using a flashlight and stadia rod, measure depth of sediment and record results on maintenance log.
 - iv. If sediment is at or above 3 inch depth, proceed to Step 2. If not, proceed to Step 3.
- B) All Isolator Row Plus
 - i. Remove cover from manhole at upstream end of Isolator Row Plus
 - ii. Using a flashlight, inspect down Isolator Row Plus through outlet pipe
 1. Mirrors on poles or cameras may be used to avoid a confined space entry
 2. Follow OSHA regulations for confined space entry if entering manhole
 - iii. If sediment is at or above the lower row of sidewall holes (approximately 3 inches), proceed to Step 2.
 2. If not, proceed to Step 3.

Step 2

Clean out Isolator Row Plus using the JetVac process.

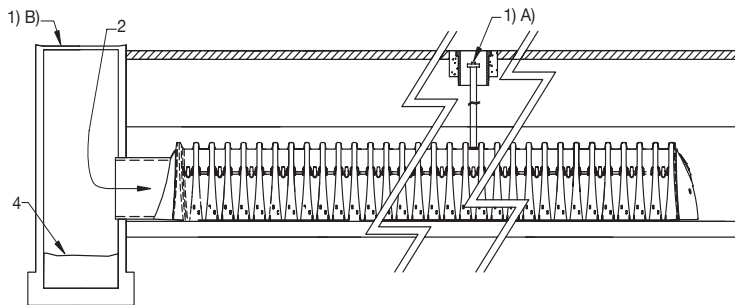
- A) A fixed floor cleaning nozzle with rear facing nozzle spread of 45 inches or more is preferable
- B) Apply multiple passes of JetVac until backflush water is clean
- C) Vacuum manhole sump as required

Step 3

Replace all caps, lids and covers, record observations and actions.

Step 4

Inspect & clean catch basins and manholes upstream of the StormTech system.



Sample Maintenance Log

Date	Stadia Rod Readings		Sediment Depth (1)-(2)	Observations/Actions	Inspector
	Fixed point to chamber bottom (1)	Fixed point to top of sediment (2)			
3/15/11	6.3 ft	none		New installation. Fixed point is CI frame at grade	DJM
9/24/11		6.2	0.1 ft	Some grit felt	SM
6/20/13		5.8	0.5 ft	Mucky feel, debris visible in manhole and in Isolator Row Plus, maintenance due	NV
7/7/13	6.3 ft		0	System jetted and vacuumed	DJM

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StormTech® Installation Guide

SC-310/SC-740/DC-780/SC-800



StormTech
Installation Video

Required Materials and Equipment List

- Acceptable fill materials per Table 1
- ADS Plus and non-woven geotextile fabrics
- StormTech solid end caps and pre-cored end caps
- StormTech chambers
- StormTech manifolds and fittings

Important Notes:

- This installation guide provides the minimum requirements for proper installation of chambers. Non-adherence to this guide may result in damage to chambers during installation. Replacement of damaged chambers during or after backfilling is costly and very time consuming. It is recommended that all installers are familiar with this guide, and that the contractor inspects the chambers for distortion, damage and joint integrity as work progresses.
- Use of a dozer to push embedment stone between the rows of chambers may cause damage to chambers and is not an acceptable backfill method. Any chambers damaged by using the “dump and push” method are not covered under the StormTech standard warranty.
- Care should be taken in the handling of chambers and end caps. Avoid dropping, prying or excessive force on chambers during removal from pallet and initial placement.

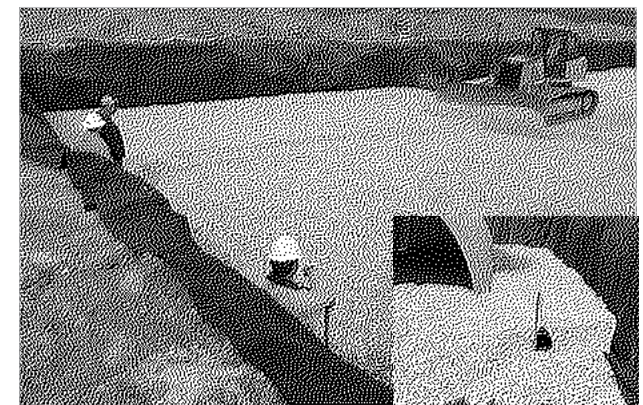
Requirements for System Installation



Excavate bed and prepare subgrade per engineer's plans.



Place non-woven geotextile over prepared soils and up excavation walls. Install underdrains if required.



Place clean, crushed, angular stone foundation 6" (150 mm) min. Compact to achieve a flat surface.

Manifold, Scour Fabric and Chamber Assembly



Install manifolds and lay out ADS Plus fabric at inlet rows (min. 12.5 ft (3.8 m)) at each inlet end cap. Place a continuous piece along entire length of Isolator® Plus Row(s).



Align the first chamber and end cap of each row with inlet pipes. Contractor may choose to postpone stone placement around end chambers and leave ends of rows open for easy inspection of chambers during the backfill process.



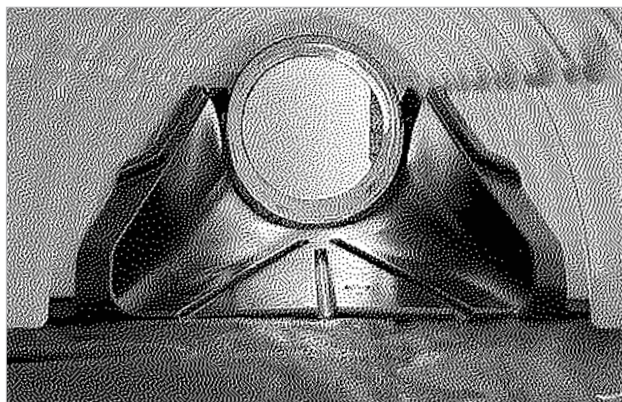
Continue installing chambers by overlapping chamber end corrugations. Chamber joints are labeled “Lower Joint – Overlap Here” and “Build this direction – Upper Joint” Be sure that the chamber placement does not exceed the reach of the construction equipment used to place the stone. Maintain minimum 6” (150 mm) spacing between rows.

Attaching the End Caps



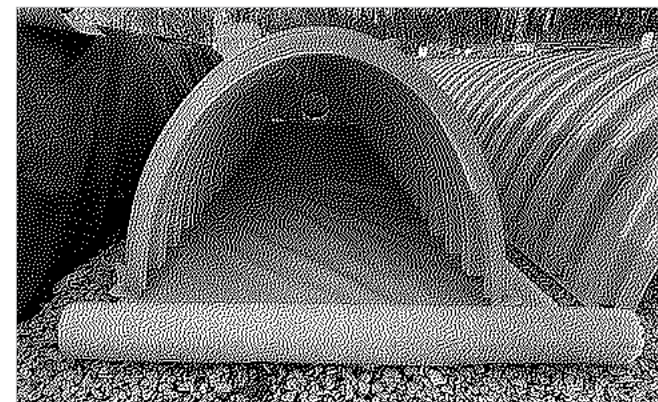
Lift the end of the chamber a few inches off the ground. With the curved face of the end cap facing outward, place the end cap into the chamber's end corrugation.

Prefabricated End Caps



24” (600 mm) inlets are the maximum size that can fit into a SC-740/DC-780/SC-800 end cap and must be prefabricated with a 24” (600 mm) pipe stub. SC-310 chambers with a 12” (300 mm) inlet pipe must use a prefabricated end cap with a 12” (300 mm) pipe stub. When used on an Isolator Row Plus, these end caps will contain a welded Flamp (flared end ramp) that will lay on top of the ADS Plus fabric (shown above)

Isolator Row Plus



Place a continuous layer of ADS Plus fabric between the foundation stone and the Isolator Row Plus chambers, making sure the fabric lays flat and extends the entire width of the chamber feet.

Initial Anchoring of Chambers – Embedment Stone



Initial embedment shall be spotted along the center line of the chamber evenly anchoring the lower portion of the chamber. This is best accomplished with a stone conveyor or excavator reaching along the row.

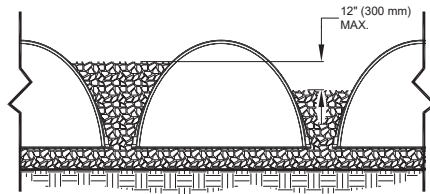


No equipment shall be operated on the bed at this stage of the installation. Excavators must be located off the bed. Dump trucks shall not dump stone directly on to the bed. Dozers or loaders are not allowed on the bed at this time.

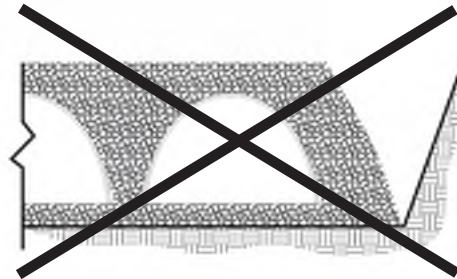
Backfill of Chambers – Embedment Stone



Uneven Backfill



Even Backfill



Perimeter Not Backfilled

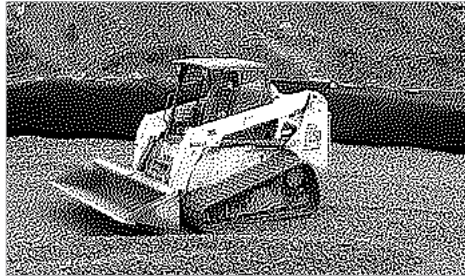


Perimeter Fully Backfilled

Backfill chambers evenly. Stone column height should never differ by more than 12" (300 mm) between adjacent chamber rows or between chamber rows and perimeter.

Perimeter stone must be brought up evenly with chamber rows. Perimeter must be fully backfilled, with stone extended horizontally to the excavation wall.

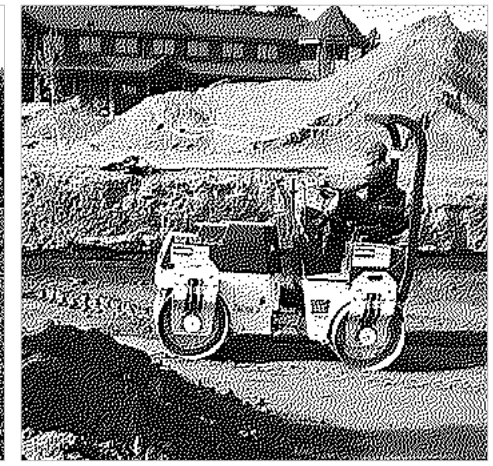
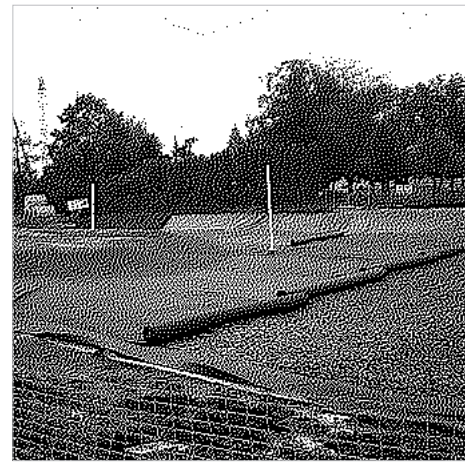
Backfill - Embedment Stone & Cover Stone



Continue evenly backfilling between rows and around perimeter until embedment stone reaches tops of chambers. Perimeter stone must extend horizontally to the excavation wall for both straight or sloped sidewalls. **Only after chambers have been backfilled to top of chamber and with a minimum 6" (150 mm) of cover stone on top of chambers can small dozers be used over the chambers for backfilling remaining cover stone.**

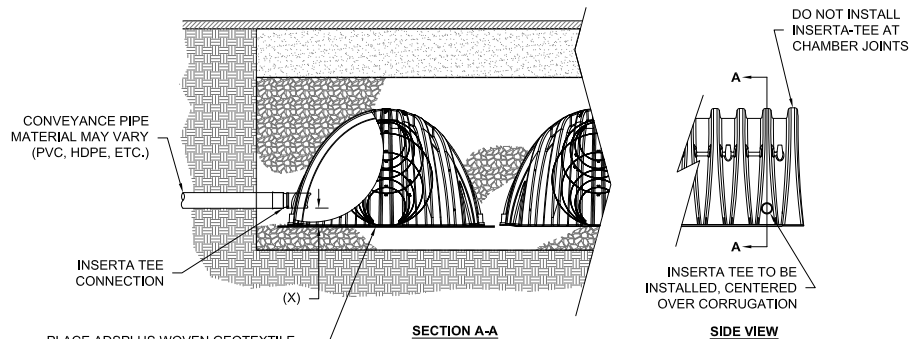
Small dozers and skid loaders may be used to finish grading stone backfill in accordance with ground pressure limits in Table 2. They must push material parallel to rows only. Never push perpendicular to rows. StormTech recommends that the contractor inspect chambers before placing final backfill. Any chambers damaged by construction shall be removed and replaced.

Final Backfill of Chambers – Fill Material



Install non-woven geotextile over stone. Geotextile must overlap 24" (600 mm) min. where edges meet. Compact each lift of backfill as specified in the site design engineer's drawings. Roller travel parallel with rows.

Inserta Tee Detail



PLACE ADPLUS WOVEN GEOTEXTILE (CENTERED ON INSERTA-TEE INLET) OVER BEDDING STONE FOR SCOUR PROTECTION AT SIDE INLET CONNECTIONS. GEOTEXTILE MUST EXTEND 6" (150 mm) PAST CHAMBER FOOT

NOTES:

- PART NUMBERS WILL VARY BASED ON INLET PIPE MATERIALS. CONTACT STORMTECH FOR MORE INFORMATION.
- CONTACT ADS ENGINEERING SERVICES IF INSERTA TEE INLET MUST BE RAISED AS NOT ALL INVERTS ARE POSSIBLE.

CHAMBER	MAX DIAMETER OF INSERTA TEE	HEIGHT FROM BASE OF CHAMBER (X)
SC-310	6" (150 mm)	4" (100 mm)
SC-740	10" (250 mm)	4" (100 mm)
SC-800	10" (250 mm)	4" (100 mm)
DC-780	10" (250 mm)	4" (100 mm)

INSERTA TEE FITTINGS AVAILABLE FOR SDR 26, SDR 35, SCH 40 IPS GASKETED & SOLVENT WELD, N-12, HP STORM, C-900 OR DUCTILE IRON

StormTech Isolator Row Plus Detail

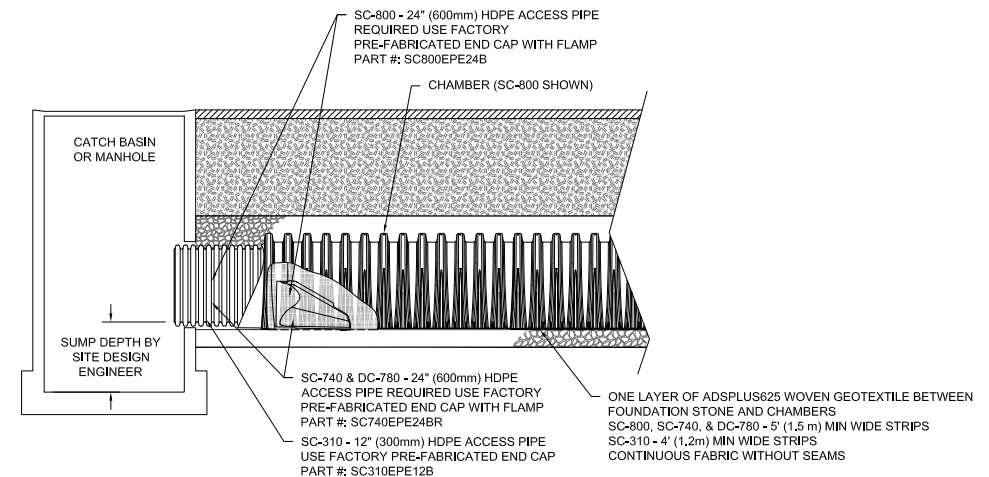


Table 1- Acceptable Fill Materials

Material Location	Description	AASHTO M43 Designation ¹	Compaction/Density Requirement
D Final Fill: Fill Material for layer 'D' starts from the top of the 'C' layer to the bottom of flexible pavement or unpaved finished grade above. Note that the pavement subbase may be part of the 'D' layer.	Any soil/rock materials, native soils or per engineer's plans. Check plans for pavement subgrade requirements.	N/A	Prepare per site design engineer's plans. Paved installations may have stringent material and preparation requirements.
C Initial Fill: Fill Material for layer 'C' starts from the top of the embedment stone ('B' layer) to 18" (450 mm) above the top of the chamber. Note that pavement subbase may be part of the 'C' layer.	Granular well-graded soil/aggregate mixtures, <35% fines or processed aggregate. Most pavement subbase materials can be used in lieu of this layer.	AASHTO M45 A-1, A-2-4, A-3 or AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	Begin compaction after min. 12" (300 mm) of material over the chambers is reached. Compact additional layers in 6" (150 mm) max. lifts to a min. 95% Proctor density for well-graded material and 95% relative density for processed aggregate materials. Roller gross vehicle weight not to exceed 12,000 lbs (53 kN). Dynamic force not to exceed 20,000 lbs (89 kN)
B Embedment Stone: Embedment Stone surrounding chambers from the foundation stone to the 'C' layer above.	Clean, crushed, angular stone or Recycled Concrete ⁴	AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57	No compaction required.
A Foundation Stone: Foundation Stone below the chambers from the subgrade up to the foot (bottom) of the chamber.	Clean, crushed, angular stone or Recycled Concrete ⁴	AASHTO M43 ¹ 3, 357, 4, 467, 5, 56, 57	Place and compact in 6" (150 mm) lifts using two full coverages with a vibratory compactor. ^{2,3}

Please Note:

- The listed AASHTO designations are for gradations only. The stone must also be clean, crushed, angular. For example, a specification for #4 stone would state: "clean, crushed, angular no. 4 (AASHTO M43) stone".
- StormTech compaction requirements are met for 'A' location materials when placed and compacted in 6" (150 mm) (max) lifts using two full coverages with a vibratory compactor.
- Where infiltration surfaces may be comprised by compaction, for standard installations and standard design load conditions, a flat surface may be achieved by raking or dragging without compaction equipment. For special load designs, contact StormTech for compaction requirements.
- Where recycled concrete aggregate is used in layers 'A' or 'B' the material should also meet the acceptable criteria outlined in ADS Technical Note 6.20 "Recycled Concrete Structural Backfill".

Figure 2 - Fill Material Locations

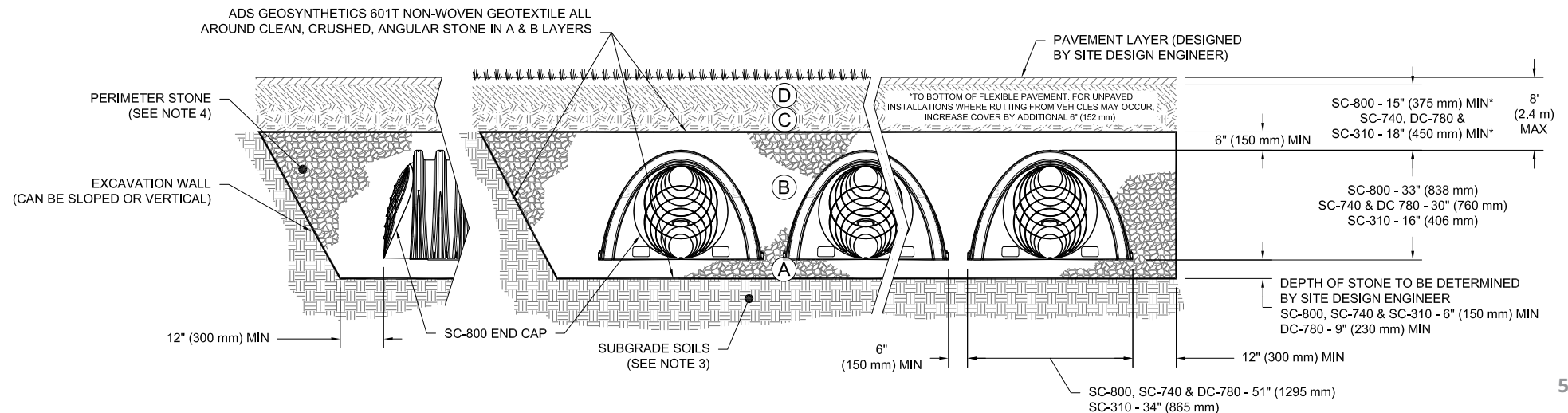
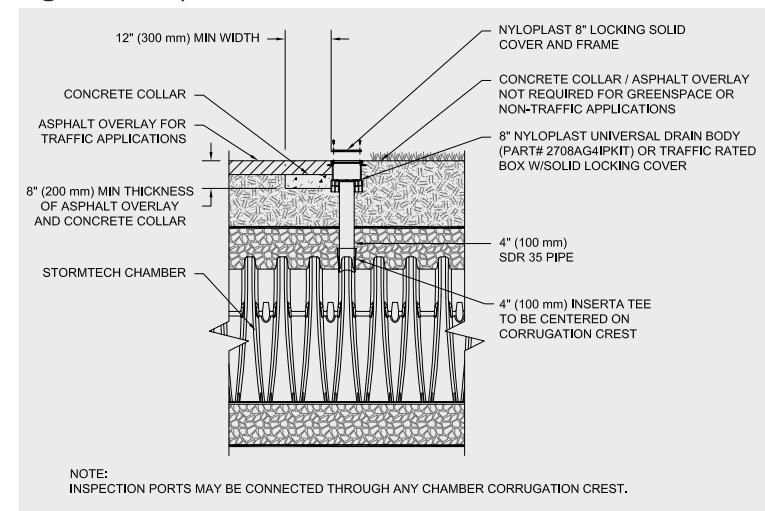


Figure 1- Inspection Port Detail



Notes:

- 36" (900 mm) of stabilized cover materials over the chambers is recommended during the construction phase if general construction activities, such as full dump truck travel and dumping, are to occur over the bed.
- During paving operations, dump truck axle loads on 18" (450 mm) of cover may be necessary. Precautions should be taken to avoid rutting of the road base layer, to ensure that compaction requirements have been met, and that a minimum of 18" (450 mm) of cover exists over the chambers. Contact StormTech for additional guidance on allowable axle loads during paving.
- Ground pressure for track dozers is the vehicle operating weight divided by total ground contact area for both tracks. Excavators will exert higher ground pressures based on loaded bucket weight and boom extension.
- Mini-excavators (< 8,000 lbs/3,628 kg) can be used with at least 12" (300 mm) of stone over the chambers and are limited by the maximum ground pressures in Table 2 based on a full bucket at maximum boom extension.
- Storage of materials such as construction materials, equipment, spoils, etc. should not be located over the StormTech system. The use of equipment over the StormTech system not covered in Table 2 (ex. soil mixing equipment, cranes, etc) is limited. Please contact StormTech for more information.
- Allowable track loads based on vehicle travel only. Excavators shall not operate on chamber beds until the total backfill reaches 3 feet (900 mm) over the entire bed.

Table 2 - Maximum Allowable Construction Vehicle Loads⁶

Material Location	Fill Depth over Chambers in. (mm)	Maximum Allowable Wheel Loads		Maximum Allowable Track Loads ⁶		Maximum Allowable Roller Loads
		Max Axle Load for Trucks lbs (kN)	Max Wheel Load for Loaders lbs (kN)	Track Width in. (mm)	Max Ground Pressure psf (kPa)	Max Drum Weight or Dynamic Force lbs (kN)
Ⓐ Final Fill Material	36" (900) Compacted	32,000 (142)	16,000 (71)	12" (305) 18" (457) 24" (610) 30" (762) 36" (914)	3880 (186) 2640 (126) 2040 (97) 1690 (81) 1470 (70)	38,000 (169)
Ⓑ Initial Fill Material	24" (600) Compacted	32,000 (142)	16,000 (71)	12" (305) 18" (457) 24" (610) 30" (762) 36" (914)	2690 (128) 1880 (90) 1490 (71) 1280 (61) 1150 (55)	20,000 (89)
	24" (600) Loose/Dumped	32,000 (142)	16,000 (71)	12" (305) 18" (457) 24" (610) 30" (762) 36" (914)	2390 (114) 1700 (81) 1370 (65) 1190 (57) 1080 (51)	20,000 (89) Roller gross vehicle weight not to exceed 12,000 lbs. (53 kN)
	18" (450)	32,000 (142)	16,000 (71)	12" (305) 18" (457) 24" (610) 30" (762) 36" (914)	2110 (101) 1510 (72) 1250 (59) 1100 (52) 1020 (48)	20,000 (89) Roller gross vehicle weight not to exceed 12,000 lbs. (53 kN)
Ⓒ Embedment Stone	12" (300)	16,000 (71)	NOT ALLOWED	12" (305) 18" (457) 24" (610) 30" (762) 36" (914)	1540 (74) 1190 (57) 1010 (48) 910 (43) 840 (40)	20,000 (89) Roller gross vehicle weight not to exceed 12,000 lbs. (53 kN)
	6" (150)	8,000 (35)	NOT ALLOWED	12" (305) 18" (457) 24" (610) 30" (762) 36" (914)	1070 (51) 900 (43) 800 (38) 760 (36) 720 (34)	NOT ALLOWED

Table 3 - Placement Methods and Descriptions

Material Location	Placement Methods/ Restrictions	Wheel Load Restrictions	Track Load Restrictions	Roller Load Restrictions
		See Table 2 for Maximum Construction Loads		
Ⓐ Final Fill Material	A variety of placement methods may be used. All construction loads must not exceed the maximum limits in Table 2.	36" (900 mm) minimum cover required for dump trucks to dump over chambers.	Dozers to push parallel to rows until 36" (900mm) compacted cover is reached. ⁴	Roller travel parallel to rows only until 36" (900 mm) compacted cover is reached.
Ⓑ Initial Fill Material	Excavator positioned off bed recommended. Small excavator allowed over chambers. Small dozer allowed.	Asphalt can be dumped into paver when compacted pavement subbase reaches 18" (450 mm) above top of chambers.	Small LGP track dozers & skid loaders allowed to grade cover stone with at least 6" (150 mm) stone under tracks at all times. Equipment must push parallel to rows at all times.	Use dynamic force of roller only after compacted fill depth reaches 12" (300 mm) over chambers. Roller travel parallel to chamber rows only.
Ⓒ Embedment Stone	No equipment allowed on bare chambers. Use excavator or stone conveyor positioned off bed or on foundation stone to evenly fill around all chambers to at least the top of chambers.	No wheel loads allowed. Material must be placed outside the limits of the chamber bed.	No tracked equipment is allowed on chambers until a min. 6" (150 mm) cover stone is in place.	No rollers allowed.
Ⓓ Foundation Stone	No StormTech restrictions. Contractor responsible for any conditions or requirements by others relative to subgrade bearing capacity, dewatering or protection of subgrade.			

StormTech® Standard Limited Warranty

STANDARD LIMITED WARRANTY OF STORMTECH LLC ("STORMTECH"): PRODUCTS

- (A) This Limited Warranty applies solely to the StormTech chambers and end plates manufactured by StormTech and sold to the original purchaser (the "Purchaser"). The chambers and end plates are collectively referred to as the "Products."
- (B) The structural integrity of the Products, when installed strictly in accordance with StormTech's written installation instructions at the time of installation, are warranted to the Purchaser against defective materials and workmanship for one (1) year from the date of purchase. Should a defect appear in the Limited Warranty period, the Purchaser shall provide StormTech with written notice of the alleged defect at StormTech's corporate headquarters within ten (10) days of the discovery of the defect. The notice shall describe the alleged defect in reasonable detail. StormTech agrees to supply replacements for those Products determined by StormTech to be defective and covered by this Limited Warranty. The supply of replacement products is the sole remedy of the Purchaser for breaches of this Limited Warranty. StormTech's liability specifically excludes the cost of removal and/or installation of the Products.
- (C) THIS LIMITED WARRANTY IS EXCLUSIVE. THERE ARE NO OTHER WARRANTIES WITH RESPECT TO THE PRODUCTS, INCLUDING NO IMPLIED WARRANTIES OF MERCHANTABILITY OR OF FITNESS FOR A PARTICULAR PURPOSE.
- (D) This Limited Warranty only applies to the Products when the Products are installed in a single layer. UNDER NO CIRCUMSTANCES, SHALL THE PRODUCTS BE INSTALLED IN A MULTI-LAYER CONFIGURATION.
- (E) No representative of StormTech has the authority to change this Limited Warranty in any manner or to extend this Limited Warranty. This Limited Warranty does not apply to any person other than to the Purchaser.
- (F) Under no circumstances shall StormTech be liable to the Purchaser or to any third party for product liability claims; claims arising from the design, shipment, or installation of the Products, or the cost of other goods or services related to the purchase and installation of the Products. For this Limited Warranty to apply, the Products must be installed in accordance with all site conditions required by state and local codes; all other applicable laws; and StormTech's written installation instructions.
- (G) THE LIMITED WARRANTY DOES NOT EXTEND TO INCIDENTAL, CONSEQUENTIAL, SPECIAL OR INDIRECT DAMAGES. STORMTECH SHALL NOT BE LIABLE FOR PENALTIES OR LIQUIDATED DAMAGES, INCLUDING LOSS OF PRODUCTION AND PROFITS; LABOR AND MATERIALS; OVERHEAD COSTS; OR OTHER LOSS OR EXPENSE INCURRED BY THE PURCHASER OR ANY THIRD PARTY. SPECIFICALLY EXCLUDED FROM LIMITED WARRANTY COVERAGE ARE DAMAGE TO THE PRODUCTS ARISING FROM ORDINARY WEAR AND TEAR; ALTERATION, ACCIDENT, MISUSE, ABUSE OR NEGLIGENCE; THE PRODUCTS BEING SUBJECTED TO VEHICLE TRAFFIC OR OTHER CONDITIONS WHICH ARE NOT PERMITTED BY STORMTECH'S WRITTEN SPECIFICATIONS OR INSTALLATION INSTRUCTIONS; FAILURE TO MAINTAIN THE MINIMUM GROUND COVERS SET FORTH IN THE INSTALLATION INSTRUCTIONS; THE PLACEMENT OF IMPROPER MATERIALS INTO THE PRODUCTS; FAILURE OF THE PRODUCTS DUE TO IMPROPER SITING OR IMPROPER SIZING; OR ANY OTHER EVENT NOT CAUSED BY STORMTECH. A PRODUCT ALSO IS EXCLUDED FROM LIMITED WARRANTY COVERAGE IF SUCH PRODUCT IS USED IN A PROJECT OR SYSTEM IN WHICH ANY GEOTEXTILE PRODUCTS OTHER THAN THOSE PROVIDED BY ADVANCED DRAINAGE SYSTEMS ARE USED. THIS LIMITED WARRANTY REPRESENTS STORMTECH'S SOLE LIABILITY TO THE PURCHASER FOR CLAIMS RELATED TO THE PRODUCTS, WHETHER THE CLAIM IS BASED UPON CONTRACT, TORT, OR OTHER LEGAL THEORY.





ADS 0601T/O NONWOVEN GEOTEXTILE SPECIFICATION

Scope

This specification describes ADS 0601T/O nonwoven geotextile.

Filter Fabric Requirements

ADS 0601T/O is an orange nonwoven geotextile composed of polypropylene fibers, which are formed into a stable network such that the fibers retain their relative position. ADS 0601T/O is inert to biological degradation and resists naturally encountered chemicals, alkali and acids. ADS 0601T/O conforms to the physical property values listed below:

Filter Fabric Properties

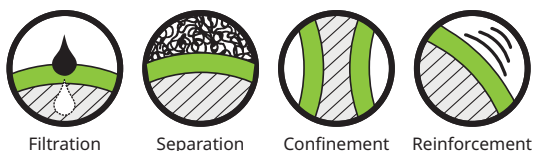
Property	Test Method	Unit	Typical Value ¹ MD	Typical Value ¹ CD
Grab Tensile Strength	ASTM D4632	lbs (N)	175 (779)	175 (779)
Grab Tensile Elongation	ASTM D4632	%	75	75
Trapezoid Tear Strength	ASTM D4533	lbs (N)	85 (378)	85 (378)
CBR Puncture Strength	ASTM D6241	lbs (N)	480 (2136)	480 (2136)
Permittivity	ASTM D4491	sec ⁻¹	1.5	1.5
Flow Rate	ASTM D4491	gal/min/ft ² (l/min/m ²)	105 (4278)	105 (4278)
UV Resistance (at 500 hours) ¹	ASTM D4355	% strength retained	80	80

Physical Properties

Property	Test Method	Unit	Typical Value ²
Weight	ASTM D5161	oz/yd ² (g/m ²)	6.5 (220)
Thickness	ASTM D5199	mils (mm)	65 (1.7)
Roll Dimensions (W x L)	-	ft (m)	15 x 300 (4.5 x 91)
Roll Area	-	yd ² (m ²)	500 (418)
Estimated Roll Weight	-	lb (kg)	220 (100)

¹ Modified, Minimum Test Value

² ASTM D4439 Standard Terminology for Geosynthetics: typical value, *n-for geosynthetics*, the mean value calculated from documented manufacturing quality control test results for a defined population obtained from one test method associated with on specific property.



ADS PLUS WOVEN GEOTEXTILE SPECIFICATION

For use with StormTech® Isolator® Row Plus

Scope

This specification describes ADS Plus woven geotextile.

ADS Plus woven geotextile fabrics are woven polypropylene materials offering optimum performance when used in stabilization applications. Produced from first quality raw materials, they provide the perfect balance of strength and separation in styles capable of functioning exceptionally well in a wide range of performance requirements.

Filter Fabric Properties

Property ¹	Test Method	Unit	M.A.R.V. (Minimum Average Roll Value) ²
Weight	ASTM D5261	oz/yd ² (g/m ²)	8.0 (271.25)
Grab Tensile Strength	ASTM D4632	lbs (kN)	325 (1.45)
Grab Elongation	ASTM D4632	%	15
Trapezoidal Tear Strength	ASTM D4533	lbs (kN)	125 (0.89)
CBR Puncture Resistance	ASTM D6241	lbs (kN)	1,124 (5.0)

1. The property values listed above are subject to change without notice.

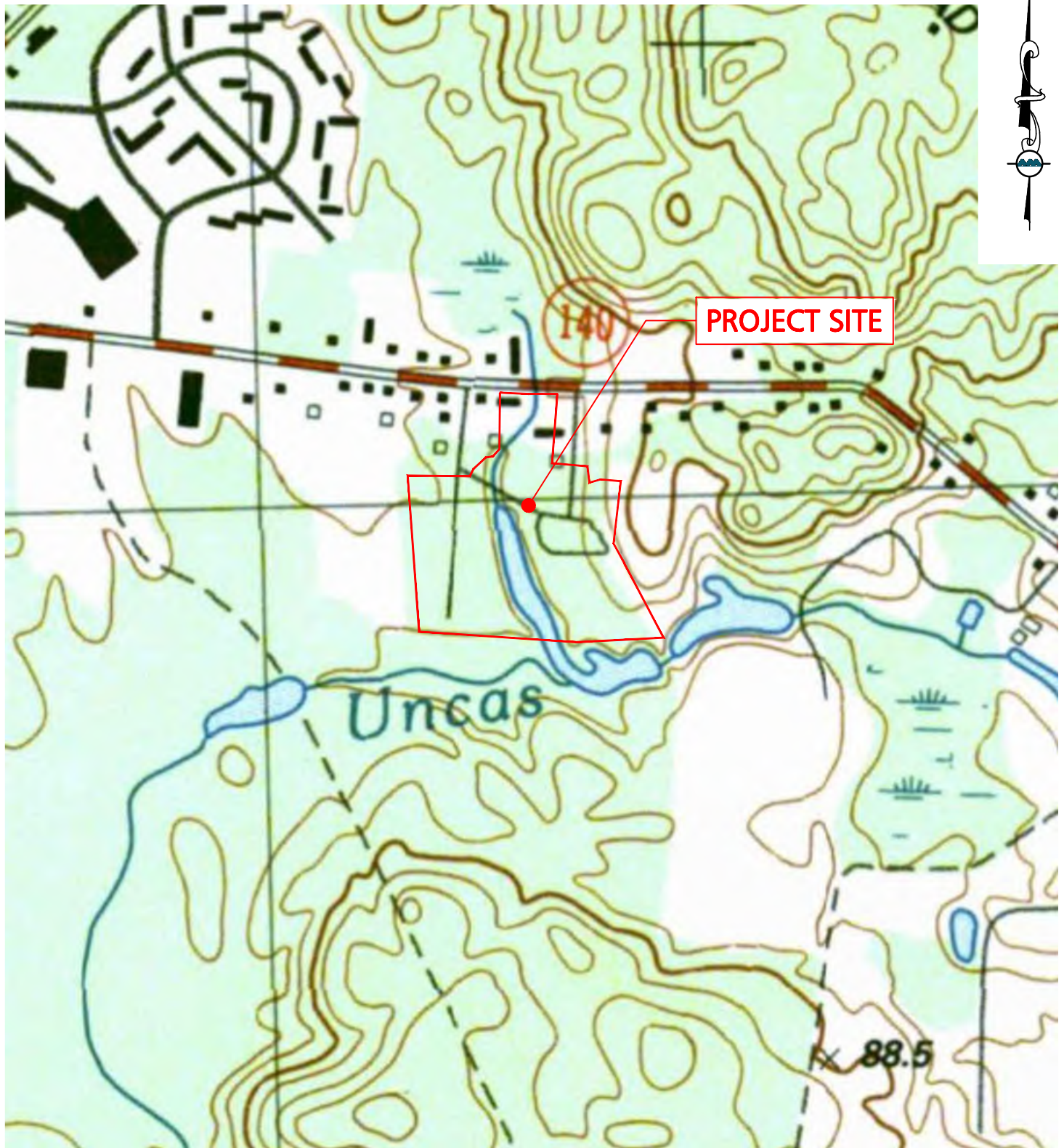
2. Minimum Average Roll Values (MARV) is calculated as the average minus two standard deviations. Statistically, it yields approximately 97.5% degree of confidence that any samples taken from quality assurance testing will meet or exceed the values described above.

Dimensions

ADS Plus shall be delivered to the jobsite in roll form with each roll individually identified and nominally measuring 12.5' (3.8 m) width x 360' (110 m) length for Plus125 and 6.25' (1.9 m) width x 360' (110 m) length for Plus625.



**SECTION 3.0 -
EXHIBITS**



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PROJECT:

**444 EAST CENTRAL STREET
FRANKLIN, MA**

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USGS SITE LOCUS MAP

PROJECT NO. 3317-01 DATE: 02/07/2025

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DESIGNED BY: MTB CHECKED BY: CMQ

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SHEET No.

EX-2

LEGEND

-  Coastal Bank Bluff or Sea Cliff
-  Coastal Beach
-  Coastal Dune
-  Cranberry Bog
-  Deep Marsh
-  Barrier Beach-Open Water
-  Open Water
-  Rocky Intertidal Shore
-  Salt Marsh
-  Shallow Marsh Meadow or Fen
-  Shrub Swamp
-  Tidal Flat
-  Wooded Swamp Coniferous
-  Wooded Swamp Deciduous
-  Wooded Swamp Mixed Trees



PROJECT SITE

MA MAPPER DEP WETLANDS

THERE ARE OPEN WATER & SHRUB SWAMP WETLANDS DELINEATED ON THE SITE

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WETLANDS MAP

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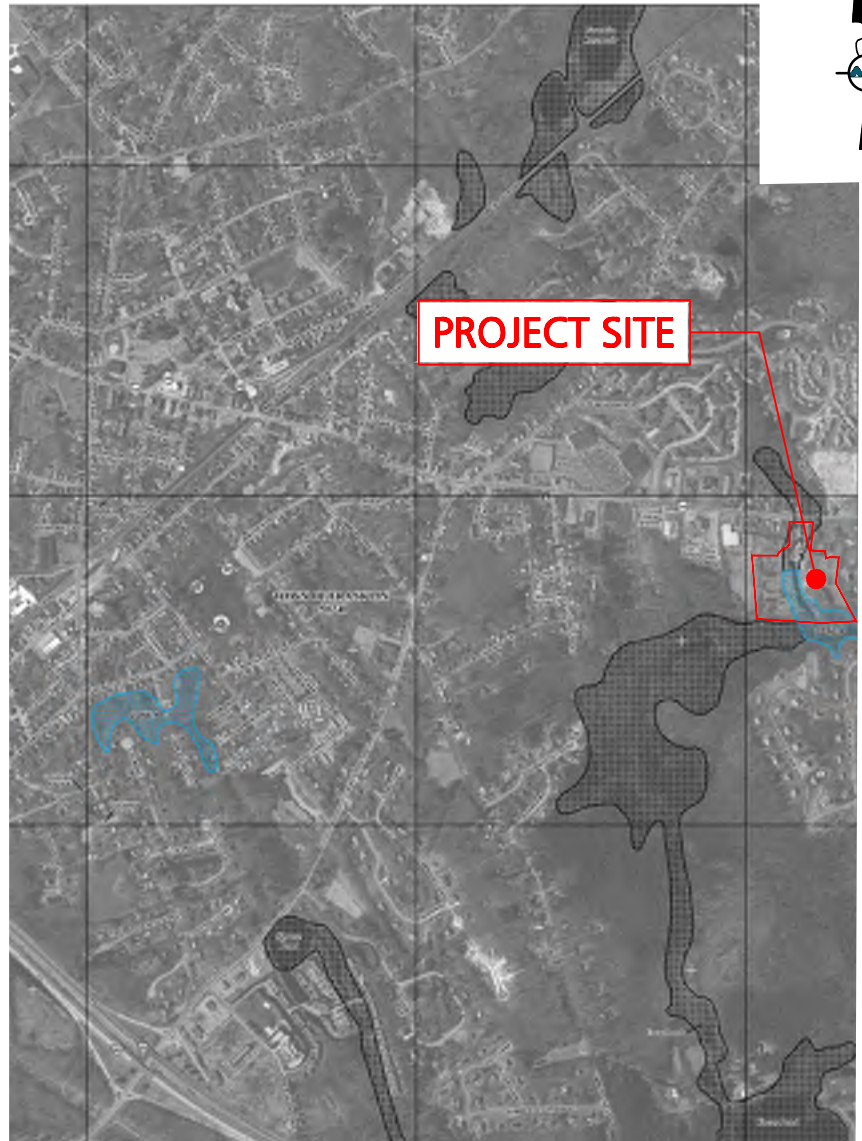
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SHEET No.

EX-3

LEGEND



FEMA FLOOD INSURANCE RATE MAP
NORFOLK COUNTY, MASSACHUSETTS
COMMUNITY PANEL 309 OF 430
MAP NUMBER 25021C0309E
EFFECTIVE DATE: JULY 17, 2012

PREPARED BY:



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FEMA FIRM MAP

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EX-4



- ☒ [NHESP Priority Habitats of Rare Species](#)
- ☒ [NHESP Estimated Habitats of Rare Wildlife](#)
- ☒ [Property Tax Parcels](#)

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**444 EAST CENTRAL STREET
FRANKLIN, MA**

DESIGNED BY:	MTB	CHECKED BY:	CMQ
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EX-5



SECTION 4.0 - EXISTING DRAINAGE ANALYSIS

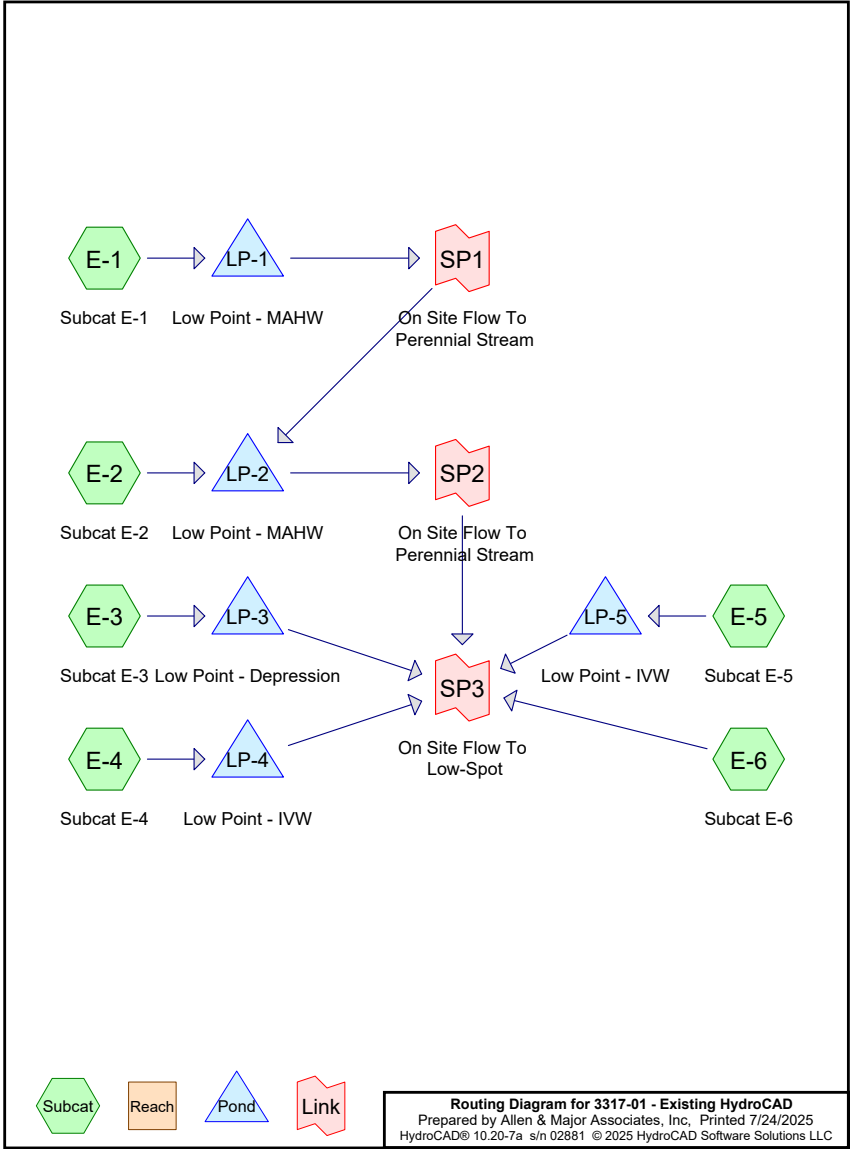
3317-01 - Existing HydroCAD

Prepared by Allen & Major Associates, Inc
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Printed 7/24/2025
Page 2

Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2-year	Type III 24-hr		Default	24.00	1	3.37	2
2	10-year	Type III 24-hr		Default	24.00	1	5.22	2
3	100-year	Type III 24-hr		Default	24.00	1	8.16	2



Area Listing (all nodes)		
Area (sq-ft)	CN	Description (subcatchment-numbers)
250,299	39	>75% Grass cover, Good, HSG A (E-1, E-2, E-3, E-4, E-5, E-6)
23,768	96	Gravel surface, HSG A (E-2, E-5, E-6)
52,273	98	Paved parking, HSG A (E-1, E-2, E-4, E-6)
22,408	98	Unconnected roofs, HSG A (E-1, E-2, E-6)
33,288	98	Water Surface, HSG A (E-1, E-2, E-6)
485,446	30	Woods, Good, HSG A (E-3, E-4, E-5, E-6)
867,482	43	TOTAL AREA

Soil Listing (all nodes)		
Area (sq-ft)	Soil Group	Subcatchment Numbers
867,482	HSG A	E-1, E-2, E-3, E-4, E-5, E-6
0	HSG B	
0	HSG C	
0	HSG D	
0	Other	
867,482		TOTAL AREA

Ground Covers (all nodes)							Sub Nur
HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	
250,299	0	0	0	0	250,299	>75% Grass cover, Good	
23,768	0	0	0	0	23,768	Gravel surface	
52,273	0	0	0	0	52,273	Paved parking	
22,408	0	0	0	0	22,408	Unconnected roofs	
33,288	0	0	0	0	33,288	Water Surface	
485,446	0	0	0	0	485,446	Woods, Good	
867,482	0	0	0	0	867,482	TOTAL AREA	

Pipe Listing (all nodes)											Node Name
Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)		
1	LP-1	267.72	267.93	59.0	-0.0036	0.014	0.0	24.0	0.0		
2	LP-1	267.48	267.48	59.0	0.0000	0.014	0.0	24.0	0.0		
3	LP-2	266.37	266.85	30.0	-0.0160	0.011	0.0	24.0	0.0		

Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment E-1: Subcat E-1	Runoff Area=101,514 sf 37.70% Impervious Runoff Depth=1.18" Flow Length=671' Tc=8.6 min CN=WQ Runoff=2.64 cfs 10,024 cf
Subcatchment E-2: Subcat E-2	Runoff Area=65,426 sf 33.91% Impervious Runoff Depth=1.12" Flow Length=485' Tc=8.1 min CN=WQ Runoff=1.64 cfs 6,133 cf
Subcatchment E-3: Subcat E-3	Runoff Area=17,073 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=170' Tc=10.4 min CN=WQ Runoff=0.00 cfs 2 cf
Subcatchment E-4: Subcat E-4	Runoff Area=111,271 sf 4.45% Impervious Runoff Depth=0.14" Flow Length=570' Tc=19.9 min CN=WQ Runoff=0.25 cfs 1,296 cf
Subcatchment E-5: Subcat E-5	Runoff Area=68,461 sf 0.00% Impervious Runoff Depth=0.05" Flow Length=351' Tc=8.1 min CN=WQ Runoff=0.07 cfs 264 cf
Subcatchment E-6: Subcat E-6	Runoff Area=503,736 sf 8.45% Impervious Runoff Depth=0.39" Flow Length=710' Tc=14.7 min UI Adjusted CN=WQ Runoff=3.66 cfs 16,358 cf
Pond LP-1: Low Point - MAHW	Peak Elev=268.31' Storage=298 cf Inflow=2.64 cfs 10,024 cf Outflow=2.63 cfs 9,929 cf
Pond LP-2: Low Point - MAHW	Peak Elev=267.57' Storage=846 cf Inflow=4.26 cfs 16,062 cf 24.0" Round Culvert n=0.011 L=30.0' S=-0.0160 '/' Outflow=3.97 cfs 16,062 cf
Pond LP-3: Low Point - Depression	Peak Elev=271.00' Storage=0 cf Inflow=0.00 cfs 2 cf Discarded=0.00 cfs 2 cf Primary=0.00 cfs 0 cf Outflow=0.00 cfs 2 cf
Pond LP-4: Low Point - IVW	Peak Elev=271.51' Storage=41 cf Inflow=0.25 cfs 1,296 cf Outflow=0.25 cfs 1,257 cf
Pond LP-5: Low Point - IVW	Peak Elev=271.06' Storage=264 cf Inflow=0.07 cfs 264 cf Outflow=0.00 cfs 0 cf
Link SP1: On Site Flow To Perennial Stream	Inflow=2.63 cfs 9,929 cf Primary=2.63 cfs 9,929 cf
Link SP2: On Site Flow To Perennial Stream	Inflow=3.97 cfs 16,062 cf Primary=3.97 cfs 16,062 cf
Link SP3: On Site Flow To Low-Spot	Inflow=7.78 cfs 33,678 cf Primary=7.78 cfs 33,678 cf

Total Runoff Area = 867,482 sf Runoff Volume = 34,078 cf Average Runoff Depth = 0.47"
87.55% Pervious = 759,513 sf 12.45% Impervious = 107,969 sf

Summary for Subcatchment E-1: Subcat E-1

Runoff = 2.64 cfs @ 12.12 hrs, Volume= 10,024 cf, Depth= 1.18"
Routed to Pond LP-1 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.37"

Area (sf)	CN	Description
63,241	39	>75% Grass cover, Good, HSG A
26,126	98	Paved parking, HSG A
11,715	98	Unconnected roofs, HSG A
431	98	Water Surface, HSG A
101,514		Weighted Average
63,241	39	62.30% Pervious Area
38,273	98	37.70% Impervious Area
11,715		30.61% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.9	50	0.1000	0.29		Sheet Flow, A-B Grass: Short n= 0.150 P2= 3.37"
2.1	195	0.0500	1.57		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
0.1	30	0.0300	3.52		Shallow Concentrated Flow, C-D Paved Kv= 20.3 fps
1.8	166	0.0480	1.53		Shallow Concentrated Flow, D-E Short Grass Pasture Kv= 7.0 fps
0.1	30	0.0300	3.52		Shallow Concentrated Flow, E-F Paved Kv= 20.3 fps
1.1	100	0.0500	1.57		Shallow Concentrated Flow, F-G Short Grass Pasture Kv= 7.0 fps
0.5	100	0.0330	3.69		Shallow Concentrated Flow, G-H Paved Kv= 20.3 fps
8.6	671	Total			

Summary for Subcatchment E-2: Subcat E-2

Runoff = 1.64 cfs @ 12.11 hrs, Volume= 6,133 cf, Depth= 1.12"
Routed to Pond LP-2 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.37"

Area (sf)	CN	Description
41,920	39	>75% Grass cover, Good, HSG A
1,317	96	Gravel surface, HSG A
14,955	98	Paved parking, HSG A
4,757	98	Unconnected roofs, HSG A
2,477	98	Water Surface, HSG A
65,426		Weighted Average
43,237	41	66.09% Pervious Area
22,189	98	33.91% Impervious Area
4,757		21.44% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.9	50	0.1000	0.29		Sheet Flow, A-B Grass: Short n= 0.150 P2= 3.37"
0.6	60	0.0600	1.71		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
0.3	55	0.0200	2.87		Shallow Concentrated Flow, C-D Paved Kv= 20.3 fps
0.8	60	0.0330	1.27		Shallow Concentrated Flow, D-E Short Grass Pasture Kv= 7.0 fps
1.2	50	0.0200	0.71		Shallow Concentrated Flow, E-F Woodland Kv= 5.0 fps
0.8	50	0.0200	0.99		Shallow Concentrated Flow, F-G Short Grass Pasture Kv= 7.0 fps
1.3	130	0.0070	1.70		Shallow Concentrated Flow, G-H Paved Kv= 20.3 fps
0.2	30	0.1000	2.85		Shallow Concentrated Flow, H-I Cultivated Straight Rows Kv= 9.0 fps
8.1	485	Total			

Summary for Subcatchment E-3: Subcat E-3

Runoff = 0.00 cfs @ 23.84 hrs, Volume= 2 cf, Depth= 0.00"
Routed to Pond LP-3 : Low Point - Depression

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.37"

Area (sf)	CN	Description
6,828	39	>75% Grass cover, Good, HSG A
10,245	30	Woods, Good, HSG A
17,073		Weighted Average
17,073	34	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	50	0.0200	0.10		Sheet Flow, A-B Grass: Dense n= 0.240 P2= 3.37"
1.6	60	0.0160	0.63		Shallow Concentrated Flow, B-C Woodland Kv= 5.0 fps
0.8	60	0.0600	1.22		Shallow Concentrated Flow, C-D Woodland Kv= 5.0 fps
10.4	170	Total			

Summary for Subcatchment E-4: Subcat E-4

Runoff = 0.25 cfs @ 12.27 hrs, Volume= 1,296 cf, Depth= 0.14"
Routed to Pond LP-4 : Low Point - IVW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.37"

Area (sf)	CN	Description
1,874	39	>75% Grass cover, Good, HSG A
4,954	98	Paved parking, HSG A
104,443	30	Woods, Good, HSG A
111,271		Weighted Average
106,317	30	95.55% Pervious Area
4,954	98	4.45% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	38	0.0200	1.16		Sheet Flow, A-B Smooth surfaces n= 0.011 P2= 3.37"
1.1	12	0.0600	0.18		Sheet Flow, B-C Grass: Short n= 0.150 P2= 3.37"
18.3	520	0.0090	0.47		Shallow Concentrated Flow, C-D Woodland Kv= 5.0 fps
19.9	570	Total			

Summary for Subcatchment E-5: Subcat E-5

Runoff = 0.07 cfs @ 12.11 hrs, Volume= 264 cf, Depth= 0.05"
Routed to Pond LP-5 : Low Point - IVW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.37"

Area (sf)	CN	Description
20,858	39	>75% Grass cover, Good, HSG A
1,061	96	Gravel surface, HSG A
46,542	30	Woods, Good, HSG A
68,461		Weighted Average
68,461	34	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.7	50	0.0300	0.18		Sheet Flow, A-B Grass: Short n= 0.150 P2= 3.37"
0.2	16	0.0300	1.21		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
3.2	285	0.0900	1.50		Shallow Concentrated Flow, C-D Woodland Kv= 5.0 fps
8.1	351	Total			

Summary for Subcatchment E-6: Subcat E-6

Runoff = 3.66 cfs @ 12.20 hrs, Volume= 16,358 cf, Depth= 0.39"
Routed to Link SP3 : On Site Flow To Low-Spot

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2-year Rainfall=3.37"

Area (sf)	CN	Adj	Description
115,578	39	39	>75% Grass cover, Good, HSG A
21,390	96	96	Gravel surface, HSG A
6,237	98	98	Paved parking, HSG A
5,936	98	98	Unconnected roofs, HSG A
30,379	98	98	Water Surface, HSG A
324,216	30	30	Woods, Good, HSG A
503,736			Weighted Average
461,184	35	35	91.55% Pervious Area
42,553	98	98	8.45% Impervious Area
5,936			13.95% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.9	50	0.1200	0.21		Sheet Flow, A-B Grass: Dense n= 0.240 P2= 3.37"
0.4	60	0.1500	2.71		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
3.3	330	0.0570	1.67		Shallow Concentrated Flow, C-D Short Grass Pasture Kv= 7.0 fps
3.8	115	0.0100	0.50		Shallow Concentrated Flow, D-E Woodland Kv= 5.0 fps
3.1	130	0.0100	0.70		Shallow Concentrated Flow, E-F Short Grass Pasture Kv= 7.0 fps
0.2	25	0.0800	1.98		Shallow Concentrated Flow, F-G Short Grass Pasture Kv= 7.0 fps
14.7	710	Total			

Summary for Pond LP-1: Low Point - MAHW

Inflow Area = 101,514 sf, 37.70% Impervious, Inflow Depth = 1.18" for 2-year event
Inflow = 2.64 cfs @ 12.12 hrs, Volume= 10,024 cf
Outflow = 2.63 cfs @ 12.13 hrs, Volume= 9,929 cf, Atten= 1%, Lag= 0.6 min
Primary = 2.63 cfs @ 12.13 hrs, Volume= 9,929 cf
Routed to Link SP1 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 268.31' @ 12.13 hrs Surf.Area= 283 sf Storage= 298 cf

Plug-Flow detention time= 18.0 min calculated for 9,929 cf (99% of inflow)
Center-of-Mass det. time= 11.7 min (770.5 - 758.8)

Volume	Invert	Avail.Storage	Storage	Description
#1	267.00'	902 cf		Custom Stage Data (Irregular) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
267.00	175	160.0	0	0	175
268.00	258	168.0	215	215	441
269.00	344	172.0	300	515	642
270.00	431	176.0	387	902	848

Device	Routing	Invert	Outlet Devices
#1	Primary	267.93'	24.0" Round RCP_Round 24" L= 59.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 267.72' / 267.93' S= -0.0036 '/' Cc= 0.900 n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf
#2	Primary	267.48'	24.0" Round RCP_Round 24" L= 59.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 267.48' / 267.48' S= 0.0000 '/' Cc= 0.900 n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf

Primary OutFlow Max=2.62 cfs @ 12.13 hrs HW=268.31' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Barrel Controls 0.84 cfs @ 1.64 fps)
2=RCP_Round 24" (Barrel Controls 1.78 cfs @ 2.15 fps)

Summary for Pond LP-2: Low Point - MAHW

Inflow Area = 166,941 sf, 36.22% Impervious, Inflow Depth = 1.15" for 2-year event
Inflow = 4.26 cfs @ 12.12 hrs, Volume= 16,062 cf
Outflow = 3.97 cfs @ 12.16 hrs, Volume= 16,062 cf, Atten= 7%, Lag= 2.2 min
Primary = 3.97 cfs @ 12.16 hrs, Volume= 16,062 cf
Routed to Link SP2 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 267.57' @ 12.16 hrs Surf.Area= 1,491 sf Storage= 846 cf

Plug-Flow detention time= 4.7 min calculated for 16,058 cf (100% of inflow)
Center-of-Mass det. time= 4.7 min (770.9 - 766.2)

Volume	Invert	Avail.Storage	Storage Description		
#1	266.85'	5,666 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
266.85	0	0.0	0	0	0
267.00	1,276	387.0	64	64	11,918
268.00	1,668	396.0	1,468	1,531	12,602
269.00	2,069	404.0	1,865	3,396	13,250
270.00	2,477	412.0	2,270	5,666	13,910

Device	Routing	Invert	Outlet Devices
#1	Primary	266.85'	24.0" Round RCP_Round 24" L= 30.0' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 266.37' / 266.85' S= -0.0160 ' / Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=3.96 cfs @ 12.16 hrs HW=267.57' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Inlet Controls 3.96 cfs @ 3.93 fps)

Summary for Pond LP-3: Low Point - Depression

Inflow Area = 17,073 sf, 0.00% Impervious, Inflow Depth = 0.00" for 2-year event
Inflow = 0.00 cfs @ 23.84 hrs, Volume= 2 cf
Outflow = 0.00 cfs @ 23.94 hrs, Volume= 2 cf, Atten= 0%, Lag= 5.8 min
Discarded = 0.00 cfs @ 23.94 hrs, Volume= 2 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Link SP3 : On Site Flow To Low-Spot

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Peak Elev= 271.00' @ 23.94 hrs Surf.Area= 65 sf Storage= 0 cf

Plug-Flow detention time= 6.9 min calculated for 2 cf (100% of inflow)
Center-of-Mass det. time= 6.9 min (1,317.3 - 1,310.4)

Volume	Invert	Avail.Storage	Storage Description		
#1	271.00'	3,637 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.00	65	35.0	0	0	65
272.00	588	106.0	283	283	865
273.00	1,595	174.0	1,050	1,333	2,386
274.00	3,094	228.0	2,303	3,637	4,125

Device	Routing	Invert	Outlet Devices
#1	Primary	273.87'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 0.13 0.43 Width (feet) 0.00 17.00 64.00
#2	Discarded	271.00'	1.040 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 23.94 hrs HW=271.00' (Free Discharge)
2=Exfiltration (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=271.00' TW=0.00' (Dynamic Tailwater)
1=Custom Weir/Orifice (Controls 0.00 cfs)

Summary for Pond LP-4: Low Point - IVW

Inflow Area = 111,271 sf, 4.45% Impervious, Inflow Depth = 0.14" for 2-year event
Inflow = 0.25 cfs @ 12.27 hrs, Volume= 1,296 cf
Outflow = 0.25 cfs @ 12.26 hrs, Volume= 1,257 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.25 cfs @ 12.26 hrs, Volume= 1,257 cf
Routed to Link SP3 : On Site Flow To Low-Spot

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 271.51' @ 12.26 hrs Surf.Area= 403 sf Storage= 41 cf

Plug-Flow detention time= 34.0 min calculated for 1,257 cf (97% of inflow)
Center-of-Mass det. time= 15.5 min (784.0 - 768.5)

Volume	Invert	Avail.Storage	Storage Description		
#1	271.20'	732 cf	Low Point 4 (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.20	0	0.0	0	0	0
272.00	2,745	222.4	732	732	3,937

Device	Routing	Invert	Outlet Devices
#1	Primary	271.50'	200.0' long x 3.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68 2.72 2.81 2.92 2.97 3.07 3.32
#2	Primary	271.50'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 0.50 Width (feet) 0.00 40.00

Primary OutFlow Max=0.25 cfs @ 12.26 hrs HW=271.51' TW=0.00' (Dynamic Tailwater)
1=Broad-Crested Rectangular Weir (Weir Controls 0.25 cfs @ 0.20 fps)
2=Custom Weir/Orifice (Weir Controls 0.00 cfs @ 0.21 fps)

Summary for Pond LP-5: Low Point - IWV

Inflow Area = 68,461 sf, 0.00% Impervious, Inflow Depth = 0.05" for 2-year event
Inflow = 0.07 cfs @ 12.11 hrs, Volume= 264 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Link SP3 : On Site Flow To Low-Spot

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 271.06' @ 24.47 hrs Surf.Area= 4,401 sf Storage= 264 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	271.00'	4,025 cf	Low Point 5.1 (Irregular) Listed below (Recalc)
#2	271.00'	4,810 cf	Low Point 5.2 (Irregular) Listed below (Recalc)
		8,835 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.00	1,533	173.1	0	0	1,533
272.00	7,215	468.2	4,025	4,025	16,596

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.00	2,382	270.9	0	0	2,382
272.00	7,752	387.7	4,810	4,810	8,512

Device	Routing	Invert	Outlet Devices
#1	Primary	271.50'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 0.50 Width (feet) 0.00 58.90

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=271.00' TW=0.00' (Dynamic Tailwater)
1=Custom Weir/Orifice (Controls 0.00 cfs)

Summary for Link SP1: On Site Flow To Perennial Stream

Inflow Area = 101,514 sf, 37.70% Impervious, Inflow Depth = 1.17" for 2-year event
Inflow = 2.63 cfs @ 12.13 hrs, Volume= 9,929 cf
Primary = 2.63 cfs @ 12.13 hrs, Volume= 9,929 cf, Atten= 0%, Lag= 0.0 min
Routed to Pond LP-2 : Low Point - MAHW

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP2: On Site Flow To Perennial Stream

Inflow Area = 166,941 sf, 36.22% Impervious, Inflow Depth = 1.15" for 2-year event
Inflow = 3.97 cfs @ 12.16 hrs, Volume= 16,062 cf
Primary = 3.97 cfs @ 12.16 hrs, Volume= 16,062 cf, Atten= 0%, Lag= 0.0 min
Routed to Link SP3 : On Site Flow To Low-Spot

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP3: On Site Flow To Low-Spot

Inflow Area = 867,482 sf, 12.45% Impervious, Inflow Depth = 0.47" for 2-year event
Inflow = 7.78 cfs @ 12.17 hrs, Volume= 33,678 cf
Primary = 7.78 cfs @ 12.17 hrs, Volume= 33,678 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment E-1: Subcat E-1	Runoff Area=101,514 sf 37.70% Impervious Runoff Depth=2.03" Flow Length=671' Tc=8.6 min CN=WQ Runoff=4.12 cfs 17,193 cf
Subcatchment E-2: Subcat E-2	Runoff Area=65,426 sf 33.91% Impervious Runoff Depth=1.94" Flow Length=485' Tc=8.1 min CN=WQ Runoff=2.57 cfs 10,597 cf
Subcatchment E-3: Subcat E-3	Runoff Area=17,073 sf 0.00% Impervious Runoff Depth=0.11" Flow Length=170' Tc=10.4 min CN=WQ Runoff=0.01 cfs 151 cf
Subcatchment E-4: Subcat E-4	Runoff Area=111,271 sf 4.45% Impervious Runoff Depth=0.24" Flow Length=570' Tc=19.9 min CN=WQ Runoff=0.40 cfs 2,207 cf
Subcatchment E-5: Subcat E-5	Runoff Area=68,461 sf 0.00% Impervious Runoff Depth=0.16" Flow Length=351' Tc=8.1 min CN=WQ Runoff=0.11 cfs 899 cf
Subcatchment E-6: Subcat E-6	Runoff Area=503,736 sf 8.45% Impervious Runoff Depth=0.69" Flow Length=710' Tc=14.7 min UI Adjusted CN=WQ Runoff=5.75 cfs 28,861 cf
Pond LP-1: Low Point - MAHW	Peak Elev=268.47' Storage=347 cf Inflow=4.12 cfs 17,193 cf Outflow=4.10 cfs 17,097 cf
Pond LP-2: Low Point - MAHW	Peak Elev=267.77' Storage=1,158 cf Inflow=6.65 cfs 27,695 cf 24.0" Round Culvert n=0.011 L=30.0' S=-0.0160 '/' Outflow=6.28 cfs 27,695 cf
Pond LP-3: Low Point - Depression	Peak Elev=271.28' Storage=30 cf Inflow=0.01 cfs 151 cf Discarded=0.00 cfs 151 cf Primary=0.00 cfs 0 cf Outflow=0.00 cfs 151 cf
Pond LP-4: Low Point - IVW	Peak Elev=271.51' Storage=42 cf Inflow=0.40 cfs 2,207 cf Outflow=0.40 cfs 2,169 cf
Pond LP-5: Low Point - IVW	Peak Elev=271.19' Storage=899 cf Inflow=0.11 cfs 899 cf Outflow=0.00 cfs 0 cf
Link SP1: On Site Flow To Perennial Stream	Inflow=4.10 cfs 17,097 cf Primary=4.10 cfs 17,097 cf
Link SP2: On Site Flow To Perennial Stream	Inflow=6.28 cfs 27,695 cf Primary=6.28 cfs 27,695 cf
Link SP3: On Site Flow To Low-Spot	Inflow=12.22 cfs 58,725 cf Primary=12.22 cfs 58,725 cf

Total Runoff Area = 867,482 sf Runoff Volume = 59,909 cf Average Runoff Depth = 0.83"
87.55% Pervious = 759,513 sf 12.45% Impervious = 107,969 sf

Summary for Subcatchment E-1: Subcat E-1

Runoff = 4.12 cfs @ 12.12 hrs, Volume= 17,193 cf, Depth= 2.03"
Routed to Pond LP-1 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.22"

Area (sf)	CN	Description
63,241	39	>75% Grass cover, Good, HSG A
26,126	98	Paved parking, HSG A
11,715	98	Unconnected roofs, HSG A
431	98	Water Surface, HSG A
101,514		Weighted Average
63,241	39	62.30% Pervious Area
38,273	98	37.70% Impervious Area
11,715		30.61% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.9	50	0.1000	0.29		Sheet Flow, A-B Grass: Short n= 0.150 P2= 3.37"
2.1	195	0.0500	1.57		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
0.1	30	0.0300	3.52		Shallow Concentrated Flow, C-D Paved Kv= 20.3 fps
1.8	166	0.0480	1.53		Shallow Concentrated Flow, D-E Short Grass Pasture Kv= 7.0 fps
0.1	30	0.0300	3.52		Shallow Concentrated Flow, E-F Paved Kv= 20.3 fps
1.1	100	0.0500	1.57		Shallow Concentrated Flow, F-G Short Grass Pasture Kv= 7.0 fps
0.5	100	0.0330	3.69		Shallow Concentrated Flow, G-H Paved Kv= 20.3 fps
8.6	671	Total			

Summary for Subcatchment E-2: Subcat E-2

Runoff = 2.57 cfs @ 12.11 hrs, Volume= 10,597 cf, Depth= 1.94"
Routed to Pond LP-2 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.22"

Area (sf)	CN	Description
41,920	39	>75% Grass cover, Good, HSG A
1,317	96	Gravel surface, HSG A
14,955	98	Paved parking, HSG A
4,757	98	Unconnected roofs, HSG A
2,477	98	Water Surface, HSG A
65,426		Weighted Average
43,237	41	66.09% Pervious Area
22,189	98	33.91% Impervious Area
4,757		21.44% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.9	50	0.1000	0.29		Sheet Flow, A-B Grass: Short n= 0.150 P2= 3.37"
0.6	60	0.0600	1.71		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
0.3	55	0.0200	2.87		Shallow Concentrated Flow, C-D Paved Kv= 20.3 fps
0.8	60	0.0330	1.27		Shallow Concentrated Flow, D-E Short Grass Pasture Kv= 7.0 fps
1.2	50	0.0200	0.71		Shallow Concentrated Flow, E-F Woodland Kv= 5.0 fps
0.8	50	0.0200	0.99		Shallow Concentrated Flow, F-G Short Grass Pasture Kv= 7.0 fps
1.3	130	0.0070	1.70		Shallow Concentrated Flow, G-H Paved Kv= 20.3 fps
0.2	30	0.1000	2.85		Shallow Concentrated Flow, H-I Cultivated Straight Rows Kv= 9.0 fps
8.1	485	Total			

Summary for Subcatchment E-3: Subcat E-3

Runoff = 0.01 cfs @ 12.51 hrs, Volume= 151 cf, Depth= 0.11"
Routed to Pond LP-3 : Low Point - Depression

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.22"

Area (sf)	CN	Description
6,828	39	>75% Grass cover, Good, HSG A
10,245	30	Woods, Good, HSG A
17,073		Weighted Average
17,073	34	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	50	0.0200	0.10		Sheet Flow, A-B Grass: Dense n= 0.240 P2= 3.37"
1.6	60	0.0160	0.63		Shallow Concentrated Flow, B-C Woodland Kv= 5.0 fps
0.8	60	0.0600	1.22		Shallow Concentrated Flow, C-D Woodland Kv= 5.0 fps
10.4	170	Total			

Summary for Subcatchment E-4: Subcat E-4

Runoff = 0.40 cfs @ 12.27 hrs, Volume= 2,207 cf, Depth= 0.24"
Routed to Pond LP-4 : Low Point - IVW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.22"

Area (sf)	CN	Description
1,874	39	>75% Grass cover, Good, HSG A
4,954	98	Paved parking, HSG A
104,443	30	Woods, Good, HSG A
111,271		Weighted Average
106,317	30	95.55% Pervious Area
4,954	98	4.45% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	38	0.0200	1.16		Sheet Flow, A-B Smooth surfaces n= 0.011 P2= 3.37"
1.1	12	0.0600	0.18		Sheet Flow, B-C Grass: Short n= 0.150 P2= 3.37"
18.3	520	0.0090	0.47		Shallow Concentrated Flow, C-D Woodland Kv= 5.0 fps
19.9	570	Total			

Summary for Subcatchment E-5: Subcat E-5

Runoff = 0.11 cfs @ 12.11 hrs, Volume= 899 cf, Depth= 0.16"
Routed to Pond LP-5 : Low Point - IVW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.22"

Area (sf)	CN	Description
20,858	39	>75% Grass cover, Good, HSG A
1,061	96	Gravel surface, HSG A
46,542	30	Woods, Good, HSG A
68,461		Weighted Average
68,461	34	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.7	50	0.0300	0.18		Sheet Flow, A-B Grass: Short n= 0.150 P2= 3.37"
0.2	16	0.0300	1.21		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
3.2	285	0.0900	1.50		Shallow Concentrated Flow, C-D Woodland Kv= 5.0 fps
8.1	351	Total			

Summary for Subcatchment E-6: Subcat E-6

Runoff = 5.75 cfs @ 12.20 hrs, Volume= 28,861 cf, Depth= 0.69"
Routed to Link SP3 : On Site Flow To Low-Spot

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10-year Rainfall=5.22"

Area (sf)	CN	Adj	Description
115,578	39	39	>75% Grass cover, Good, HSG A
21,390	96	96	Gravel surface, HSG A
6,237	98	98	Paved parking, HSG A
5,936	98	98	Unconnected roofs, HSG A
30,379	98	98	Water Surface, HSG A
324,216	30	30	Woods, Good, HSG A
503,736			Weighted Average
461,184	35	35	91.55% Pervious Area
42,553	98	98	8.45% Impervious Area
5,936			13.95% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.9	50	0.1200	0.21		Sheet Flow, A-B Grass: Dense n= 0.240 P2= 3.37"
0.4	60	0.1500	2.71		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
3.3	330	0.0570	1.67		Shallow Concentrated Flow, C-D Short Grass Pasture Kv= 7.0 fps
3.8	115	0.0100	0.50		Shallow Concentrated Flow, D-E Woodland Kv= 5.0 fps
3.1	130	0.0100	0.70		Shallow Concentrated Flow, E-F Short Grass Pasture Kv= 7.0 fps
0.2	25	0.0800	1.98		Shallow Concentrated Flow, F-G Short Grass Pasture Kv= 7.0 fps
14.7	710	Total			

Summary for Pond LP-1: Low Point - MAHW

Inflow Area = 101,514 sf, 37.70% Impervious, Inflow Depth = 2.03" for 10-year event
Inflow = 4.12 cfs @ 12.12 hrs, Volume= 17,193 cf
Outflow = 4.10 cfs @ 12.12 hrs, Volume= 17,097 cf, Atten= 0%, Lag= 0.5 min
Primary = 4.10 cfs @ 12.12 hrs, Volume= 17,097 cf
Routed to Link SP1 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 268.47' @ 12.12 hrs Surf.Area= 297 sf Storage= 347 cf

Plug-Flow detention time= 11.9 min calculated for 17,093 cf (99% of inflow)
Center-of-Mass det. time= 8.3 min (776.8 - 768.5)

Volume	Invert	Avail.Storage	Storage	Description
#1	267.00'	902 cf		Custom Stage Data (Irregular) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
267.00	175	160.0	0	0	175
268.00	258	168.0	215	215	441
269.00	344	172.0	300	515	642
270.00	431	176.0	387	902	848

Device	Routing	Invert	Outlet Devices
#1	Primary	267.93'	24.0" Round RCP_Round 24" L= 59.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 267.72' / 267.93' S= -0.0036 '/' Cc= 0.900 n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf
#2	Primary	267.48'	24.0" Round RCP_Round 24" L= 59.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 267.48' / 267.48' S= 0.0000 '/' Cc= 0.900 n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf

Primary OutFlow Max=4.10 cfs @ 12.12 hrs HW=268.47' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Barrel Controls 1.46 cfs @ 2.01 fps)
2=RCP_Round 24" (Barrel Controls 2.63 cfs @ 2.47 fps)

Summary for Pond LP-2: Low Point - MAHW

Inflow Area = 166,941 sf, 36.22% Impervious, Inflow Depth = 1.99" for 10-year event
Inflow = 6.65 cfs @ 12.12 hrs, Volume= 27,695 cf
Outflow = 6.28 cfs @ 12.15 hrs, Volume= 27,695 cf, Atten= 6%, Lag= 2.0 min
Primary = 6.28 cfs @ 12.15 hrs, Volume= 27,695 cf
Routed to Link SP2 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 267.77' @ 12.15 hrs Surf.Area= 1,573 sf Storage= 1,158 cf

Plug-Flow detention time= 4.8 min calculated for 27,687 cf (100% of inflow)
Center-of-Mass det. time= 4.8 min (779.1 - 774.3)

Volume	Invert	Avail.Storage	Storage Description		
#1	266.85'	5,666 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
266.85	0	0.0	0	0	0
267.00	1,276	387.0	64	64	11,918
268.00	1,668	396.0	1,468	1,531	12,602
269.00	2,069	404.0	1,865	3,396	13,250
270.00	2,477	412.0	2,270	5,666	13,910

Device	Routing	Invert	Outlet Devices
#1	Primary	266.85'	24.0" Round RCP_Round 24" L= 30.0' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 266.37' / 266.85' S= -0.0160 ' / Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=6.28 cfs @ 12.15 hrs HW=267.77' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Inlet Controls 6.28 cfs @ 4.45 fps)

Summary for Pond LP-3: Low Point - Depression

Inflow Area = 17,073 sf, 0.00% Impervious, Inflow Depth = 0.11" for 10-year event
Inflow = 0.01 cfs @ 12.51 hrs, Volume= 151 cf
Outflow = 0.00 cfs @ 15.99 hrs, Volume= 151 cf, Atten= 58%, Lag= 209.2 min
Discarded = 0.00 cfs @ 15.99 hrs, Volume= 151 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Link SP3 : On Site Flow To Low-Spot

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Peak Elev= 271.28' @ 15.99 hrs Surf.Area= 158 sf Storage= 30 cf

Plug-Flow detention time= 105.5 min calculated for 151 cf (100% of inflow)
Center-of-Mass det. time= 105.5 min (1,124.3 - 1,018.7)

Volume	Invert	Avail.Storage	Storage Description		
#1	271.00'	3,637 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.00	65	35.0	0	0	65
272.00	588	106.0	283	283	865
273.00	1,595	174.0	1,050	1,333	2,386
274.00	3,094	228.0	2,303	3,637	4,125

Device	Routing	Invert	Outlet Devices
#1	Primary	273.87'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 0.13 0.43 Width (feet) 0.00 17.00 64.00
#2	Discarded	271.00'	1.040 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.00 cfs @ 15.99 hrs HW=271.28' (Free Discharge)
2=Exfiltration (Exfiltration Controls 0.00 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=271.00' TW=0.00' (Dynamic Tailwater)
1=Custom Weir/Orifice (Controls 0.00 cfs)

Summary for Pond LP-4: Low Point - IVW

Inflow Area = 111,271 sf, 4.45% Impervious, Inflow Depth = 0.24" for 10-year event
Inflow = 0.40 cfs @ 12.27 hrs, Volume= 2,207 cf
Outflow = 0.40 cfs @ 12.26 hrs, Volume= 2,169 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.40 cfs @ 12.26 hrs, Volume= 2,169 cf
Routed to Link SP3 : On Site Flow To Low-Spot

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 271.51' @ 12.26 hrs Surf.Area= 409 sf Storage= 42 cf

Plug-Flow detention time= 22.2 min calculated for 2,169 cf (98% of inflow)
Center-of-Mass det. time= 10.9 min (801.0 - 790.1)

Volume	Invert	Avail.Storage	Storage Description		
#1	271.20'	732 cf	Low Point 4 (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.20	0	0.0	0	0	0
272.00	2,745	222.4	732	732	3,937

Device	Routing	Invert	Outlet Devices
#1	Primary	271.50'	200.0' long x 3.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68 2.72 2.81 2.92 2.97 3.07 3.32
#2	Primary	271.50'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 0.50 Width (feet) 0.00 40.00

Primary OutFlow Max=0.40 cfs @ 12.26 hrs HW=271.51' TW=0.00' (Dynamic Tailwater)
1=Broad-Crested Rectangular Weir (Weir Controls 0.40 cfs @ 0.23 fps)
2=Custom Weir/Orifice (Weir Controls 0.00 cfs @ 0.24 fps)

Summary for Pond LP-5: Low Point - IWV

Inflow Area = 68,461 sf, 0.00% Impervious, Inflow Depth = 0.16" for 10-year event
Inflow = 0.11 cfs @ 12.11 hrs, Volume= 899 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 100%, Lag= 0.0 min
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Link SP3 : On Site Flow To Low-Spot

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 271.19' @ 24.47 hrs Surf.Area= 5,476 sf Storage= 899 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	271.00'	4,025 cf	Low Point 5.1 (Irregular) Listed below (Recalc)
#2	271.00'	4,810 cf	Low Point 5.2 (Irregular) Listed below (Recalc)
		8,835 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.00	1,533	173.1	0	0	1,533
272.00	7,215	468.2	4,025	4,025	16,596

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.00	2,382	270.9	0	0	2,382
272.00	7,752	387.7	4,810	4,810	8,512

Device	Routing	Invert	Outlet Devices
#1	Primary	271.50'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 0.50 Width (feet) 0.00 58.90

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=271.00' TW=0.00' (Dynamic Tailwater)
1=Custom Weir/Orifice (Controls 0.00 cfs)

Summary for Link SP1: On Site Flow To Perennial Stream

Inflow Area = 101,514 sf, 37.70% Impervious, Inflow Depth = 2.02" for 10-year event
Inflow = 4.10 cfs @ 12.12 hrs, Volume= 17,097 cf
Primary = 4.10 cfs @ 12.12 hrs, Volume= 17,097 cf, Atten= 0%, Lag= 0.0 min
Routed to Pond LP-2 : Low Point - MAHW

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP2: On Site Flow To Perennial Stream

Inflow Area = 166,941 sf, 36.22% Impervious, Inflow Depth = 1.99" for 10-year event
Inflow = 6.28 cfs @ 12.15 hrs, Volume= 27,695 cf
Primary = 6.28 cfs @ 12.15 hrs, Volume= 27,695 cf, Atten= 0%, Lag= 0.0 min
Routed to Link SP3 : On Site Flow To Low-Spot

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP3: On Site Flow To Low-Spot

Inflow Area = 867,482 sf, 12.45% Impervious, Inflow Depth = 0.81" for 10-year event
Inflow = 12.22 cfs @ 12.17 hrs, Volume= 58,725 cf
Primary = 12.22 cfs @ 12.17 hrs, Volume= 58,725 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment E-1: Subcat E-1	Runoff Area=101,514 sf 37.70% Impervious Runoff Depth=3.75" Flow Length=671' Tc=8.6 min CN=WQ Runoff=7.61 cfs 31,715 cf
Subcatchment E-2: Subcat E-2	Runoff Area=65,426 sf 33.91% Impervious Runoff Depth=3.63" Flow Length=485' Tc=8.1 min CN=WQ Runoff=4.81 cfs 19,767 cf
Subcatchment E-3: Subcat E-3	Runoff Area=17,073 sf 0.00% Impervious Runoff Depth=0.76" Flow Length=170' Tc=10.4 min CN=WQ Runoff=0.13 cfs 1,085 cf
Subcatchment E-4: Subcat E-4	Runoff Area=111,271 sf 4.45% Impervious Runoff Depth=0.80" Flow Length=570' Tc=19.9 min CN=WQ Runoff=0.68 cfs 7,420 cf
Subcatchment E-5: Subcat E-5	Runoff Area=68,461 sf 0.00% Impervious Runoff Depth=0.80" Flow Length=351' Tc=8.1 min CN=WQ Runoff=0.60 cfs 4,572 cf
Subcatchment E-6: Subcat E-6	Runoff Area=503,736 sf 8.45% Impervious Runoff Depth=1.57" Flow Length=710' Tc=14.7 min UI Adjusted CN=WQ Runoff=10.82 cfs 65,862 cf
Pond LP-1: Low Point - MAHW	Peak Elev=268.79' Storage=443 cf Inflow=7.61 cfs 31,715 cf Outflow=7.59 cfs 31,620 cf
Pond LP-2: Low Point - MAHW	Peak Elev=268.25' Storage=1,965 cf Inflow=12.37 cfs 51,386 cf 24.0" Round Culvert n=0.011 L=30.0' S=-0.0160 '/' Outflow=11.39 cfs 51,386 cf
Pond LP-3: Low Point - Depression	Peak Elev=272.27' Storage=470 cf Inflow=0.13 cfs 1,085 cf Discarded=0.02 cfs 1,082 cf Primary=0.00 cfs 0 cf Outflow=0.02 cfs 1,082 cf
Pond LP-4: Low Point - IVW	Peak Elev=271.51' Storage=44 cf Inflow=0.68 cfs 7,420 cf Outflow=0.68 cfs 7,382 cf
Pond LP-5: Low Point - IVW	Peak Elev=271.54' Storage=3,400 cf Inflow=0.60 cfs 4,572 cf Outflow=0.05 cfs 1,464 cf
Link SP1: On Site Flow To Perennial Stream	Inflow=7.59 cfs 31,620 cf Primary=7.59 cfs 31,620 cf
Link SP2: On Site Flow To Perennial Stream	Inflow=11.39 cfs 51,386 cf Primary=11.39 cfs 51,386 cf
Link SP3: On Site Flow To Low-Spot	Inflow=22.52 cfs 126,094 cf Primary=22.52 cfs 126,094 cf

Total Runoff Area = 867,482 sf Runoff Volume = 130,421 cf Average Runoff Depth = 1.80"
87.55% Pervious = 759,513 sf 12.45% Impervious = 107,969 sf

Summary for Subcatchment E-1: Subcat E-1

Runoff = 7.61 cfs @ 12.12 hrs, Volume= 31,715 cf, Depth= 3.75"
Routed to Pond LP-1 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=8.16"

Area (sf)	CN	Description
63,241	39	>75% Grass cover, Good, HSG A
26,126	98	Paved parking, HSG A
11,715	98	Unconnected roofs, HSG A
431	98	Water Surface, HSG A
101,514		Weighted Average
63,241	39	62.30% Pervious Area
38,273	98	37.70% Impervious Area
11,715		30.61% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.9	50	0.1000	0.29		Sheet Flow, A-B Grass: Short n= 0.150 P2= 3.37"
2.1	195	0.0500	1.57		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
0.1	30	0.0300	3.52		Shallow Concentrated Flow, C-D Paved Kv= 20.3 fps
1.8	166	0.0480	1.53		Shallow Concentrated Flow, D-E Short Grass Pasture Kv= 7.0 fps
0.1	30	0.0300	3.52		Shallow Concentrated Flow, E-F Paved Kv= 20.3 fps
1.1	100	0.0500	1.57		Shallow Concentrated Flow, F-G Short Grass Pasture Kv= 7.0 fps
0.5	100	0.0330	3.69		Shallow Concentrated Flow, G-H Paved Kv= 20.3 fps
8.6	671	Total			

Summary for Subcatchment E-2: Subcat E-2

Runoff = 4.81 cfs @ 12.12 hrs, Volume= 19,767 cf, Depth= 3.63"
Routed to Pond LP-2 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=8.16"

Area (sf)	CN	Description
41,920	39	>75% Grass cover, Good, HSG A
1,317	96	Gravel surface, HSG A
14,955	98	Paved parking, HSG A
4,757	98	Unconnected roofs, HSG A
2,477	98	Water Surface, HSG A
65,426		Weighted Average
43,237	41	66.09% Pervious Area
22,189	98	33.91% Impervious Area
4,757		21.44% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.9	50	0.1000	0.29		Sheet Flow, A-B Grass: Short n= 0.150 P2= 3.37"
0.6	60	0.0600	1.71		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
0.3	55	0.0200	2.87		Shallow Concentrated Flow, C-D Paved Kv= 20.3 fps
0.8	60	0.0330	1.27		Shallow Concentrated Flow, D-E Short Grass Pasture Kv= 7.0 fps
1.2	50	0.0200	0.71		Shallow Concentrated Flow, E-F Woodland Kv= 5.0 fps
0.8	50	0.0200	0.99		Shallow Concentrated Flow, F-G Short Grass Pasture Kv= 7.0 fps
1.3	130	0.0070	1.70		Shallow Concentrated Flow, G-H Paved Kv= 20.3 fps
0.2	30	0.1000	2.85		Shallow Concentrated Flow, H-I Cultivated Straight Rows Kv= 9.0 fps
8.1	485	Total			

Summary for Subcatchment E-3: Subcat E-3

Runoff = 0.13 cfs @ 12.21 hrs, Volume= 1,085 cf, Depth= 0.76"
Routed to Pond LP-3 : Low Point - Depression

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=8.16"

Area (sf)	CN	Description
6,828	39	>75% Grass cover, Good, HSG A
10,245	30	Woods, Good, HSG A
17,073		Weighted Average
17,073	34	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0	50	0.0200	0.10		Sheet Flow, A-B Grass: Dense n= 0.240 P2= 3.37"
1.6	60	0.0160	0.63		Shallow Concentrated Flow, B-C Woodland Kv= 5.0 fps
0.8	60	0.0600	1.22		Shallow Concentrated Flow, C-D Woodland Kv= 5.0 fps
10.4	170	Total			

Summary for Subcatchment E-4: Subcat E-4

Runoff = 0.68 cfs @ 12.32 hrs, Volume= 7,420 cf, Depth= 0.80"
Routed to Pond LP-4 : Low Point - IVW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=8.16"

Area (sf)	CN	Description
1,874	39	>75% Grass cover, Good, HSG A
4,954	98	Paved parking, HSG A
104,443	30	Woods, Good, HSG A
111,271		Weighted Average
106,317	30	95.55% Pervious Area
4,954	98	4.45% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	38	0.0200	1.16		Sheet Flow, A-B Smooth surfaces n= 0.011 P2= 3.37"
1.1	12	0.0600	0.18		Sheet Flow, B-C Grass: Short n= 0.150 P2= 3.37"
18.3	520	0.0090	0.47		Shallow Concentrated Flow, C-D Woodland Kv= 5.0 fps
19.9	570	Total			

Summary for Subcatchment E-5: Subcat E-5

Runoff = 0.60 cfs @ 12.14 hrs, Volume= 4,572 cf, Depth= 0.80"
Routed to Pond LP-5 : Low Point - IVW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=8.16"

Area (sf)	CN	Description
20,858	39	>75% Grass cover, Good, HSG A
1,061	96	Gravel surface, HSG A
46,542	30	Woods, Good, HSG A
68,461		Weighted Average
68,461	34	100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.7	50	0.0300	0.18		Sheet Flow, A-B Grass: Short n= 0.150 P2= 3.37"
0.2	16	0.0300	1.21		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
3.2	285	0.0900	1.50		Shallow Concentrated Flow, C-D Woodland Kv= 5.0 fps
8.1	351	Total			

Summary for Subcatchment E-6: Subcat E-6

Runoff = 10.82 cfs @ 12.20 hrs, Volume= 65,862 cf, Depth= 1.57"
Routed to Link SP3 : On Site Flow To Low-Spot

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100-year Rainfall=8.16"

Area (sf)	CN	Adj	Description
115,578	39	39	>75% Grass cover, Good, HSG A
21,390	96	96	Gravel surface, HSG A
6,237	98	98	Paved parking, HSG A
5,936	98	98	Unconnected roofs, HSG A
30,379	98	98	Water Surface, HSG A
324,216	30	30	Woods, Good, HSG A
503,736			Weighted Average
461,184	35	35	91.55% Pervious Area
42,553	98	98	8.45% Impervious Area
5,936			13.95% Unconnected

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.9	50	0.1200	0.21		Sheet Flow, A-B Grass: Dense n= 0.240 P2= 3.37"
0.4	60	0.1500	2.71		Shallow Concentrated Flow, B-C Short Grass Pasture Kv= 7.0 fps
3.3	330	0.0570	1.67		Shallow Concentrated Flow, C-D Short Grass Pasture Kv= 7.0 fps
3.8	115	0.0100	0.50		Shallow Concentrated Flow, D-E Woodland Kv= 5.0 fps
3.1	130	0.0100	0.70		Shallow Concentrated Flow, E-F Short Grass Pasture Kv= 7.0 fps
0.2	25	0.0800	1.98		Shallow Concentrated Flow, F-G Short Grass Pasture Kv= 7.0 fps
14.7	710	Total			

Summary for Pond LP-1: Low Point - MAHW

Inflow Area = 101,514 sf, 37.70% Impervious, Inflow Depth = 3.75" for 100-year event
Inflow = 7.61 cfs @ 12.12 hrs, Volume= 31,715 cf
Outflow = 7.59 cfs @ 12.13 hrs, Volume= 31,620 cf, Atten= 0%, Lag= 0.4 min
Primary = 7.59 cfs @ 12.13 hrs, Volume= 31,620 cf
Routed to Link SP1 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 268.79' @ 12.13 hrs Surf.Area= 324 sf Storage= 443 cf

Plug-Flow detention time= 7.4 min calculated for 31,611 cf (100% of inflow)
Center-of-Mass det. time= 5.5 min (783.2 - 777.7)

Volume	Invert	Avail.Storage	Storage	Description
#1	267.00'	902 cf		Custom Stage Data (Irregular) Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
267.00	175	160.0	0	0	175
268.00	258	168.0	215	215	441
269.00	344	172.0	300	515	642
270.00	431	176.0	387	902	848

Device	Routing	Invert	Outlet Devices
#1	Primary	267.93'	24.0" Round RCP_Round 24" L= 59.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 267.72' / 267.93' S= -0.0036 '/' Cc= 0.900 n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf
#2	Primary	267.48'	24.0" Round RCP_Round 24" L= 59.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 267.48' / 267.48' S= 0.0000 '/' Cc= 0.900 n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf

Primary OutFlow Max=7.59 cfs @ 12.13 hrs HW=268.79' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Barrel Controls 3.04 cfs @ 2.59 fps)
2=RCP_Round 24" (Barrel Controls 4.55 cfs @ 2.98 fps)

Summary for Pond LP-2: Low Point - MAHW

Inflow Area = 166,941 sf, 36.22% Impervious, Inflow Depth = 3.69" for 100-year event
Inflow = 12.37 cfs @ 12.13 hrs, Volume= 51,386 cf
Outflow = 11.39 cfs @ 12.17 hrs, Volume= 51,386 cf, Atten= 8%, Lag= 2.5 min
Primary = 11.39 cfs @ 12.17 hrs, Volume= 51,386 cf
Routed to Link SP2 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 268.25' @ 12.17 hrs Surf.Area= 1,765 sf Storage= 1,965 cf

Plug-Flow detention time= 4.7 min calculated for 51,372 cf (100% of inflow)
Center-of-Mass det. time= 4.7 min (786.6 - 781.9)

Volume	Invert	Avail.Storage	Storage Description		
#1	266.85'	5,666 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
266.85	0	0.0	0	0	0
267.00	1,276	387.0	64	64	11,918
268.00	1,668	396.0	1,468	1,531	12,602
269.00	2,069	404.0	1,865	3,396	13,250
270.00	2,477	412.0	2,270	5,666	13,910

Device	Routing	Invert	Outlet Devices
#1	Primary	266.85'	24.0" Round RCP_Round 24" L= 30.0' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 266.37' / 266.85' S= -0.0160 ' / Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=11.38 cfs @ 12.17 hrs HW=268.25' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Barrel Controls 11.38 cfs @ 4.80 fps)

Summary for Pond LP-3: Low Point - Depression

Inflow Area = 17,073 sf, 0.00% Impervious, Inflow Depth = 0.76" for 100-year event
Inflow = 0.13 cfs @ 12.21 hrs, Volume= 1,085 cf
Outflow = 0.02 cfs @ 16.90 hrs, Volume= 1,082 cf, Atten= 85%, Lag= 281.2 min
Discarded = 0.02 cfs @ 16.90 hrs, Volume= 1,082 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Link SP3 : On Site Flow To Low-Spot

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Peak Elev= 272.27' @ 16.90 hrs Surf.Area= 810 sf Storage= 470 cf

Plug-Flow detention time= 339.4 min calculated for 1,081 cf (100% of inflow)
Center-of-Mass det. time= 337.8 min (1,277.5 - 939.7)

Volume	Invert	Avail.Storage	Storage Description		
#1	271.00'	3,637 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.00	65	35.0	0	0	65
272.00	588	106.0	283	283	865
273.00	1,595	174.0	1,050	1,333	2,386
274.00	3,094	228.0	2,303	3,637	4,125

Device	Routing	Invert	Outlet Devices
#1	Primary	273.87'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 0.13 0.43 Width (feet) 0.00 17.00 64.00
#2	Discarded	271.00'	1.040 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.02 cfs @ 16.90 hrs HW=272.27' (Free Discharge)
2=Exfiltration (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=271.00' TW=0.00' (Dynamic Tailwater)
1=Custom Weir/Orifice (Controls 0.00 cfs)

Summary for Pond LP-4: Low Point - IVW

Inflow Area = 111,271 sf, 4.45% Impervious, Inflow Depth = 0.80" for 100-year event
Inflow = 0.68 cfs @ 12.32 hrs, Volume= 7,420 cf
Outflow = 0.68 cfs @ 12.32 hrs, Volume= 7,382 cf, Atten= 0%, Lag= 0.1 min
Primary = 0.68 cfs @ 12.32 hrs, Volume= 7,382 cf
Routed to Link SP3 : On Site Flow To Low-Spot

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 271.51' @ 12.32 hrs Surf.Area= 419 sf Storage= 44 cf

Plug-Flow detention time= 7.1 min calculated for 7,382 cf (99% of inflow)
Center-of-Mass det. time= 4.1 min (891.1 - 887.0)

Volume	Invert	Avail.Storage	Storage Description		
#1	271.20'	732 cf	Low Point 4 (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.20	0	0.0	0	0	0
272.00	2,745	222.4	732	732	3,937

Device	Routing	Invert	Outlet Devices
#1	Primary	271.50'	200.0' long x 3.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68 2.72 2.81 2.92 2.97 3.07 3.32
#2	Primary	271.50'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 0.50 Width (feet) 0.00 40.00

Primary OutFlow Max=0.68 cfs @ 12.32 hrs HW=271.51' TW=0.00' (Dynamic Tailwater)
1=Broad-Crested Rectangular Weir (Weir Controls 0.68 cfs @ 0.27 fps)
2=Custom Weir/Orifice (Weir Controls 0.00 cfs @ 0.29 fps)

Summary for Pond LP-5: Low Point - IWV

Inflow Area = 68,461 sf, 0.00% Impervious, Inflow Depth = 0.80" for 100-year event
Inflow = 0.60 cfs @ 12.14 hrs, Volume= 4,572 cf
Outflow = 0.05 cfs @ 18.91 hrs, Volume= 1,464 cf, Atten= 91%, Lag= 406.2 min
Primary = 0.05 cfs @ 18.91 hrs, Volume= 1,464 cf
Routed to Link SP3 : On Site Flow To Low-Spot

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Peak Elev= 271.54' @ 18.91 hrs Surf.Area= 8,997 sf Storage= 3,400 cf

Plug-Flow detention time= 571.5 min calculated for 1,463 cf (32% of inflow)
Center-of-Mass det. time= 371.7 min (1,287.9 - 916.2)

Volume	Invert	Avail.Storage	Storage Description
#1	271.00'	4,025 cf	Low Point 5.1 (Irregular) Listed below (Recalc)
#2	271.00'	4,810 cf	Low Point 5.2 (Irregular) Listed below (Recalc)
		8,835 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.00	1,533	173.1	0	0	1,533
272.00	7,215	468.2	4,025	4,025	16,596

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
271.00	2,382	270.9	0	0	2,382
272.00	7,752	387.7	4,810	4,810	8,512

Device	Routing	Invert	Outlet Devices
#1	Primary	271.50'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28) Head (feet) 0.00 0.50 Width (feet) 0.00 58.90

Primary OutFlow Max=0.05 cfs @ 18.91 hrs HW=271.54' TW=0.00' (Dynamic Tailwater)
1=Custom Weir/Orifice (Weir Controls 0.05 cfs @ 0.53 fps)

Summary for Link SP1: On Site Flow To Perennial Stream

Inflow Area = 101,514 sf, 37.70% Impervious, Inflow Depth = 3.74" for 100-year event
Inflow = 7.59 cfs @ 12.13 hrs, Volume= 31,620 cf
Primary = 7.59 cfs @ 12.13 hrs, Volume= 31,620 cf, Atten= 0%, Lag= 0.0 min
Routed to Pond LP-2 : Low Point - MAHW

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP2: On Site Flow To Perennial Stream

Inflow Area = 166,941 sf, 36.22% Impervious, Inflow Depth = 3.69" for 100-year event
Inflow = 11.39 cfs @ 12.17 hrs, Volume= 51,386 cf
Primary = 11.39 cfs @ 12.17 hrs, Volume= 51,386 cf, Atten= 0%, Lag= 0.0 min
Routed to Link SP3 : On Site Flow To Low-Spot

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP3: On Site Flow To Low-Spot

Inflow Area = 867,482 sf, 12.45% Impervious, Inflow Depth = 1.74" for 100-year event
Inflow = 22.52 cfs @ 12.18 hrs, Volume= 126,094 cf
Primary = 22.52 cfs @ 12.18 hrs, Volume= 126,094 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



SECTION 5.0 - PROPOSED DRAINAGE ANALYSIS

3317-01 - Proposed HydroCAD

Prepared by Allen & Major Associates, Inc

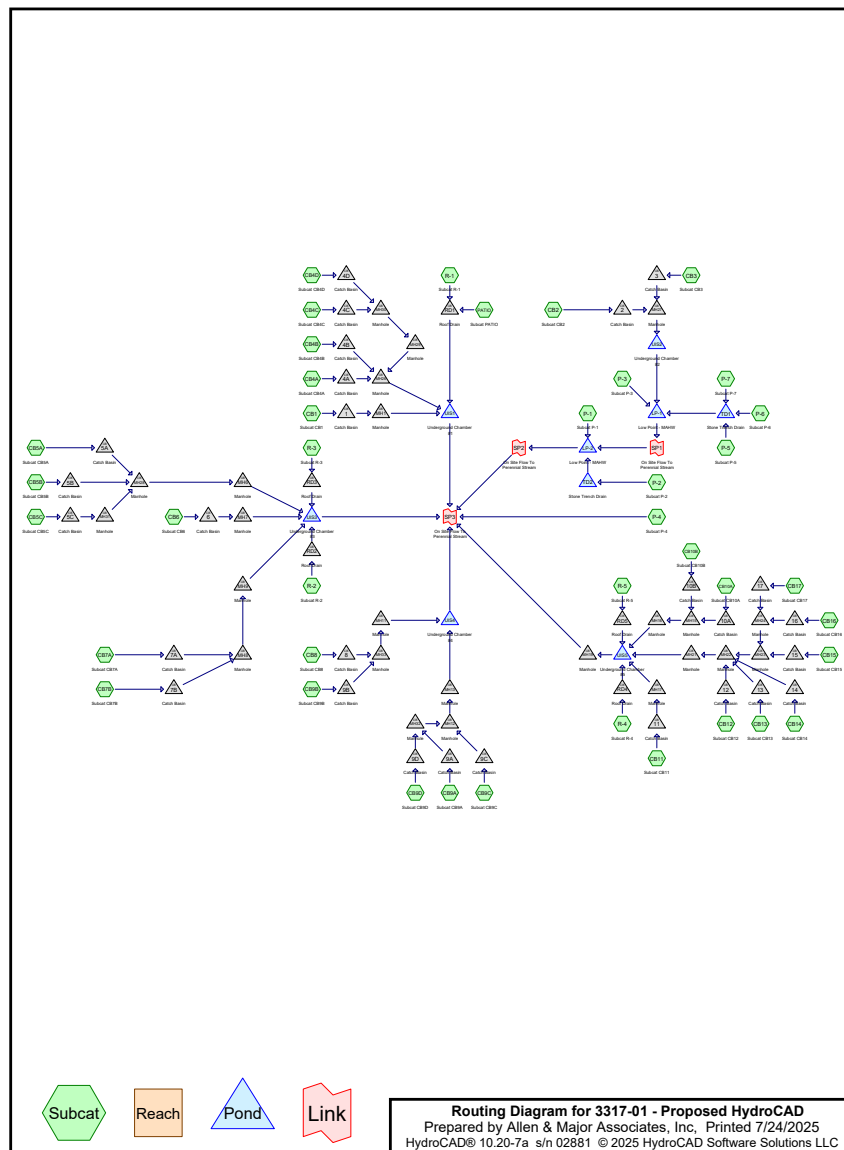
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Rainfall Events Listing

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2 Year	Type III 24-hr		Default	24.00	1	3.37	2
2	10 Year	Type III 24-hr		Default	24.00	1	5.22	2
3	100 Year	Type III 24-hr		Default	24.00	1	8.16	2



3317-01 - Proposed HydroCAD

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Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
376,527	39	>75% Grass cover, Good, HSG A (CB1, CB10A, CB10B, CB11, CB12, CB13, CB14, CB15, CB16, CB17, CB2, CB3, CB4A, CB4B, CB4C, CB4D, CB5A, CB5B, CB5C, CB6, CB7A, CB7B, CB8, CB9A, CB9B, CB9C, CB9D, P-1, P-2, P-3, P-4, P-5, P-6, P-7)
7,992	96	Gravel surface, HSG A (CB3, P-2, P-4)
190,651	98	Paved parking, HSG A (CB1, CB10A, CB10B, CB11, CB12, CB13, CB14, CB15, CB16, CB17, CB2, CB3, CB4A, CB4B, CB4C, CB4D, CB5A, CB5B, CB5C, CB6, CB7A, CB7B, CB8, CB9A, CB9B, CB9C, CB9D, P-1, P-2, P-3, P-4, P-5, P-6, P-7, PATIO)
91,695	98	Roofs, HSG A (CB14, CB15, CB17, CB4C, CB5A, CB5B, CB7B, CB8, CB9A, CB9C, R-1, R-2, R-3, R-4, R-5)
33,288	98	Water Surface, HSG A (P-1, P-3, P-4)
167,329	30	Woods, Good, HSG A (P-4)
867,482	59	TOTAL AREA

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Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
867,482	HSG A	CB1, CB10A, CB10B, CB11, CB12, CB13, CB14, CB15, CB16, CB17, CB2, CB3, CB4A, CB4B, CB4C, CB4D, CB5A, CB5B, CB5C, CB6, CB7A, CB7B, CB8, CB9A, CB9B, CB9C, CB9D, P-1, P-2, P-3, P-4, P-5, P-6, P-7, PATIO, R-1, R-2, R-3, R-4, R-5
0	HSG B	
0	HSG C	
0	HSG D	
0	Other	
867,482		TOTAL AREA

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Ground Covers (all nodes)

HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Sub Nur
376,527	0	0	0	0	376,527	>75% Grass cover, Good	
7,992	0	0	0	0	7,992	Gravel surface	
190,651	0	0	0	0	190,651	Paved parking	
91,695	0	0	0	0	91,695	Roofs	
33,288	0	0	0	0	33,288	Water Surface	
167,329	0	0	0	0	167,329	Woods, Good	
867,482	0	0	0	0	867,482	TOTAL AREA	

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Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)	Node Name
1	1	272.20	271.70	84.0	0.0060	0.012	0.0	12.0	0.0	
2	2	271.61	271.34	47.0	0.0057	0.012	0.0	12.0	0.0	
3	3	271.74	271.34	77.0	0.0052	0.012	0.0	12.0	0.0	
4	4A	272.19	272.12	7.0	0.0100	0.012	0.0	12.0	0.0	
5	4B	272.40	272.13	27.0	0.0100	0.012	0.0	12.0	0.0	
6	4C	273.25	272.78	61.0	0.0077	0.012	0.0	12.0	0.0	
7	4D	273.16	272.78	38.0	0.0100	0.012	0.0	12.0	0.0	
8	5A	273.47	273.19	28.0	0.0100	0.012	0.0	12.0	0.0	
9	5B	273.64	273.19	45.0	0.0100	0.012	0.0	12.0	0.0	
10	5C	273.90	273.49	41.0	0.0100	0.012	0.0	12.0	0.0	
11	6	272.76	272.50	26.0	0.0100	0.012	0.0	12.0	0.0	
12	7A	272.92	272.82	10.0	0.0100	0.012	0.0	12.0	0.0	
13	7B	273.22	272.82	40.0	0.0100	0.012	0.0	12.0	0.0	
14	8	273.47	273.20	27.0	0.0100	0.012	0.0	12.0	0.0	
15	9A	273.58	273.51	14.0	0.0050	0.012	0.0	12.0	0.0	
16	9B	273.58	273.20	64.0	0.0059	0.012	0.0	12.0	0.0	
17	9C	273.24	273.16	7.0	0.0114	0.012	0.0	12.0	0.0	
18	9D	273.95	273.51	49.0	0.0090	0.012	0.0	12.0	0.0	
19	10A	271.41	270.54	87.0	0.0100	0.012	0.0	12.0	0.0	
20	10B	270.69	270.54	15.0	0.0100	0.012	0.0	12.0	0.0	
21	11	270.55	270.40	15.0	0.0100	0.012	0.0	12.0	0.0	
22	12	270.74	270.60	14.0	0.0100	0.012	0.0	12.0	0.0	
23	13	271.79	270.92	87.0	0.0100	0.012	0.0	12.0	0.0	
24	14	271.25	270.92	33.0	0.0100	0.012	0.0	12.0	0.0	
25	15	272.42	272.23	19.0	0.0100	0.012	0.0	12.0	0.0	
26	16	274.29	274.10	19.0	0.0100	0.012	0.0	12.0	0.0	
27	17	274.97	274.10	143.0	0.0061	0.012	0.0	15.0	0.0	
28	LP-1	267.72	267.93	59.0	-0.0036	0.014	0.0	24.0	0.0	
29	LP-1	267.48	267.48	59.0	0.0000	0.014	0.0	24.0	0.0	
30	LP-2	266.37	266.85	30.0	-0.0160	0.011	0.0	24.0	0.0	
31	MH1	271.70	271.70	10.0	0.0000	0.012	0.0	24.0	0.0	
32	MH11	273.00	273.00	4.0	0.0000	0.012	0.0	24.0	0.0	
33	MH12	273.16	273.00	28.0	0.0057	0.012	0.0	15.0	0.0	
34	MH13	273.00	273.00	10.0	0.0000	0.012	0.0	24.0	0.0	
35	MH15	270.44	270.30	14.0	0.0100	0.012	0.0	12.0	0.0	
36	MH16	270.30	270.30	5.0	0.0000	0.012	0.0	24.0	0.0	
37	MH17	270.30	270.30	7.0	0.0000	0.012	0.0	24.0	0.0	
38	MH19	269.87	269.52	51.0	0.0069	0.012	0.0	18.0	0.0	
39	MH21	270.50	270.30	20.0	0.0100	0.012	0.0	24.0	0.0	
40	MH22	270.82	270.60	22.0	0.0100	0.012	0.0	24.0	0.0	
41	MH23	272.13	270.92	121.0	0.0100	0.012	0.0	18.0	0.0	

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Pipe Listing (all nodes) (continued)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)	Node Name
42	MH24	273.99	272.23	176.0	0.0100	0.012	0.0	18.0	0.0	
43	MH26	272.13	271.70	43.0	0.0100	0.012	0.0	18.0	0.0	
44	MH27	272.34	272.30	8.0	0.0050	0.012	0.0	15.0	0.0	
45	MH28	273.09	272.50	59.0	0.0100	0.012	0.0	15.0	0.0	
46	MH29	272.24	272.13	11.0	0.0100	0.012	0.0	15.0	0.0	
47	MH30	272.68	272.34	34.0	0.0100	0.012	0.0	15.0	0.0	
48	MH31	273.39	273.19	20.0	0.0100	0.012	0.0	12.0	0.0	
49	MH32	273.41	273.16	48.0	0.0052	0.012	0.0	12.0	0.0	
50	MH33	273.20	273.00	20.0	0.0100	0.012	0.0	15.0	0.0	
51	MH6	272.50	272.50	6.0	0.0000	0.012	0.0	24.0	0.0	
52	MH7	272.50	272.50	6.0	0.0000	0.012	0.0	24.0	0.0	
53	MH8	272.72	272.50	22.0	0.0100	0.012	0.0	15.0	0.0	
54	MH9	272.50	272.50	8.0	0.0000	0.012	0.0	24.0	0.0	
55	RD1	271.91	271.70	21.0	0.0100	0.012	0.0	12.0	0.0	
56	RD2	272.81	272.50	31.0	0.0100	0.012	0.0	12.0	0.0	
57	RD3	272.65	272.50	14.5	0.0103	0.012	0.0	12.0	0.0	
58	RD4	270.55	270.30	24.0	0.0104	0.012	0.0	12.0	0.0	
59	RD5	270.48	270.30	18.0	0.0100	0.012	0.0	15.0	0.0	
60	TD1	271.70	270.90	60.0	0.0133	0.012	0.0	12.0	0.0	
61	TD2	273.03	272.50	60.0	0.0088	0.012	0.0	12.0	0.0	
62	UIS1	271.70	269.85	139.0	0.0133	0.013	0.0	15.0	0.0	
63	UIS2	272.60	272.00	60.0	0.0100	0.013	0.0	12.0	0.0	
64	UIS3	273.29	272.00	110.0	0.0117	0.013	0.0	12.0	0.0	
65	UIS4	273.00	271.00	264.0	0.0076	0.013	0.0	12.0	0.0	
66	UIS5	273.05	272.00	97.0	0.0108	0.013	0.0	12.0	0.0	

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Type III 24-hr 2 Year Rainfall=3.37"

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Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment CB1: Subcat CB1	Runoff Area=10,622 sf 86.84% Impervious Runoff Depth=2.72" Tc=6.0 min CN=WQ Runoff=0.69 cfs 2,412 cf
Subcatchment CB10A: Subcat CB10A	Runoff Area=2,889 sf 81.34% Impervious Runoff Depth=2.55" Tc=6.0 min CN=WQ Runoff=0.18 cfs 614 cf
Subcatchment CB10B: Subcat CB10B	Runoff Area=10,414 sf 86.69% Impervious Runoff Depth=2.72" Tc=6.0 min CN=WQ Runoff=0.68 cfs 2,360 cf
Subcatchment CB11: Subcat CB11	Runoff Area=11,505 sf 85.23% Impervious Runoff Depth=2.67" Tc=6.0 min CN=WQ Runoff=0.74 cfs 2,564 cf
Subcatchment CB12: Subcat CB12	Runoff Area=7,149 sf 71.54% Impervious Runoff Depth=2.25" Tc=6.0 min CN=WQ Runoff=0.38 cfs 1,337 cf
Subcatchment CB13: Subcat CB13	Runoff Area=4,141 sf 88.54% Impervious Runoff Depth=2.78" Tc=6.0 min CN=WQ Runoff=0.28 cfs 958 cf
Subcatchment CB14: Subcat CB14	Runoff Area=6,167 sf 87.00% Impervious Runoff Depth=2.73" Tc=6.0 min CN=WQ Runoff=0.40 cfs 1,403 cf
Subcatchment CB15: Subcat CB15	Runoff Area=20,154 sf 44.65% Impervious Runoff Depth=1.40" Tc=6.0 min CN=WQ Runoff=0.68 cfs 2,356 cf
Subcatchment CB16: Subcat CB16	Runoff Area=20,584 sf 35.58% Impervious Runoff Depth=1.12" Tc=6.0 min CN=WQ Runoff=0.55 cfs 1,918 cf
Subcatchment CB17: Subcat CB17	Runoff Area=36,035 sf 27.88% Impervious Runoff Depth=0.88" Tc=6.0 min CN=WQ Runoff=0.76 cfs 2,634 cf
Subcatchment CB2: Subcat CB2	Runoff Area=12,976 sf 8.11% Impervious Runoff Depth=0.26" Tc=6.0 min CN=WQ Runoff=0.08 cfs 279 cf
Subcatchment CB3: Subcat CB3	Runoff Area=9,660 sf 44.27% Impervious Runoff Depth=2.33" Tc=6.0 min CN=WQ Runoff=0.55 cfs 1,874 cf
Subcatchment CB4A: Subcat CB4A	Runoff Area=3,976 sf 91.93% Impervious Runoff Depth=2.88" Tc=6.0 min CN=WQ Runoff=0.27 cfs 956 cf
Subcatchment CB4B: Subcat CB4B	Runoff Area=10,218 sf 73.42% Impervious Runoff Depth=2.30" Tc=6.0 min CN=WQ Runoff=0.56 cfs 1,962 cf
Subcatchment CB4C: Subcat CB4C	Runoff Area=10,741 sf 84.88% Impervious Runoff Depth=2.66" Tc=6.0 min CN=WQ Runoff=0.69 cfs 2,384 cf
Subcatchment CB4D: Subcat CB4D	Runoff Area=10,203 sf 79.98% Impervious Runoff Depth=2.51" Tc=6.0 min CN=WQ Runoff=0.61 cfs 2,134 cf

Subcatchment CB5A: Subcat CB5A	Runoff Area=10,459 sf 93.11% Impervious Runoff Depth=2.92" Tc=6.0 min CN=WQ Runoff=0.73 cfs 2,546 cf
Subcatchment CB5B: Subcat CB5B	Runoff Area=3,865 sf 95.02% Impervious Runoff Depth=2.98" Tc=6.0 min CN=WQ Runoff=0.28 cfs 960 cf
Subcatchment CB5C: Subcat CB5C	Runoff Area=2,619 sf 91.40% Impervious Runoff Depth=2.87" Tc=6.0 min CN=WQ Runoff=0.18 cfs 626 cf
Subcatchment CB6: Subcat CB6	Runoff Area=12,631 sf 77.41% Impervious Runoff Depth=2.43" Tc=6.0 min CN=WQ Runoff=0.74 cfs 2,557 cf
Subcatchment CB7A: Subcat CB7A	Runoff Area=7,101 sf 79.91% Impervious Runoff Depth=2.51" Tc=6.0 min CN=WQ Runoff=0.43 cfs 1,484 cf
Subcatchment CB7B: Subcat CB7B	Runoff Area=11,340 sf 75.16% Impervious Runoff Depth=2.36" Tc=6.0 min CN=WQ Runoff=0.64 cfs 2,229 cf
Subcatchment CB8: Subcat CB8	Runoff Area=10,991 sf 81.26% Impervious Runoff Depth=2.55" Tc=6.0 min CN=WQ Runoff=0.67 cfs 2,335 cf
Subcatchment CB9A: Subcat CB9A	Runoff Area=11,743 sf 81.25% Impervious Runoff Depth=2.55" Tc=6.0 min CN=WQ Runoff=0.72 cfs 2,495 cf
Subcatchment CB9B: Subcat CB9B	Runoff Area=14,736 sf 67.40% Impervious Runoff Depth=2.12" Tc=6.0 min CN=WQ Runoff=0.75 cfs 2,598 cf
Subcatchment CB9C: Subcat CB9C	Runoff Area=3,708 sf 100.00% Impervious Runoff Depth=3.14" Tc=6.0 min CN=WQ Runoff=0.28 cfs 969 cf
Subcatchment CB9D: Subcat CB9D	Runoff Area=1,543 sf 58.43% Impervious Runoff Depth=1.83" Tc=6.0 min CN=WQ Runoff=0.07 cfs 236 cf
Subcatchment P-1: Subcat P-1	Runoff Area=14,881 sf 16.65% Impervious Runoff Depth=0.53" Tc=6.0 min CN=WQ Runoff=0.19 cfs 651 cf
Subcatchment P-2: Subcat P-2	Runoff Area=11,308 sf 0.00% Impervious Runoff Depth=0.65" Tc=6.0 min CN=WQ Runoff=0.18 cfs 612 cf
Subcatchment P-3: Subcat P-3	Runoff Area=7,515 sf 5.81% Impervious Runoff Depth=0.19" Tc=6.0 min CN=WQ Runoff=0.03 cfs 116 cf
Subcatchment P-4: Subcat P-4	Runoff Area=291,674 sf 12.39% Impervious Runoff Depth=0.41" Tc=12.0 min CN=WQ Runoff=2.39 cfs 10,054 cf
Subcatchment P-5: Subcat P-5	Runoff Area=76,040 sf 11.69% Impervious Runoff Depth=0.37" Tc=8.0 min CN=WQ Runoff=0.63 cfs 2,344 cf
Subcatchment P-6: Subcat P-6	Runoff Area=25,354 sf 10.27% Impervious Runoff Depth=0.33" Tc=6.0 min CN=WQ Runoff=0.20 cfs 688 cf

Subcatchment P-7: Subcat P-7	Runoff Area=79,230 sf 5.41% Impervious Runoff Depth=0.17" Tc=11.4 min CN=WQ Runoff=0.27 cfs 1,144 cf
Subcatchment PATIO: Subcat PATIO	Runoff Area=2,833 sf 100.00% Impervious Runoff Depth=3.14" Tc=6.0 min CN=98 Runoff=0.21 cfs 741 cf
Subcatchment R-1: Subcat R-1	Runoff Area=5,046 sf 100.00% Impervious Runoff Depth=3.14" Tc=6.0 min CN=98 Runoff=0.38 cfs 1,319 cf
Subcatchment R-2: Subcat R-2	Runoff Area=18,290 sf 100.00% Impervious Runoff Depth=3.14" Tc=6.0 min CN=98 Runoff=1.38 cfs 4,781 cf
Subcatchment R-3: Subcat R-3	Runoff Area=18,290 sf 100.00% Impervious Runoff Depth=3.14" Tc=6.0 min CN=98 Runoff=1.38 cfs 4,781 cf
Subcatchment R-4: Subcat R-4	Runoff Area=18,290 sf 100.00% Impervious Runoff Depth=3.14" Tc=6.0 min CN=98 Runoff=1.38 cfs 4,781 cf
Subcatchment R-5: Subcat R-5	Runoff Area=20,565 sf 100.00% Impervious Runoff Depth=3.14" Tc=6.0 min CN=98 Runoff=1.55 cfs 5,376 cf
Pond 1: Catch Basin	Peak Elev=272.68' Inflow=0.69 cfs 2,412 cf 12.0" Round Culvert n=0.012 L=84.0' S=0.0060 '/' Outflow=0.69 cfs 2,412 cf
Pond 2: Catch Basin	Peak Elev=273.81' Inflow=0.08 cfs 279 cf 12.0" Round Culvert n=0.012 L=47.0' S=0.0057 '/' Outflow=0.08 cfs 278 cf
Pond 3: Catch Basin	Peak Elev=273.81' Inflow=0.55 cfs 1,874 cf 12.0" Round Culvert n=0.012 L=77.0' S=0.0052 '/' Outflow=0.55 cfs 1,874 cf
Pond 4A: Catch Basin	Peak Elev=272.83' Inflow=0.27 cfs 956 cf 12.0" Round Culvert n=0.012 L=7.0' S=0.0100 '/' Outflow=0.27 cfs 956 cf
Pond 4B: Catch Basin	Peak Elev=272.95' Inflow=0.56 cfs 1,962 cf 12.0" Round Culvert n=0.012 L=27.0' S=0.0100 '/' Outflow=0.56 cfs 1,962 cf
Pond 4C: Catch Basin	Peak Elev=273.73' Inflow=0.69 cfs 2,384 cf 12.0" Round Culvert n=0.012 L=61.0' S=0.0077 '/' Outflow=0.69 cfs 2,384 cf
Pond 4D: Catch Basin	Peak Elev=273.62' Inflow=0.61 cfs 2,134 cf 12.0" Round Culvert n=0.012 L=38.0' S=0.0100 '/' Outflow=0.61 cfs 2,134 cf
Pond 5A: Catch Basin	Peak Elev=273.96' Inflow=0.73 cfs 2,546 cf 12.0" Round Culvert n=0.012 L=28.0' S=0.0100 '/' Outflow=0.73 cfs 2,546 cf
Pond 5B: Catch Basin	Peak Elev=273.94' Inflow=0.28 cfs 960 cf 12.0" Round Culvert n=0.012 L=45.0' S=0.0100 '/' Outflow=0.28 cfs 960 cf
Pond 5C: Catch Basin	Peak Elev=274.11' Inflow=0.18 cfs 626 cf 12.0" Round Culvert n=0.012 L=41.0' S=0.0100 '/' Outflow=0.18 cfs 626 cf

Pond 6: Catch Basin	Peak Elev=273.25' Inflow=0.74 cfs 2,557 cf 12.0" Round Culvert n=0.012 L=26.0' S=0.0100 '/' Outflow=0.74 cfs 2,557 cf
Pond 7A: Catch Basin	Peak Elev=273.39' Inflow=0.43 cfs 1,484 cf 12.0" Round Culvert n=0.012 L=10.0' S=0.0100 '/' Outflow=0.43 cfs 1,484 cf
Pond 7B: Catch Basin	Peak Elev=273.67' Inflow=0.64 cfs 2,229 cf 12.0" Round Culvert n=0.012 L=40.0' S=0.0100 '/' Outflow=0.64 cfs 2,229 cf
Pond 8: Catch Basin	Peak Elev=274.05' Inflow=0.67 cfs 2,335 cf 12.0" Round Culvert n=0.012 L=27.0' S=0.0100 '/' Outflow=0.67 cfs 2,335 cf
Pond 9A: Catch Basin	Peak Elev=274.16' Inflow=0.72 cfs 2,495 cf 12.0" Round Culvert n=0.012 L=14.0' S=0.0050 '/' Outflow=0.72 cfs 2,495 cf
Pond 9B: Catch Basin	Peak Elev=274.16' Inflow=0.75 cfs 2,598 cf 12.0" Round Culvert n=0.012 L=64.0' S=0.0059 '/' Outflow=0.75 cfs 2,598 cf
Pond 9C: Catch Basin	Peak Elev=273.83' Inflow=0.28 cfs 969 cf 12.0" Round Culvert n=0.012 L=7.0' S=0.0114 '/' Outflow=0.28 cfs 969 cf
Pond 9D: Catch Basin	Peak Elev=274.13' Inflow=0.07 cfs 236 cf 12.0" Round Culvert n=0.012 L=49.0' S=0.0090 '/' Outflow=0.07 cfs 236 cf
Pond 10A: Catch Basin	Peak Elev=271.63' Inflow=0.18 cfs 614 cf 12.0" Round Culvert n=0.012 L=87.0' S=0.0100 '/' Outflow=0.18 cfs 614 cf
Pond 10B: Catch Basin	Peak Elev=271.20' Inflow=0.68 cfs 2,360 cf 12.0" Round Culvert n=0.012 L=15.0' S=0.0100 '/' Outflow=0.68 cfs 2,360 cf
Pond 11: Catch Basin	Peak Elev=271.10' Inflow=0.74 cfs 2,564 cf 12.0" Round Culvert n=0.012 L=15.0' S=0.0100 '/' Outflow=0.74 cfs 2,564 cf
Pond 12: Catch Basin	Peak Elev=271.68' Inflow=0.38 cfs 1,337 cf 12.0" Round Culvert n=0.012 L=14.0' S=0.0100 '/' Outflow=0.38 cfs 1,337 cf
Pond 13: Catch Basin	Peak Elev=272.10' Inflow=0.28 cfs 958 cf 12.0" Round Culvert n=0.012 L=87.0' S=0.0100 '/' Outflow=0.28 cfs 958 cf
Pond 14: Catch Basin	Peak Elev=271.76' Inflow=0.40 cfs 1,403 cf 12.0" Round Culvert n=0.012 L=33.0' S=0.0100 '/' Outflow=0.40 cfs 1,403 cf
Pond 15: Catch Basin	Peak Elev=272.95' Inflow=0.68 cfs 2,356 cf 12.0" Round Culvert n=0.012 L=19.0' S=0.0100 '/' Outflow=0.68 cfs 2,356 cf
Pond 16: Catch Basin	Peak Elev=274.72' Inflow=0.55 cfs 1,918 cf 12.0" Round Culvert n=0.012 L=19.0' S=0.0100 '/' Outflow=0.55 cfs 1,918 cf
Pond 17: Catch Basin	Peak Elev=275.41' Inflow=0.76 cfs 2,634 cf 15.0" Round Culvert n=0.012 L=143.0' S=0.0061 '/' Outflow=0.76 cfs 2,634 cf

Pond LP-1: Low Point - MAHW	Peak Elev=268.04' Storage=225 cf Inflow=0.84 cfs 1,240 cf Outflow=0.83 cfs 1,145 cf
Pond LP-2: Low Point - MAHW	Peak Elev=267.16' Storage=267 cf Inflow=0.96 cfs 1,796 cf 24.0" Round Culvert n=0.011 L=30.0' S=-0.0160 '/' Outflow=0.78 cfs 1,796 cf
Pond MH1: Manhole	Peak Elev=272.15' Inflow=0.69 cfs 2,412 cf 24.0" Round Culvert n=0.012 L=10.0' S=0.0000 '/' Outflow=0.69 cfs 2,412 cf
Pond MH11: Manhole	Peak Elev=273.82' Inflow=1.42 cfs 4,933 cf 24.0" Round Culvert n=0.012 L=4.0' S=0.0000 '/' Outflow=1.42 cfs 4,933 cf
Pond MH12: Manhole	Peak Elev=273.83' Inflow=1.06 cfs 3,700 cf 15.0" Round Culvert n=0.012 L=28.0' S=0.0057 '/' Outflow=1.06 cfs 3,700 cf
Pond MH13: Manhole	Peak Elev=273.82' Inflow=1.06 cfs 3,700 cf 24.0" Round Culvert n=0.012 L=10.0' S=0.0000 '/' Outflow=1.06 cfs 3,700 cf
Pond MH15: Manhole	Peak Elev=271.10' Inflow=0.86 cfs 2,975 cf 12.0" Round Culvert n=0.012 L=14.0' S=0.0100 '/' Outflow=0.86 cfs 2,975 cf
Pond MH16: Manhole	Peak Elev=271.09' Inflow=0.86 cfs 2,975 cf 24.0" Round Culvert n=0.012 L=5.0' S=0.0000 '/' Outflow=0.86 cfs 2,975 cf
Pond MH17: Manhole	Peak Elev=271.09' Inflow=0.74 cfs 2,564 cf 24.0" Round Culvert n=0.012 L=7.0' S=0.0000 '/' Outflow=0.74 cfs 2,564 cf
Pond MH19: Manhole	Peak Elev=269.87' Inflow=0.00 cfs 0 cf 18.0" Round Culvert n=0.012 L=51.0' S=0.0069 '/' Outflow=0.00 cfs 0 cf
Pond MH21: Manhole	Peak Elev=271.31' Inflow=3.05 cfs 10,607 cf 24.0" Round Culvert n=0.012 L=20.0' S=0.0100 '/' Outflow=3.05 cfs 10,607 cf
Pond MH22: Manhole	Peak Elev=271.67' Inflow=3.05 cfs 10,607 cf 24.0" Round Culvert n=0.012 L=22.0' S=0.0100 '/' Outflow=3.05 cfs 10,607 cf
Pond MH23: Manhole	Peak Elev=272.78' Inflow=1.98 cfs 6,908 cf 18.0" Round Culvert n=0.012 L=121.0' S=0.0100 '/' Outflow=1.98 cfs 6,908 cf
Pond MH24: Manhole	Peak Elev=274.50' Inflow=1.31 cfs 4,552 cf 18.0" Round Culvert n=0.012 L=176.0' S=0.0100 '/' Outflow=1.31 cfs 4,552 cf
Pond MH26: Manhole	Peak Elev=272.82' Inflow=2.14 cfs 7,435 cf 18.0" Round Culvert n=0.012 L=43.0' S=0.0100 '/' Outflow=2.14 cfs 7,435 cf
Pond MH27: Manhole	Peak Elev=273.81' Inflow=0.63 cfs 2,152 cf 15.0" Round Culvert n=0.012 L=8.0' S=0.0050 '/' Outflow=0.63 cfs 2,152 cf
Pond MH28: Manhole	Peak Elev=273.64' Inflow=1.19 cfs 4,132 cf 15.0" Round Culvert n=0.012 L=59.0' S=0.0100 '/' Outflow=1.19 cfs 4,132 cf

Pond MH29: Manhole	Peak Elev=272.98' Inflow=1.30 cfs 4,517 cf 15.0" Round Culvert n=0.012 L=11.0' S=0.0100 'l' Outflow=1.30 cfs 4,517 cf
Pond MH30: Manhole	Peak Elev=273.32' Inflow=1.30 cfs 4,517 cf 15.0" Round Culvert n=0.012 L=34.0' S=0.0100 'l' Outflow=1.30 cfs 4,517 cf
Pond MH31: Manhole	Peak Elev=273.71' Inflow=0.18 cfs 626 cf 12.0" Round Culvert n=0.012 L=20.0' S=0.0100 'l' Outflow=0.18 cfs 626 cf
Pond MH32: Manhole	Peak Elev=274.01' Inflow=0.79 cfs 2,731 cf 12.0" Round Culvert n=0.012 L=48.0' S=0.0052 'l' Outflow=0.79 cfs 2,731 cf
Pond MH33: Manhole	Peak Elev=273.89' Inflow=1.42 cfs 4,933 cf 15.0" Round Culvert n=0.012 L=20.0' S=0.0100 'l' Outflow=1.42 cfs 4,933 cf
Pond MH6: Manhole	Peak Elev=273.08' Inflow=1.19 cfs 4,132 cf 24.0" Round Culvert n=0.012 L=6.0' S=0.0000 'l' Outflow=1.19 cfs 4,132 cf
Pond MH7: Manhole	Peak Elev=272.96' Inflow=0.74 cfs 2,557 cf 24.0" Round Culvert n=0.012 L=6.0' S=0.0000 'l' Outflow=0.74 cfs 2,557 cf
Pond MH8: Manhole	Peak Elev=273.30' Inflow=1.07 cfs 3,713 cf 15.0" Round Culvert n=0.012 L=22.0' S=0.0100 'l' Outflow=1.07 cfs 3,713 cf
Pond MH9: Manhole	Peak Elev=273.05' Inflow=1.07 cfs 3,713 cf 24.0" Round Culvert n=0.012 L=8.0' S=0.0000 'l' Outflow=1.07 cfs 3,713 cf
Pond RD1: Roof Drain	Peak Elev=272.32' Inflow=0.59 cfs 2,059 cf 12.0" Round Culvert n=0.012 L=21.0' S=0.0100 'l' Outflow=0.59 cfs 2,059 cf
Pond RD2: Roof Drain	Peak Elev=273.46' Inflow=1.38 cfs 4,781 cf 12.0" Round Culvert n=0.012 L=31.0' S=0.0100 'l' Outflow=1.38 cfs 4,781 cf
Pond RD3: Roof Drain	Peak Elev=273.34' Inflow=1.38 cfs 4,781 cf 12.0" Round Culvert n=0.012 L=14.5' S=0.0103 'l' Outflow=1.38 cfs 4,781 cf
Pond RD4: Roof Drain	Peak Elev=271.21' Inflow=1.38 cfs 4,781 cf 12.0" Round Culvert n=0.012 L=24.0' S=0.0104 'l' Outflow=1.38 cfs 4,781 cf
Pond RD5: Roof Drain	Peak Elev=271.14' Inflow=1.55 cfs 5,376 cf 15.0" Round Culvert n=0.012 L=18.0' S=0.0100 'l' Outflow=1.55 cfs 5,376 cf
Pond TD1: Stone Trench Drain	Peak Elev=272.02' Storage=130 cf Inflow=1.06 cfs 4,176 cf Discarded=0.20 cfs 3,151 cf Primary=0.81 cfs 1,025 cf Outflow=1.02 cfs 4,176 cf
Pond TD2: Stone Trench Drain	Peak Elev=272.36' Storage=55 cf Inflow=0.18 cfs 612 cf Discarded=0.08 cfs 612 cf Primary=0.00 cfs 0 cf Outflow=0.08 cfs 612 cf
Pond UIS1: Underground Chamber #1	Peak Elev=272.07' Storage=2,265 cf Inflow=3.42 cfs 11,906 cf Discarded=0.84 cfs 11,906 cf Primary=0.00 cfs 0 cf Outflow=0.84 cfs 11,906 cf

Pond UIS2: Underground Chamber #2	Peak Elev=273.81' Storage=519 cf Inflow=0.63 cfs 2,152 cf Discarded=0.09 cfs 2,053 cf Primary=0.15 cfs 99 cf Outflow=0.24 cfs 2,152 cf
Pond UIS3: Underground Chamber #3	Peak Elev=272.96' Storage=4,026 cf Inflow=5.74 cfs 19,963 cf Discarded=1.33 cfs 19,963 cf Primary=0.00 cfs 0 cf Outflow=1.33 cfs 19,963 cf
Pond UIS4: Underground Chamber #4	Peak Elev=273.82' Storage=2,075 cf Inflow=2.48 cfs 8,633 cf Discarded=0.46 cfs 8,633 cf Primary=0.00 cfs 0 cf Outflow=0.46 cfs 8,633 cf
Pond UIS5: Underground Chamber #5	Peak Elev=271.09' Storage=5,561 cf Inflow=7.56 cfs 26,303 cf Discarded=1.66 cfs 26,303 cf Primary=0.00 cfs 0 cf Outflow=1.66 cfs 26,303 cf
Link SP1: On Site Flow To Perennial Stream	Inflow=0.83 cfs 1,145 cf Primary=0.83 cfs 1,145 cf
Link SP2: On Site Flow To Perennial Stream	Inflow=0.78 cfs 1,796 cf Primary=0.78 cfs 1,796 cf
Link SP3: On Site Flow To Perennial Stream	Inflow=3.08 cfs 11,850 cf Primary=3.08 cfs 11,850 cf
Total Runoff Area = 867,482 sf Runoff Volume = 84,567 cf Average Runoff Depth = 1.17" 63.61% Pervious = 551,848 sf 36.39% Impervious = 315,634 sf	

Summary for Subcatchment CB1: Subcat CB1

Runoff = 0.69 cfs @ 12.08 hrs, Volume= 2,412 cf, Depth= 2.72"
Routed to Pond 1 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
1,398	39	>75% Grass cover, Good, HSG A
9,224	98	Paved parking, HSG A
10,622		Weighted Average
1,398	39	13.16% Pervious Area
9,224	98	86.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB10A: Subcat CB10A

Runoff = 0.18 cfs @ 12.08 hrs, Volume= 614 cf, Depth= 2.55"
Routed to Pond 10A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
539	39	>75% Grass cover, Good, HSG A
2,350	98	Paved parking, HSG A
2,889		Weighted Average
539	39	18.66% Pervious Area
2,350	98	81.34% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB10B: Subcat CB10B

Runoff = 0.68 cfs @ 12.08 hrs, Volume= 2,360 cf, Depth= 2.72"
Routed to Pond 10B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
1,386	39	>75% Grass cover, Good, HSG A
9,027	98	Paved parking, HSG A
10,414		Weighted Average
1,386	39	13.31% Pervious Area
9,027	98	86.69% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB11: Subcat CB11

Runoff = 0.74 cfs @ 12.08 hrs, Volume= 2,564 cf, Depth= 2.67"
Routed to Pond 11 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
1,699	39	>75% Grass cover, Good, HSG A
9,806	98	Paved parking, HSG A
11,505		Weighted Average
1,699	39	14.77% Pervious Area
9,806	98	85.23% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB12: Subcat CB12

Runoff = 0.38 cfs @ 12.08 hrs, Volume= 1,337 cf, Depth= 2.25"
Routed to Pond 12 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
2,035	39	>75% Grass cover, Good, HSG A
5,114	98	Paved parking, HSG A
7,149		Weighted Average
2,035	39	28.46% Pervious Area
5,114	98	71.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB13: Subcat CB13

Runoff = 0.28 cfs @ 12.08 hrs, Volume= 958 cf, Depth= 2.78"
Routed to Pond 13 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
474	39	>75% Grass cover, Good, HSG A
3,666	98	Paved parking, HSG A
4,141		Weighted Average
474	39	11.46% Pervious Area
3,666	98	88.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB14: Subcat CB14

Runoff = 0.40 cfs @ 12.08 hrs, Volume= 1,403 cf, Depth= 2.73"
Routed to Pond 14 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
801	39	>75% Grass cover, Good, HSG A
4,765	98	Paved parking, HSG A
600	98	Roofs, HSG A
6,167		Weighted Average
801	39	13.00% Pervious Area
5,365	98	87.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB15: Subcat CB15

Runoff = 0.68 cfs @ 12.08 hrs, Volume= 2,356 cf, Depth= 1.40"
Routed to Pond 15 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
11,154	39	>75% Grass cover, Good, HSG A
8,024	98	Paved parking, HSG A
975	98	Roofs, HSG A
20,154		Weighted Average
11,154	39	55.35% Pervious Area
9,000	98	44.65% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB16: Subcat CB16

Runoff = 0.55 cfs @ 12.08 hrs, Volume= 1,918 cf, Depth= 1.12"
Routed to Pond 16 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
13,260	39	>75% Grass cover, Good, HSG A
7,324	98	Paved parking, HSG A
20,584		Weighted Average
13,260	39	64.42% Pervious Area
7,324	98	35.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB17: Subcat CB17

Runoff = 0.76 cfs @ 12.08 hrs, Volume= 2,634 cf, Depth= 0.88"
Routed to Pond 17 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
25,990	39	>75% Grass cover, Good, HSG A
8,281	98	Paved parking, HSG A
1,764	98	Roofs, HSG A
36,035		Weighted Average
25,990	39	72.12% Pervious Area
10,045	98	27.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB2: Subcat CB2

Runoff = 0.08 cfs @ 12.08 hrs, Volume= 279 cf, Depth= 0.26"
Routed to Pond 2 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
11,923	39	>75% Grass cover, Good, HSG A
1,053	98	Paved parking, HSG A
12,976		Weighted Average
11,923	39	91.89% Pervious Area
1,053	98	8.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB3: Subcat CB3

Runoff = 0.55 cfs @ 12.08 hrs, Volume= 1,874 cf, Depth= 2.33"
Routed to Pond 3 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
2,275	39	>75% Grass cover, Good, HSG A
3,109	96	Gravel surface, HSG A
4,276	98	Paved parking, HSG A
9,660		Weighted Average
5,384	72	55.73% Pervious Area
4,276	98	44.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4A: Subcat CB4A

Runoff = 0.27 cfs @ 12.08 hrs, Volume= 956 cf, Depth= 2.88"
Routed to Pond 4A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
321	39	>75% Grass cover, Good, HSG A
3,655	98	Paved parking, HSG A
3,976		Weighted Average
321	39	8.07% Pervious Area
3,655	98	91.93% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4B: Subcat CB4B

Runoff = 0.56 cfs @ 12.08 hrs, Volume= 1,962 cf, Depth= 2.30"
Routed to Pond 4B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
2,716	39	>75% Grass cover, Good, HSG A
7,501	98	Paved parking, HSG A
10,218		Weighted Average
2,716	39	26.58% Pervious Area
7,501	98	73.42% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4C: Subcat CB4C

Runoff = 0.69 cfs @ 12.08 hrs, Volume= 2,384 cf, Depth= 2.66"
Routed to Pond 4C : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
1,624	39	>75% Grass cover, Good, HSG A
7,101	98	Paved parking, HSG A
2,016	98	Roofs, HSG A
10,741		Weighted Average
1,624	39	15.12% Pervious Area
9,117	98	84.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4D: Subcat CB4D

Runoff = 0.61 cfs @ 12.08 hrs, Volume= 2,134 cf, Depth= 2.51"
Routed to Pond 4D : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
2,043	39	>75% Grass cover, Good, HSG A
8,160	98	Paved parking, HSG A
10,203		Weighted Average
2,043	39	20.02% Pervious Area
8,160	98	79.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB5A: Subcat CB5A

Runoff = 0.73 cfs @ 12.08 hrs, Volume= 2,546 cf, Depth= 2.92"
Routed to Pond 5A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
721	39	>75% Grass cover, Good, HSG A
9,224	98	Paved parking, HSG A
514	98	Roofs, HSG A
10,459		Weighted Average
721	39	6.89% Pervious Area
9,738	98	93.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB5B: Subcat CB5B

Runoff = 0.28 cfs @ 12.08 hrs, Volume= 960 cf, Depth= 2.98"
Routed to Pond 5B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
193	39	>75% Grass cover, Good, HSG A
2,913	98	Paved parking, HSG A
760	98	Roofs, HSG A
3,865		Weighted Average
193	39	4.98% Pervious Area
3,673	98	95.02% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB5C: Subcat CB5C

Runoff = 0.18 cfs @ 12.08 hrs, Volume= 626 cf, Depth= 2.87"
Routed to Pond 5C : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
225	39	>75% Grass cover, Good, HSG A
2,394	98	Paved parking, HSG A
2,619		Weighted Average
225	39	8.60% Pervious Area
2,394	98	91.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB6: Subcat CB6

Runoff = 0.74 cfs @ 12.08 hrs, Volume= 2,557 cf, Depth= 2.43"
Routed to Pond 6 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
2,854	39	>75% Grass cover, Good, HSG A
9,777	98	Paved parking, HSG A
12,631		Weighted Average
2,854	39	22.59% Pervious Area
9,777	98	77.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment CB7A: Subcat CB7A

Runoff = 0.43 cfs @ 12.08 hrs, Volume= 1,484 cf, Depth= 2.51"
Routed to Pond 7A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
1,426	39	>75% Grass cover, Good, HSG A
5,674	98	Paved parking, HSG A
7,101		Weighted Average
1,426	39	20.09% Pervious Area
5,674	98	79.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB7B: Subcat CB7B

Runoff = 0.64 cfs @ 12.08 hrs, Volume= 2,229 cf, Depth= 2.36"
Routed to Pond 7B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
2,817	39	>75% Grass cover, Good, HSG A
7,718	98	Paved parking, HSG A
805	98	Roofs, HSG A
11,340		Weighted Average
2,817	39	24.84% Pervious Area
8,523	98	75.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB8: Subcat CB8

Runoff = 0.67 cfs @ 12.08 hrs, Volume= 2,335 cf, Depth= 2.55"
Routed to Pond 8 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
2,060	39	>75% Grass cover, Good, HSG A
8,538	98	Paved parking, HSG A
393	98	Roofs, HSG A
10,991		Weighted Average
2,060	39	18.74% Pervious Area
8,931	98	81.26% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9A: Subcat CB9A

Runoff = 0.72 cfs @ 12.08 hrs, Volume= 2,495 cf, Depth= 2.55"
Routed to Pond 9A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
2,202	39	>75% Grass cover, Good, HSG A
7,525	98	Paved parking, HSG A
2,016	98	Roofs, HSG A
11,743		Weighted Average
2,202	39	18.75% Pervious Area
9,541	98	81.25% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9B: Subcat CB9B

Runoff = 0.75 cfs @ 12.08 hrs, Volume= 2,598 cf, Depth= 2.12"
Routed to Pond 9B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
4,805	39	>75% Grass cover, Good, HSG A
9,931	98	Paved parking, HSG A
14,736		Weighted Average
4,805	39	32.60% Pervious Area
9,931	98	67.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9C: Subcat CB9C

Runoff = 0.28 cfs @ 12.08 hrs, Volume= 969 cf, Depth= 3.14"
Routed to Pond 9C : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
0	39	>75% Grass cover, Good, HSG A
2,337	98	Paved parking, HSG A
1,371	98	Roofs, HSG A
3,708		Weighted Average
0	39	0.00% Pervious Area
3,708	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9D: Subcat CB9D

Runoff = 0.07 cfs @ 12.08 hrs, Volume= 236 cf, Depth= 1.83"
Routed to Pond 9D : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
641	39	>75% Grass cover, Good, HSG A
901	98	Paved parking, HSG A
1,543		Weighted Average
641	39	41.57% Pervious Area
901	98	58.43% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-1: Subcat P-1

Runoff = 0.19 cfs @ 12.08 hrs, Volume= 651 cf, Depth= 0.53"
Routed to Pond LP-2 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
12,404	39	>75% Grass cover, Good, HSG A
0	98	Paved parking, HSG A
2,477	98	Water Surface, HSG A
14,881		Weighted Average
12,404	39	83.35% Pervious Area
2,477	98	16.65% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-2: Subcat P-2

Runoff = 0.18 cfs @ 12.08 hrs, Volume= 612 cf, Depth= 0.65"
Routed to Pond TD2 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
8,801	39	>75% Grass cover, Good, HSG A
2,507	96	Gravel surface, HSG A
0	98	Paved parking, HSG A
11,308		Weighted Average
11,308	52	100.00% Pervious Area
0	98	0.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-3: Subcat P-3

Runoff = 0.03 cfs @ 12.08 hrs, Volume= 116 cf, Depth= 0.19"
Routed to Pond LP-1 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
7,078	39	>75% Grass cover, Good, HSG A
5	98	Paved parking, HSG A
431	98	Water Surface, HSG A
7,515		Weighted Average
7,078	39	94.19% Pervious Area
436	98	5.81% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-4: Subcat P-4

Runoff = 2.39 cfs @ 12.16 hrs, Volume= 10,054 cf, Depth= 0.41"
Routed to Link SP3 : On Site Flow To Perennial Stream

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
85,817	39	>75% Grass cover, Good, HSG A
2,376	96	Gravel surface, HSG A
5,773	98	Paved parking, HSG A
30,379	98	Water Surface, HSG A
167,329	30	Woods, Good, HSG A
291,674		Weighted Average
255,522	34	87.61% Pervious Area
36,152	98	12.39% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.0					Direct Entry, Copied from EWS

Summary for Subcatchment P-5: Subcat P-5

Runoff = 0.63 cfs @ 12.11 hrs, Volume= 2,344 cf, Depth= 0.37"
Routed to Pond TD1 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
67,151	39	>75% Grass cover, Good, HSG A
8,890	98	Paved parking, HSG A
76,040		Weighted Average
67,151	39	88.31% Pervious Area
8,890	98	11.69% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, Copied From EWS

Summary for Subcatchment P-6: Subcat P-6

Runoff = 0.20 cfs @ 12.08 hrs, Volume= 688 cf, Depth= 0.33"
Routed to Pond TD1 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
22,749	39	>75% Grass cover, Good, HSG A
2,605	98	Paved parking, HSG A
25,354		Weighted Average
22,749	39	89.73% Pervious Area
2,605	98	10.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Copied from EWS

Summary for Subcatchment P-7: Subcat P-7

Runoff = 0.27 cfs @ 12.15 hrs, Volume= 1,144 cf, Depth= 0.17"
Routed to Pond TD1 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
74,943	39	>75% Grass cover, Good, HSG A
4,287	98	Paved parking, HSG A
79,230		Weighted Average
74,943	39	94.59% Pervious Area
4,287	98	5.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.4					Direct Entry, Copied From EWS

Summary for Subcatchment PATIO: Subcat PATIO

Runoff = 0.21 cfs @ 12.08 hrs, Volume= 741 cf, Depth= 3.14"
Routed to Pond RD1 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
2,833	98	Paved parking, HSG A
2,833	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-1: Subcat R-1

Runoff = 0.38 cfs @ 12.08 hrs, Volume= 1,319 cf, Depth= 3.14"
Routed to Pond RD1 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
5,046	98	Roofs, HSG A
5,046	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-2: Subcat R-2

Runoff = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf, Depth= 3.14"
Routed to Pond RD2 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
18,290	98	Roofs, HSG A
18,290	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-3: Subcat R-3

Runoff = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf, Depth= 3.14"
Routed to Pond RD3 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
18,290	98	Roofs, HSG A
18,290	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-4: Subcat R-4

Runoff = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf, Depth= 3.14"
Routed to Pond RD4 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
18,290	98	Roofs, HSG A
18,290	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-5: Subcat R-5

Runoff = 1.55 cfs @ 12.08 hrs, Volume= 5,376 cf, Depth= 3.14"
Routed to Pond RD5 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 2 Year Rainfall=3.37"

Area (sf)	CN	Description
20,565	98	Roofs, HSG A
20,565	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Pond 1: Catch Basin

Inflow Area = 10,622 sf, 86.84% Impervious, Inflow Depth = 2.72" for 2 Year event
Inflow = 0.69 cfs @ 12.08 hrs, Volume= 2,412 cf
Outflow = 0.69 cfs @ 12.08 hrs, Volume= 2,412 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.69 cfs @ 12.08 hrs, Volume= 2,412 cf
Routed to Pond MH1 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 272.68' @ 12.08 hrs
Flood Elev= 274.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.20'	12.0" Round Culvert L= 84.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.20' / 271.70' S= 0.0060 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.69 cfs @ 12.08 hrs HW=272.68' TW=272.15' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.69 cfs @ 2.73 fps)

Summary for Pond 2: Catch Basin

Inflow Area = 12,976 sf, 8.11% Impervious, Inflow Depth = 0.26" for 2 Year event
Inflow = 0.08 cfs @ 12.08 hrs, Volume= 279 cf
Outflow = 0.08 cfs @ 12.08 hrs, Volume= 278 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.08 cfs @ 12.08 hrs, Volume= 278 cf
Routed to Pond MH27 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.81' @ 12.31 hrs
Flood Elev= 274.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.61'	12.0" Round Culvert L= 47.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.61' / 271.34' S= 0.0057 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.08 cfs @ 12.08 hrs HW=272.93' TW=272.93' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.08 cfs @ 0.10 fps)

Summary for Pond 3: Catch Basin

Inflow Area = 9,660 sf, 44.27% Impervious, Inflow Depth = 2.33" for 2 Year event
Inflow = 0.55 cfs @ 12.08 hrs, Volume= 1,874 cf
Outflow = 0.55 cfs @ 12.08 hrs, Volume= 1,874 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.55 cfs @ 12.08 hrs, Volume= 1,874 cf
Routed to Pond MH27 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.81' @ 12.30 hrs
Flood Elev= 274.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.74'	12.0" Round Culvert L= 77.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.74' / 271.34' S= 0.0052 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.55 cfs @ 12.08 hrs HW=272.96' TW=272.93' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.55 cfs @ 0.73 fps)

Summary for Pond 4A: Catch Basin

Inflow Area = 3,976 sf, 91.93% Impervious, Inflow Depth = 2.88" for 2 Year event
Inflow = 0.27 cfs @ 12.08 hrs, Volume= 956 cf
Outflow = 0.27 cfs @ 12.08 hrs, Volume= 956 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.27 cfs @ 12.08 hrs, Volume= 956 cf
Routed to Pond MH26 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.83' @ 12.08 hrs
Flood Elev= 275.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.19'	12.0" Round Culvert L= 7.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.19' / 272.12' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.27 cfs @ 12.08 hrs HW=272.83' TW=272.82' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.27 cfs @ 0.73 fps)

Summary for Pond 4B: Catch Basin

Inflow Area = 10,218 sf, 73.42% Impervious, Inflow Depth = 2.30" for 2 Year event
Inflow = 0.56 cfs @ 12.08 hrs, Volume= 1,962 cf
Outflow = 0.56 cfs @ 12.08 hrs, Volume= 1,962 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.56 cfs @ 12.08 hrs, Volume= 1,962 cf
Routed to Pond MH26 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.95' @ 12.08 hrs
Flood Elev= 275.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.40'	12.0" Round Culvert L= 27.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.40' / 272.13' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.56 cfs @ 12.08 hrs HW=272.95' TW=272.82' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.56 cfs @ 1.86 fps)

Summary for Pond 4C: Catch Basin

Inflow Area = 10,741 sf, 84.88% Impervious, Inflow Depth = 2.66" for 2 Year event
Inflow = 0.69 cfs @ 12.08 hrs, Volume= 2,384 cf
Outflow = 0.69 cfs @ 12.08 hrs, Volume= 2,384 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.69 cfs @ 12.08 hrs, Volume= 2,384 cf
Routed to Pond MH30 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.73' @ 12.08 hrs
Flood Elev= 275.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.25'	12.0" Round Culvert L= 61.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.25' / 272.78' S= 0.0077 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.68 cfs @ 12.08 hrs HW=273.73' TW=273.32' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.68 cfs @ 2.68 fps)

Summary for Pond 4D: Catch Basin

Inflow Area = 10,203 sf, 79.98% Impervious, Inflow Depth = 2.51" for 2 Year event
Inflow = 0.61 cfs @ 12.08 hrs, Volume= 2,134 cf
Outflow = 0.61 cfs @ 12.08 hrs, Volume= 2,134 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.61 cfs @ 12.08 hrs, Volume= 2,134 cf
Routed to Pond MH30 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.62' @ 12.08 hrs
Flood Elev= 275.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.16'	12.0" Round Culvert L= 38.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.16' / 272.78' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.61 cfs @ 12.08 hrs HW=273.62' TW=273.32' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.61 cfs @ 2.56 fps)

Summary for Pond 5A: Catch Basin

Inflow Area = 10,459 sf, 93.11% Impervious, Inflow Depth = 2.92" for 2 Year event
Inflow = 0.73 cfs @ 12.08 hrs, Volume= 2,546 cf
Outflow = 0.73 cfs @ 12.08 hrs, Volume= 2,546 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.73 cfs @ 12.08 hrs, Volume= 2,546 cf
Routed to Pond MH28 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.96' @ 12.08 hrs
Flood Elev= 276.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.47'	12.0" Round Culvert L= 28.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.47' / 273.19' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.73 cfs @ 12.08 hrs HW=273.95' TW=273.64' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.73 cfs @ 2.83 fps)

Summary for Pond 5B: Catch Basin

Inflow Area = 3,865 sf, 95.02% Impervious, Inflow Depth = 2.98" for 2 Year event
Inflow = 0.28 cfs @ 12.08 hrs, Volume= 960 cf
Outflow = 0.28 cfs @ 12.08 hrs, Volume= 960 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.28 cfs @ 12.08 hrs, Volume= 960 cf
Routed to Pond MH28 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.94' @ 12.08 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.64'	12.0" Round Culvert L= 45.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.64' / 273.19' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.28 cfs @ 12.08 hrs HW=273.94' TW=273.64' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.28 cfs @ 2.11 fps)

Summary for Pond 5C: Catch Basin

Inflow Area = 2,619 sf, 91.40% Impervious, Inflow Depth = 2.87" for 2 Year event
Inflow = 0.18 cfs @ 12.08 hrs, Volume= 626 cf
Outflow = 0.18 cfs @ 12.08 hrs, Volume= 626 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.18 cfs @ 12.08 hrs, Volume= 626 cf
Routed to Pond MH31 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.11' @ 12.08 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.90'	12.0" Round Culvert L= 41.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.90' / 273.49' S= 0.0100 ' /' Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.18 cfs @ 12.08 hrs HW=274.11' TW=273.71' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.18 cfs @ 2.22 fps)

Summary for Pond 6: Catch Basin

Inflow Area = 12,631 sf, 77.41% Impervious, Inflow Depth = 2.43" for 2 Year event
Inflow = 0.74 cfs @ 12.08 hrs, Volume= 2,557 cf
Outflow = 0.74 cfs @ 12.08 hrs, Volume= 2,557 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.74 cfs @ 12.08 hrs, Volume= 2,557 cf
Routed to Pond MH7 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.25' @ 12.08 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.76'	12.0" Round Culvert L= 26.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.76' / 272.50' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.73 cfs @ 12.08 hrs HW=273.25' TW=272.95' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.73 cfs @ 2.80 fps)

Summary for Pond 7A: Catch Basin

Inflow Area = 7,101 sf, 79.91% Impervious, Inflow Depth = 2.51" for 2 Year event
Inflow = 0.43 cfs @ 12.08 hrs, Volume= 1,484 cf
Outflow = 0.43 cfs @ 12.08 hrs, Volume= 1,484 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.43 cfs @ 12.08 hrs, Volume= 1,484 cf
Routed to Pond MH8 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.39' @ 12.08 hrs
Flood Elev= 276.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.92'	12.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.92' / 272.82' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.43 cfs @ 12.08 hrs HW=273.39' TW=273.30' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.43 cfs @ 1.72 fps)

Summary for Pond 7B: Catch Basin

Inflow Area = 11,340 sf, 75.16% Impervious, Inflow Depth = 2.36" for 2 Year event
Inflow = 0.64 cfs @ 12.08 hrs, Volume= 2,229 cf
Outflow = 0.64 cfs @ 12.08 hrs, Volume= 2,229 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.64 cfs @ 12.08 hrs, Volume= 2,229 cf
Routed to Pond MH8 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.67' @ 12.08 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.22'	12.0" Round Culvert L= 40.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.22' / 272.82' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.64 cfs @ 12.08 hrs HW=273.67' TW=273.30' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.64 cfs @ 2.76 fps)

Summary for Pond 8: Catch Basin

Inflow Area = 10,991 sf, 81.26% Impervious, Inflow Depth = 2.55" for 2 Year event
Inflow = 0.67 cfs @ 12.08 hrs, Volume= 2,335 cf
Outflow = 0.67 cfs @ 12.08 hrs, Volume= 2,335 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.67 cfs @ 12.08 hrs, Volume= 2,335 cf
Routed to Pond MH33 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.05' @ 12.08 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.47'	12.0" Round Culvert L= 27.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.47' / 273.20' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.67 cfs @ 12.08 hrs HW=274.05' TW=273.89' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.67 cfs @ 2.07 fps)

Summary for Pond 9A: Catch Basin

Inflow Area = 11,743 sf, 81.25% Impervious, Inflow Depth = 2.55" for 2 Year event
Inflow = 0.72 cfs @ 12.08 hrs, Volume= 2,495 cf
Outflow = 0.72 cfs @ 12.08 hrs, Volume= 2,495 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.72 cfs @ 12.08 hrs, Volume= 2,495 cf
Routed to Pond MH32 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.16' @ 12.08 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.58'	12.0" Round Culvert L= 14.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.58' / 273.51' S= 0.0050 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.72 cfs @ 12.08 hrs HW=274.16' TW=274.01' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.72 cfs @ 2.19 fps)

Summary for Pond 9B: Catch Basin

Inflow Area = 14,736 sf, 67.40% Impervious, Inflow Depth = 2.12" for 2 Year event
Inflow = 0.75 cfs @ 12.08 hrs, Volume= 2,598 cf
Outflow = 0.75 cfs @ 12.08 hrs, Volume= 2,598 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.75 cfs @ 12.08 hrs, Volume= 2,598 cf
Routed to Pond MH33 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.16' @ 12.08 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.58'	12.0" Round Culvert L= 64.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.58' / 273.20' S= 0.0059 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.75 cfs @ 12.08 hrs HW=274.16' TW=273.89' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.75 cfs @ 2.26 fps)

Summary for Pond 9C: Catch Basin

Inflow Area = 3,708 sf, 100.00% Impervious, Inflow Depth = 3.14" for 2 Year event
Inflow = 0.28 cfs @ 12.08 hrs, Volume= 969 cf
Outflow = 0.28 cfs @ 12.08 hrs, Volume= 969 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.28 cfs @ 12.08 hrs, Volume= 969 cf
Routed to Pond MH12 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.83' @ 12.51 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.24'	12.0" Round Culvert L= 7.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.24' / 273.16' S= 0.0114 '/ S Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.28 cfs @ 12.08 hrs HW=273.80' TW=273.78' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.28 cfs @ 0.89 fps)

Summary for Pond 9D: Catch Basin

Inflow Area = 1,543 sf, 58.43% Impervious, Inflow Depth = 1.83" for 2 Year event
Inflow = 0.07 cfs @ 12.08 hrs, Volume= 236 cf
Outflow = 0.07 cfs @ 12.08 hrs, Volume= 236 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.07 cfs @ 12.08 hrs, Volume= 236 cf
Routed to Pond MH32 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.13' @ 12.08 hrs
Flood Elev= 276.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.95'	12.0" Round Culvert L= 49.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.95' / 273.51' S= 0.0090 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.07 cfs @ 12.08 hrs HW=274.13' TW=274.01' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.07 cfs @ 1.05 fps)

Summary for Pond 10A: Catch Basin

Inflow Area = 2,889 sf, 81.34% Impervious, Inflow Depth = 2.55" for 2 Year event
Inflow = 0.18 cfs @ 12.08 hrs, Volume= 614 cf
Outflow = 0.18 cfs @ 12.08 hrs, Volume= 614 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.18 cfs @ 12.08 hrs, Volume= 614 cf
Routed to Pond MH15 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.63' @ 12.08 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.41'	12.0" Round Culvert L= 87.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.41' / 270.54' S= 0.0100 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.18 cfs @ 12.08 hrs HW=271.63' TW=271.01' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.18 cfs @ 2.08 fps)

Summary for Pond 10B: Catch Basin

Inflow Area = 10,414 sf, 86.69% Impervious, Inflow Depth = 2.72" for 2 Year event
Inflow = 0.68 cfs @ 12.08 hrs, Volume= 2,360 cf
Outflow = 0.68 cfs @ 12.08 hrs, Volume= 2,360 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.68 cfs @ 12.08 hrs, Volume= 2,360 cf
Routed to Pond MH15 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.20' @ 12.08 hrs
Flood Elev= 276.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.69'	12.0" Round Culvert L= 15.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.69' / 270.54' S= 0.0100 ' / ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.68 cfs @ 12.08 hrs HW=271.20' TW=271.01' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.68 cfs @ 2.47 fps)

Summary for Pond 11: Catch Basin

Inflow Area = 11,505 sf, 85.23% Impervious, Inflow Depth = 2.67" for 2 Year event
Inflow = 0.74 cfs @ 12.08 hrs, Volume= 2,564 cf
Outflow = 0.74 cfs @ 12.08 hrs, Volume= 2,564 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.74 cfs @ 12.08 hrs, Volume= 2,564 cf
Routed to Pond MH17 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.10' @ 12.46 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.55'	12.0" Round Culvert L= 15.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.55' / 270.40' S= 0.0100 ' / ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.74 cfs @ 12.08 hrs HW=271.03' TW=270.76' (Dynamic Tailwater)
1=Culvert (Barrel Controls 0.74 cfs @ 2.91 fps)

Summary for Pond 12: Catch Basin

Inflow Area = 7,149 sf, 71.54% Impervious, Inflow Depth = 2.25" for 2 Year event
Inflow = 0.38 cfs @ 12.08 hrs, Volume= 1,337 cf
Outflow = 0.38 cfs @ 12.09 hrs, Volume= 1,337 cf, Atten= 0%, Lag= 0.1 min
Primary = 0.38 cfs @ 12.09 hrs, Volume= 1,337 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.68' @ 12.08 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.74'	12.0" Round Culvert L= 14.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.74' / 270.60' S= 0.0100 ' / ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.38 cfs @ 12.09 hrs HW=271.68' TW=271.67' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.38 cfs @ 0.65 fps)

Summary for Pond 13: Catch Basin

Inflow Area = 4,141 sf, 88.54% Impervious, Inflow Depth = 2.78" for 2 Year event
Inflow = 0.28 cfs @ 12.08 hrs, Volume= 958 cf
Outflow = 0.28 cfs @ 12.08 hrs, Volume= 958 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.28 cfs @ 12.08 hrs, Volume= 958 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.10' @ 12.08 hrs
Flood Elev= 275.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.79'	12.0" Round Culvert L= 87.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.79' / 270.92' S= 0.0100 ' / ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.28 cfs @ 12.08 hrs HW=272.10' TW=271.67' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.28 cfs @ 2.02 fps)

Summary for Pond 14: Catch Basin

Inflow Area = 6,167 sf, 87.00% Impervious, Inflow Depth = 2.73" for 2 Year event
Inflow = 0.40 cfs @ 12.08 hrs, Volume= 1,403 cf
Outflow = 0.40 cfs @ 12.08 hrs, Volume= 1,403 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.40 cfs @ 12.08 hrs, Volume= 1,403 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.76' @ 12.08 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.25'	12.0" Round Culvert L= 33.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.25' / 270.92' S= 0.0100 ' / ' Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.40 cfs @ 12.08 hrs HW=271.76' TW=271.67' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.40 cfs @ 1.46 fps)

Summary for Pond 15: Catch Basin

Inflow Area = 20,154 sf, 44.65% Impervious, Inflow Depth = 1.40" for 2 Year event
Inflow = 0.68 cfs @ 12.08 hrs, Volume= 2,356 cf
Outflow = 0.68 cfs @ 12.08 hrs, Volume= 2,356 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.68 cfs @ 12.08 hrs, Volume= 2,356 cf
Routed to Pond MH23 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.95' @ 12.08 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.42'	12.0" Round Culvert L= 19.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.42' / 272.23' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.68 cfs @ 12.08 hrs HW=272.95' TW=272.77' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.68 cfs @ 2.31 fps)

Summary for Pond 16: Catch Basin

Inflow Area = 20,584 sf, 35.58% Impervious, Inflow Depth = 1.12" for 2 Year event
Inflow = 0.55 cfs @ 12.08 hrs, Volume= 1,918 cf
Outflow = 0.55 cfs @ 12.08 hrs, Volume= 1,918 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.55 cfs @ 12.08 hrs, Volume= 1,918 cf
Routed to Pond MH24 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.72' @ 12.08 hrs
Flood Elev= 277.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	274.29'	12.0" Round Culvert L= 19.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 274.29' / 274.10' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.55 cfs @ 12.08 hrs HW=274.72' TW=274.50' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.55 cfs @ 2.49 fps)

Summary for Pond 17: Catch Basin

Inflow Area = 36,035 sf, 27.88% Impervious, Inflow Depth = 0.88" for 2 Year event
Inflow = 0.76 cfs @ 12.08 hrs, Volume= 2,634 cf
Outflow = 0.76 cfs @ 12.08 hrs, Volume= 2,634 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.76 cfs @ 12.08 hrs, Volume= 2,634 cf
Routed to Pond MH24 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.41' @ 12.08 hrs
Flood Elev= 277.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	274.97'	15.0" Round Culvert L= 143.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 274.97' / 274.10' S= 0.0061 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=0.75 cfs @ 12.08 hrs HW=275.41' TW=274.50' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.75 cfs @ 2.90 fps)

Summary for Pond LP-1: Low Point - MAHW

Inflow Area = 210,774 sf, 10.22% Impervious, Inflow Depth = 0.07" for 2 Year event
Inflow = 0.84 cfs @ 12.14 hrs, Volume= 1,240 cf
Outflow = 0.83 cfs @ 12.16 hrs, Volume= 1,145 cf, Atten= 2%, Lag= 1.1 min
Primary = 0.83 cfs @ 12.16 hrs, Volume= 1,145 cf
Routed to Link SP1 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 268.04' @ 12.16 hrs Surf.Area= 261 sf Storage= 225 cf

Plug-Flow detention time= 23.7 min calculated for 1,145 cf (92% of inflow)
Center-of-Mass det. time= 14.6 min (749.8 - 735.1)

Volume	Invert	Avail.Storage	Storage Description		
#1	267.00'	902 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
267.00	175	160.0	0	0	175
268.00	258	168.0	215	215	441
269.00	344	172.0	300	515	642
270.00	431	176.0	387	902	848

Device	Routing	Invert	Outlet Devices
#1	Primary	267.93'	24.0" Round RCP_Round 24" L= 59.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 267.72' / 267.93' S= -0.0036 '/' Cc= 0.900 n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf

#2 Primary 267.48' **24.0" Round RCP_Round 24"**
L= 59.0' RCP, square edge headwall, Ke= 0.500
Inlet / Outlet Invert= 267.48' / 267.48' S= 0.0000 '/ S= 0.0000 ' Cc= 0.900
n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf

Primary OutFlow Max=0.83 cfs @ 12.16 hrs HW=268.04' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Inlet Controls 0.07 cfs @ 1.12 fps)
2=RCP_Round 24" (Barrel Controls 0.75 cfs @ 1.57 fps)

Summary for Pond LP-2: Low Point - MAHW

Inflow Area = 236,963 sf, 10.14% Impervious, Inflow Depth = 0.09" for 2 Year event
Inflow = 0.96 cfs @ 12.15 hrs, Volume= 1,796 cf
Outflow = 0.78 cfs @ 12.23 hrs, Volume= 1,796 cf, Atten= 19%, Lag= 4.7 min
Primary = 0.78 cfs @ 12.23 hrs, Volume= 1,796 cf
Routed to Link SP2 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 267.16' @ 12.23 hrs Surf.Area= 1,334 sf Storage= 267 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time= 4.7 min (757.6 - 752.9)

Volume	Invert	Avail.Storage	Storage Description		
#1	266.85'	5,666 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
266.85	0	0.0	0	0	0
267.00	1,276	387.0	64	64	11,918
268.00	1,668	396.0	1,468	1,531	12,602
269.00	2,069	404.0	1,865	3,396	13,250
270.00	2,477	412.0	2,270	5,666	13,910

Device	Routing	Invert	Outlet Devices
#1	Primary	266.85'	24.0" Round RCP_Round 24" L= 30.0' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 266.37' / 266.85' S= -0.0160 '/ S= -0.0160 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=0.78 cfs @ 12.23 hrs HW=267.16' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Inlet Controls 0.78 cfs @ 2.57 fps)

Summary for Pond MH1: Manhole

Inflow Area = 10,622 sf, 86.84% Impervious, Inflow Depth = 2.72" for 2 Year event
Inflow = 0.69 cfs @ 12.08 hrs, Volume= 2,412 cf
Outflow = 0.69 cfs @ 12.08 hrs, Volume= 2,412 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.69 cfs @ 12.08 hrs, Volume= 2,412 cf
Routed to Pond UIS1 : Underground Chamber #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.15' @ 12.08 hrs
Flood Elev= 275.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.70'	24.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.70' / 271.70' S= 0.0000 '/ S= 0.0000 ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=0.69 cfs @ 12.08 hrs HW=272.15' TW=271.75' (Dynamic Tailwater)
1=Culvert (Barrel Controls 0.69 cfs @ 1.98 fps)

Summary for Pond MH11: Manhole

Inflow Area = 25,727 sf, 73.32% Impervious, Inflow Depth = 2.30" for 2 Year event
Inflow = 1.42 cfs @ 12.08 hrs, Volume= 4,933 cf
Outflow = 1.42 cfs @ 12.08 hrs, Volume= 4,933 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.42 cfs @ 12.08 hrs, Volume= 4,933 cf
Routed to Pond UIS4 : Underground Chamber #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.82' @ 12.52 hrs
Flood Elev= 276.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.00'	24.0" Round Culvert L= 4.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.00' / 273.00' S= 0.0000 '/ S= 0.0000 ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.42 cfs @ 12.08 hrs HW=273.63' TW=273.25' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.42 cfs @ 2.52 fps)

Summary for Pond MH12: Manhole

Inflow Area = 16,994 sf, 83.27% Impervious, Inflow Depth = 2.61" for 2 Year event
Inflow = 1.06 cfs @ 12.08 hrs, Volume= 3,700 cf
Outflow = 1.06 cfs @ 12.08 hrs, Volume= 3,700 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.06 cfs @ 12.08 hrs, Volume= 3,700 cf
Routed to Pond MH13 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.83' @ 12.50 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.16'	15.0" Round Culvert L= 28.0' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 273.16' / 273.00' S= 0.0057 '/' Cc= 0.900
n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.06 cfs @ 12.08 hrs HW=273.78' TW=273.55' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.06 cfs @ 2.55 fps)

Summary for Pond MH13: Manhole

Inflow Area = 16,994 sf, 83.27% Impervious, Inflow Depth = 2.61" for 2 Year event
Inflow = 1.06 cfs @ 12.08 hrs, Volume= 3,700 cf
Outflow = 1.06 cfs @ 12.08 hrs, Volume= 3,700 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.06 cfs @ 12.08 hrs, Volume= 3,700 cf
Routed to Pond UIS4 : Underground Chamber #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.82' @ 12.52 hrs
Flood Elev= 277.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.00'	24.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.00' / 273.00' S= 0.0000 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.06 cfs @ 12.08 hrs HW=273.55' TW=273.25' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.06 cfs @ 2.25 fps)

Summary for Pond MH15: Manhole

Inflow Area = 13,302 sf, 85.53% Impervious, Inflow Depth = 2.68" for 2 Year event
Inflow = 0.86 cfs @ 12.08 hrs, Volume= 2,975 cf
Outflow = 0.86 cfs @ 12.08 hrs, Volume= 2,975 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.86 cfs @ 12.08 hrs, Volume= 2,975 cf
Routed to Pond MH16 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.10' @ 12.46 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.44'	12.0" Round Culvert L= 14.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.44' / 270.30' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.85 cfs @ 12.08 hrs HW=271.01' TW=270.79' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.85 cfs @ 2.70 fps)

Summary for Pond MH16: Manhole

Inflow Area = 13,302 sf, 85.53% Impervious, Inflow Depth = 2.68" for 2 Year event
Inflow = 0.86 cfs @ 12.08 hrs, Volume= 2,975 cf
Outflow = 0.86 cfs @ 12.08 hrs, Volume= 2,975 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.86 cfs @ 12.08 hrs, Volume= 2,975 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.09' @ 12.48 hrs
Flood Elev= 276.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.30'	24.0" Round Culvert L= 5.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.30' / 270.30' S= 0.0000 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=0.85 cfs @ 12.08 hrs HW=270.79' TW=270.56' (Dynamic Tailwater)
1=Culvert (Barrel Controls 0.85 cfs @ 2.18 fps)

Summary for Pond MH17: Manhole

Inflow Area = 11,505 sf, 85.23% Impervious, Inflow Depth = 2.67" for 2 Year event
Inflow = 0.74 cfs @ 12.08 hrs, Volume= 2,564 cf
Outflow = 0.74 cfs @ 12.08 hrs, Volume= 2,564 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.74 cfs @ 12.08 hrs, Volume= 2,564 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.09' @ 12.48 hrs
Flood Elev= 277.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.30'	24.0" Round Culvert L= 7.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.30' / 270.30' S= 0.0000 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=0.74 cfs @ 12.08 hrs HW=270.76' TW=270.56' (Dynamic Tailwater)
1=Culvert (Barrel Controls 0.74 cfs @ 2.06 fps)

Summary for Pond MH19: Manhole

Inflow Area = 157,891 sf, 63.68% Impervious, Inflow Depth = 0.00" for 2 Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 269.87' @ 0.00 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	269.87'	18.0" Round Culvert L= 51.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 269.87' / 269.52' S= 0.0069 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=269.87' TW=0.00' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)

Summary for Pond MH21: Manhole

Inflow Area = 94,229 sf, 43.00% Impervious, Inflow Depth = 1.35" for 2 Year event
Inflow = 3.05 cfs @ 12.08 hrs, Volume= 10,607 cf
Outflow = 3.05 cfs @ 12.08 hrs, Volume= 10,607 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.05 cfs @ 12.08 hrs, Volume= 10,607 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.31' @ 12.08 hrs
Flood Elev= 277.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.50'	24.0" Round Culvert L= 20.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.50' / 270.30' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=3.04 cfs @ 12.08 hrs HW=271.31' TW=270.56' (Dynamic Tailwater)
1=Culvert (Barrel Controls 3.04 cfs @ 3.75 fps)

Summary for Pond MH22: Manhole

Inflow Area = 94,229 sf, 43.00% Impervious, Inflow Depth = 1.35" for 2 Year event
Inflow = 3.05 cfs @ 12.08 hrs, Volume= 10,607 cf
Outflow = 3.05 cfs @ 12.08 hrs, Volume= 10,607 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.05 cfs @ 12.08 hrs, Volume= 10,607 cf
Routed to Pond MH21 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.67' @ 12.08 hrs
Flood Elev= 277.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.82'	24.0" Round Culvert L= 22.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.82' / 270.60' S= 0.0100 '/ Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=3.04 cfs @ 12.08 hrs HW=271.67' TW=271.31' (Dynamic Tailwater)
1=Culvert (Outlet Controls 3.04 cfs @ 3.52 fps)

Summary for Pond MH23: Manhole

Inflow Area = 76,773 sf, 34.35% Impervious, Inflow Depth = 1.08" for 2 Year event
Inflow = 1.98 cfs @ 12.08 hrs, Volume= 6,908 cf
Outflow = 1.98 cfs @ 12.08 hrs, Volume= 6,908 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.98 cfs @ 12.08 hrs, Volume= 6,908 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.78' @ 12.08 hrs
Flood Elev= 277.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.13'	18.0" Round Culvert L= 121.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.13' / 270.92' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=1.98 cfs @ 12.08 hrs HW=272.77' TW=271.67' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.98 cfs @ 4.02 fps)

Summary for Pond MH24: Manhole

Inflow Area = 56,619 sf, 30.68% Impervious, Inflow Depth = 0.96" for 2 Year event
Inflow = 1.31 cfs @ 12.08 hrs, Volume= 4,552 cf
Outflow = 1.31 cfs @ 12.08 hrs, Volume= 4,552 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.31 cfs @ 12.08 hrs, Volume= 4,552 cf
Routed to Pond MH23 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.50' @ 12.08 hrs
Flood Elev= 278.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.99'	18.0" Round Culvert L= 176.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.99' / 272.23' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=1.30 cfs @ 12.08 hrs HW=274.50' TW=272.77' (Dynamic Tailwater)
1=Culvert (Inlet Controls 1.30 cfs @ 2.44 fps)

Summary for Pond MH26: Manhole

Inflow Area = 35,138 sf, 80.92% Impervious, Inflow Depth = 2.54" for 2 Year event
Inflow = 2.14 cfs @ 12.08 hrs, Volume= 7,435 cf
Outflow = 2.14 cfs @ 12.08 hrs, Volume= 7,435 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.14 cfs @ 12.08 hrs, Volume= 7,435 cf
Routed to Pond UIS1 : Underground Chamber #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.82' @ 12.08 hrs
Flood Elev= 275.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.13'	18.0" Round Culvert L= 43.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.13' / 271.70' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=2.13 cfs @ 12.08 hrs HW=272.82' TW=271.75' (Dynamic Tailwater)
1=Culvert (Barrel Controls 2.13 cfs @ 3.97 fps)

Summary for Pond MH27: Manhole

Inflow Area = 22,636 sf, 23.54% Impervious, Inflow Depth = 1.14" for 2 Year event
Inflow = 0.63 cfs @ 12.08 hrs, Volume= 2,152 cf
Outflow = 0.63 cfs @ 12.08 hrs, Volume= 2,152 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.63 cfs @ 12.08 hrs, Volume= 2,152 cf
Routed to Pond UIS2 : Underground Chamber #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.81' @ 12.31 hrs
Flood Elev= 275.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.34'	15.0" Round Culvert L= 8.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.34' / 272.30' S= 0.0050 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=0.63 cfs @ 12.08 hrs HW=272.93' TW=272.86' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.63 cfs @ 1.63 fps)

Summary for Pond MH28: Manhole

Inflow Area = 16,944 sf, 93.28% Impervious, Inflow Depth = 2.93" for 2 Year event
Inflow = 1.19 cfs @ 12.08 hrs, Volume= 4,132 cf
Outflow = 1.19 cfs @ 12.08 hrs, Volume= 4,132 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.19 cfs @ 12.08 hrs, Volume= 4,132 cf
Routed to Pond MH6 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.64' @ 12.08 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.09'	15.0" Round Culvert L= 59.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.09' / 272.50' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.19 cfs @ 12.08 hrs HW=273.64' TW=273.08' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.19 cfs @ 3.35 fps)

Summary for Pond MH29: Manhole

Inflow Area = 20,944 sf, 82.49% Impervious, Inflow Depth = 2.59" for 2 Year event
Inflow = 1.30 cfs @ 12.08 hrs, Volume= 4,517 cf
Outflow = 1.30 cfs @ 12.08 hrs, Volume= 4,517 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.30 cfs @ 12.08 hrs, Volume= 4,517 cf
Routed to Pond MH26 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.98' @ 12.08 hrs
Flood Elev= 275.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.24'	15.0" Round Culvert L= 11.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.24' / 272.13' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.30 cfs @ 12.08 hrs HW=272.98' TW=272.82' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.30 cfs @ 2.46 fps)

Summary for Pond MH30: Manhole

Inflow Area = 20,944 sf, 82.49% Impervious, Inflow Depth = 2.59" for 2 Year event
Inflow = 1.30 cfs @ 12.08 hrs, Volume= 4,517 cf
Outflow = 1.30 cfs @ 12.08 hrs, Volume= 4,517 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.30 cfs @ 12.08 hrs, Volume= 4,517 cf
Routed to Pond MH29 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.32' @ 12.08 hrs
Flood Elev= 275.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.68'	15.0" Round Culvert L= 34.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.68' / 272.34' S= 0.0100 ' /' Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.30 cfs @ 12.08 hrs HW=273.32' TW=272.98' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.30 cfs @ 3.02 fps)

Summary for Pond MH31: Manhole

Inflow Area = 2,619 sf, 91.40% Impervious, Inflow Depth = 2.87" for 2 Year event
Inflow = 0.18 cfs @ 12.08 hrs, Volume= 626 cf
Outflow = 0.18 cfs @ 12.08 hrs, Volume= 626 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.18 cfs @ 12.08 hrs, Volume= 626 cf
Routed to Pond MH28 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.71' @ 12.08 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.39'	12.0" Round Culvert L= 20.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.39' / 273.19' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.18 cfs @ 12.08 hrs HW=273.71' TW=273.64' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.18 cfs @ 1.26 fps)

Summary for Pond MH32: Manhole

Inflow Area = 13,286 sf, 78.60% Impervious, Inflow Depth = 2.47" for 2 Year event
Inflow = 0.79 cfs @ 12.08 hrs, Volume= 2,731 cf
Outflow = 0.79 cfs @ 12.08 hrs, Volume= 2,731 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.79 cfs @ 12.08 hrs, Volume= 2,731 cf
Routed to Pond MH12 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.01' @ 12.08 hrs
Flood Elev= 276.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.41'	12.0" Round Culvert L= 48.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.41' / 273.16' S= 0.0052 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.78 cfs @ 12.08 hrs HW=274.01' TW=273.78' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.78 cfs @ 2.27 fps)

Summary for Pond MH33: Manhole

Inflow Area = 25,727 sf, 73.32% Impervious, Inflow Depth = 2.30" for 2 Year event
Inflow = 1.42 cfs @ 12.08 hrs, Volume= 4,933 cf
Outflow = 1.42 cfs @ 12.08 hrs, Volume= 4,933 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.42 cfs @ 12.08 hrs, Volume= 4,933 cf
Routed to Pond MH11 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.89' @ 12.08 hrs
Flood Elev= 277.05'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.20'	15.0" Round Culvert L= 20.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.20' / 273.00' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.42 cfs @ 12.08 hrs HW=273.89' TW=273.63' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.42 cfs @ 2.94 fps)

Summary for Pond MH6: Manhole

Inflow Area = 16,944 sf, 93.28% Impervious, Inflow Depth = 2.93" for 2 Year event
Inflow = 1.19 cfs @ 12.08 hrs, Volume= 4,132 cf
Outflow = 1.19 cfs @ 12.08 hrs, Volume= 4,132 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.19 cfs @ 12.08 hrs, Volume= 4,132 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.08' @ 12.08 hrs
Flood Elev= 277.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.50'	24.0" Round Culvert L= 6.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.50' / 272.50' S= 0.0000 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.19 cfs @ 12.08 hrs HW=273.08' TW=272.59' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.19 cfs @ 2.37 fps)

Summary for Pond MH7: Manhole

Inflow Area = 12,631 sf, 77.41% Impervious, Inflow Depth = 2.43" for 2 Year event
Inflow = 0.74 cfs @ 12.08 hrs, Volume= 2,557 cf
Outflow = 0.74 cfs @ 12.08 hrs, Volume= 2,557 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.74 cfs @ 12.08 hrs, Volume= 2,557 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.96' @ 12.46 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.50'	24.0" Round Culvert L= 6.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.50' / 272.50' S= 0.0000 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=0.73 cfs @ 12.08 hrs HW=272.95' TW=272.59' (Dynamic Tailwater)
1=Culvert (Barrel Controls 0.73 cfs @ 2.07 fps)

Summary for Pond MH8: Manhole

Inflow Area = 18,441 sf, 76.99% Impervious, Inflow Depth = 2.42" for 2 Year event
Inflow = 1.07 cfs @ 12.08 hrs, Volume= 3,713 cf
Outflow = 1.07 cfs @ 12.08 hrs, Volume= 3,713 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.07 cfs @ 12.08 hrs, Volume= 3,713 cf
Routed to Pond MH9 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.30' @ 12.08 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.72'	15.0" Round Culvert L= 22.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.72' / 272.50' S= 0.0100 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.07 cfs @ 12.08 hrs HW=273.30' TW=273.05' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.07 cfs @ 2.78 fps)

Summary for Pond MH9: Manhole

Inflow Area = 18,441 sf, 76.99% Impervious, Inflow Depth = 2.42" for 2 Year event
Inflow = 1.07 cfs @ 12.08 hrs, Volume= 3,713 cf
Outflow = 1.07 cfs @ 12.08 hrs, Volume= 3,713 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.07 cfs @ 12.08 hrs, Volume= 3,713 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.05' @ 12.08 hrs
Flood Elev= 277.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.50'	24.0" Round Culvert L= 8.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.50' / 272.50' S= 0.0000 '/ S Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.07 cfs @ 12.08 hrs HW=273.05' TW=272.59' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.07 cfs @ 2.27 fps)

Summary for Pond RD1: Roof Drain

Inflow Area = 7,878 sf, 100.00% Impervious, Inflow Depth = 3.14" for 2 Year event
Inflow = 0.59 cfs @ 12.08 hrs, Volume= 2,059 cf
Outflow = 0.59 cfs @ 12.08 hrs, Volume= 2,059 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.59 cfs @ 12.08 hrs, Volume= 2,059 cf
Routed to Pond UIS1 : Underground Chamber #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.32' @ 12.08 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.91'	12.0" Round Culvert L= 21.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.91' / 271.70' S= 0.0100 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.59 cfs @ 12.08 hrs HW=272.32' TW=271.75' (Dynamic Tailwater)
1=Culvert (Barrel Controls 0.59 cfs @ 2.89 fps)

Summary for Pond RD2: Roof Drain

Inflow Area = 18,290 sf, 100.00% Impervious, Inflow Depth = 3.14" for 2 Year event
Inflow = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf
Outflow = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.46' @ 12.08 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.81'	12.0" Round Culvert L= 31.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.81' / 272.50' S= 0.0100 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.37 cfs @ 12.08 hrs HW=273.46' TW=272.59' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.37 cfs @ 3.60 fps)

Summary for Pond RD3: Roof Drain

Inflow Area = 18,290 sf,100.00% Impervious, Inflow Depth = 3.14" for 2 Year event
Inflow = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf
Outflow = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.34' @ 12.08 hrs
Flood Elev= 278.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.65'	12.0" Round Culvert L= 14.5' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.65' / 272.50' S= 0.0103 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.37 cfs @ 12.08 hrs HW=273.34' TW=272.59' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.37 cfs @ 3.33 fps)

Summary for Pond RD4: Roof Drain

Inflow Area = 18,290 sf,100.00% Impervious, Inflow Depth = 3.14" for 2 Year event
Inflow = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf
Outflow = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.38 cfs @ 12.08 hrs, Volume= 4,781 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.21' @ 12.08 hrs
Flood Elev= 277.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.55'	12.0" Round Culvert L= 24.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.55' / 270.30' S= 0.0104 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.37 cfs @ 12.08 hrs HW=271.21' TW=270.56' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.37 cfs @ 3.53 fps)

Summary for Pond RD5: Roof Drain

Inflow Area = 20,565 sf,100.00% Impervious, Inflow Depth = 3.14" for 2 Year event
Inflow = 1.55 cfs @ 12.08 hrs, Volume= 5,376 cf
Outflow = 1.55 cfs @ 12.08 hrs, Volume= 5,376 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.55 cfs @ 12.08 hrs, Volume= 5,376 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.14' @ 12.08 hrs
Flood Elev= 278.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.48'	15.0" Round Culvert L= 18.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.48' / 270.30' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.54 cfs @ 12.08 hrs HW=271.14' TW=270.56' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.54 cfs @ 3.39 fps)

Summary for Pond TD1: Stone Trench Drain

Inflow Area = 180,624 sf, 8.74% Impervious, Inflow Depth = 0.28" for 2 Year event
Inflow = 1.06 cfs @ 12.11 hrs, Volume= 4,176 cf
Outflow = 1.02 cfs @ 12.14 hrs, Volume= 4,176 cf, Atten= 5%, Lag= 1.9 min
Discarded = 0.20 cfs @ 11.69 hrs, Volume= 3,151 cf
Primary = 0.81 cfs @ 12.14 hrs, Volume= 1,025 cf
Routed to Pond LP-1 : Low Point - MAHW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.02' @ 12.14 hrs Surf.Area= 1,050 sf Storage= 130 cf
Flood Elev= 272.90' Surf.Area= 1,050 sf Storage= 475 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time= 0.8 min (765.2 - 764.4)

Volume	Invert	Avail.Storage	Storage Description
#1	271.20'	4 cf	3.50'W x 300.00'L x 1.70'H Prismatic 1,785 cf Overall - 471 cf Embedded = 1,314 cf x 0.3% Voids
#2	271.70'	471 cf	12.0" Round Pipe Storage x 2 Inside #1 L= 300.0'
#3	272.90'	3,296 cf	Custom Stage Data (Irregular) Listed below (Recalc)
		3,771 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
272.90	0	0.0	0	0	0
273.00	2,670	657.2	89	89	34,370
274.00	3,775	679.8	3,207	3,296	36,866

Device	Routing	Invert	Outlet Devices
#1	Primary	271.70'	12.0" Round Culvert X 2.00 L= 60.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.70' / 270.90' S= 0.0133 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Discarded	271.20'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.20 cfs @ 11.69 hrs HW=271.31' (Free Discharge)
2=Exfiltration (Exfiltration Controls 0.20 cfs)

Primary OutFlow Max=0.81 cfs @ 12.14 hrs HW=272.02' TW=268.04' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.81 cfs @ 1.91 fps)

Summary for Pond TD2: Stone Trench Drain

Inflow Area = 11,308 sf, 0.00% Impervious, Inflow Depth = 0.65" for 2 Year event
Inflow = 0.18 cfs @ 12.08 hrs, Volume= 612 cf
Outflow = 0.08 cfs @ 12.01 hrs, Volume= 612 cf, Atten= 55%, Lag= 0.0 min
Discarded = 0.08 cfs @ 12.01 hrs, Volume= 612 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Pond LP-2 : Low Point - MAHW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.36' @ 12.26 hrs Surf.Area= 427 sf Storage= 55 cf
Flood Elev= 275.00' Surf.Area= 1,466 sf Storage= 536 cf

Plug-Flow detention time= 2.5 min calculated for 612 cf (100% of inflow)
Center-of-Mass det. time= 2.5 min (777.5 - 774.9)

Volume	Invert	Avail.Storage	Storage Description
#1	272.00'	167 cf	3.50'W x 122.00'L x 1.75'H Prismatoid 747 cf Overall - 192 cf Embedded = 556 cf x 30.0% Voids
#2	272.25'	192 cf	12.0" Round Pipe Storage x 2 Inside #1 L= 122.0'
#3	273.75'	178 cf	Custom Stage Data (Irregular) Listed below (Recalc)
			536 cf Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
273.75	430	253.0	0	0	430
274.00	1,039	301.0	178	178	2,547

Device	Routing	Invert	Outlet Devices
#1	Primary	273.03'	12.0" Round Culvert L= 60.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.03' / 272.50' S= 0.0088 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Discarded	272.00'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.08 cfs @ 12.01 hrs HW=272.04' (Free Discharge)
2=Exfiltration (Exfiltration Controls 0.08 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=272.00' TW=266.85' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)

Summary for Pond UIS1: Underground Chamber #1

Inflow Area = 53,638 sf, 84.89% Impervious, Inflow Depth = 2.66" for 2 Year event
Inflow = 3.42 cfs @ 12.08 hrs, Volume= 11,906 cf
Outflow = 0.84 cfs @ 11.83 hrs, Volume= 11,906 cf, Atten= 75%, Lag= 0.0 min
Discarded = 0.84 cfs @ 11.83 hrs, Volume= 11,906 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.07' @ 12.46 hrs Surf.Area= 4,410 sf Storage= 2,265 cf

Plug-Flow detention time= 12.1 min calculated for 11,902 cf (100% of inflow)
Center-of-Mass det. time= 12.1 min (767.5 - 755.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	271.20'	4,137 cf	53.75'W x 82.05'L x 3.75'H Field A 16,538 cf Overall - 6,197 cf Embedded = 10,341 cf x 40.0% Voids
#2A	271.70'	6,197 cf	ADS StormTech SC-800 +Cap x 121 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 121 Chambers in 11 Rows Cap Storage= 3.4 cf x 2 x 11 rows = 75.2 cf
			10,333 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	271.70'	15.0" Round Culvert L= 139.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 271.70' / 269.85' S= 0.0133 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf
#2	Device 1	274.30'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Discarded	271.20'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.84 cfs @ 11.83 hrs HW=271.24' (Free Discharge)
3=Exfiltration (Exfiltration Controls 0.84 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=271.20' TW=0.00' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond UIS2: Underground Chamber #2

Inflow Area = 22,636 sf, 23.54% Impervious, Inflow Depth = 1.14" for 2 Year event
Inflow = 0.63 cfs @ 12.08 hrs, Volume= 2,152 cf
Outflow = 0.24 cfs @ 12.31 hrs, Volume= 2,152 cf, Atten= 61%, Lag= 13.6 min
Discarded = 0.09 cfs @ 11.69 hrs, Volume= 2,053 cf
Primary = 0.15 cfs @ 12.31 hrs, Volume= 99 cf
Routed to Pond LP-1 : Low Point - MAHW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.81' @ 12.31 hrs Surf.Area= 470 sf Storage= 519 cf
Flood Elev= 275.00' Surf.Area= 470 sf Storage= 580 cf

Plug-Flow detention time= 30.2 min calculated for 2,151 cf (100% of inflow)
Center-of-Mass det. time= 30.2 min (792.7 - 762.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	271.80'	344 cf	14.83'W x 31.68'L x 2.33'H Field A 1,096 cf Overall - 236 cf Embedded = 861 cf x 40.0% Voids
#2A	272.30'	236 cf	ADS_StormTech SC-310 +Cap x 16 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 16 Chambers in 4 Rows
			580 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	272.60'	12.0" Round Culvert L= 60.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 272.60' / 272.00' S= 0.0100 ' /' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	273.75'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Discarded	271.80'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.09 cfs @ 11.69 hrs HW=271.84' (Free Discharge)
↳ **3=Exfiltration** (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.15 cfs @ 12.31 hrs HW=273.81' TW=267.98' (Dynamic Tailwater)
↳ **1=Culvert** (Passes 0.15 cfs of 2.51 cfs potential flow)
↳ **2=Broad-Crested Rectangular Weir** (Weir Controls 0.15 cfs @ 0.67 fps)

Summary for Pond UIS3: Underground Chamber #3

Inflow Area = 84,596 sf, 90.26% Impervious, Inflow Depth = 2.83" for 2 Year event
Inflow = 5.74 cfs @ 12.08 hrs, Volume= 19,963 cf
Outflow = 1.33 cfs @ 11.81 hrs, Volume= 19,963 cf, Atten= 77%, Lag= 0.0 min
Discarded = 1.33 cfs @ 11.81 hrs, Volume= 19,963 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.96' @ 12.47 hrs Surf.Area= 6,945 sf Storage= 4,026 cf

Plug-Flow detention time= 14.0 min calculated for 19,958 cf (100% of inflow)
Center-of-Mass det. time= 14.0 min (769.4 - 755.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	272.00'	6,516 cf	30.00'W x 231.50'L x 3.75'H Field A 26,044 cf Overall - 9,755 cf Embedded = 16,289 cf x 40.0% Voids
#2A	272.50'	9,755 cf	ADS_StormTech SC-800 +Cap x 192 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 192 Chambers in 6 Rows Cap Storage= 3.4 cf x 2 x 6 rows = 41.0 cf
			16,270 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	273.29'	12.0" Round Culvert L= 110.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 273.29' / 272.00' S= 0.0117 ' /' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	275.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Device 1	273.50'	9.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	272.00'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=1.33 cfs @ 11.81 hrs HW=272.04' (Free Discharge)
↳ **4=Exfiltration** (Exfiltration Controls 1.33 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=272.00' TW=0.00' (Dynamic Tailwater)
↳ **1=Culvert** (Controls 0.00 cfs)
↳ **2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)
↳ **3=Orifice/Grate** (Controls 0.00 cfs)

Summary for Pond UIS4: Underground Chamber #4

Inflow Area = 42,721 sf, 77.28% Impervious, Inflow Depth = 2.42" for 2 Year event
Inflow = 2.48 cfs @ 12.08 hrs, Volume= 8,633 cf
Outflow = 0.46 cfs @ 11.75 hrs, Volume= 8,633 cf, Atten= 82%, Lag= 0.0 min
Discarded = 0.46 cfs @ 11.75 hrs, Volume= 8,633 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.82' @ 12.52 hrs Surf.Area= 2,398 sf Storage= 2,075 cf

Plug-Flow detention time= 22.9 min calculated for 8,633 cf (100% of inflow)
Center-of-Mass det. time= 22.9 min (778.5 - 755.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	272.50'	2,279 cf	39.50'W x 60.70'L x 3.75'H Field A 8,991 cf Overall - 3,293 cf Embedded = 5,699 cf x 40.0% Voids
#2A	273.00'	3,293 cf	ADS_StormTech SC-800 +Cap x 64 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 64 Chambers in 8 Rows Cap Storage= 3.4 cf x 2 x 8 rows = 54.7 cf
		5,572 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	273.00'	12.0" Round Culvert L= 264.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 273.00' / 271.00' S= 0.0076 ' /' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	276.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Device 1	274.25'	9.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	272.50'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.46 cfs @ 11.75 hrs HW=272.54' (Free Discharge)
4=Exfiltration (Exfiltration Controls 0.46 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=272.50' TW=0.00' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
3=Orifice/Grate (Controls 0.00 cfs)

Summary for Pond UIS5: Underground Chamber #5

Inflow Area = 157,891 sf, 63.68% Impervious, Inflow Depth = 2.00" for 2 Year event
Inflow = 7.56 cfs @ 12.08 hrs, Volume= 26,303 cf
Outflow = 1.66 cfs @ 11.81 hrs, Volume= 26,303 cf, Atten= 78%, Lag= 0.0 min
Discarded = 1.66 cfs @ 11.81 hrs, Volume= 26,303 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Pond MH19 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.09' @ 12.49 hrs Surf.Area= 8,653 sf Storage= 5,561 cf

Plug-Flow detention time= 15.9 min calculated for 26,303 cf (100% of inflow)
Center-of-Mass det. time= 15.9 min (771.6 - 755.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	269.80'	11,567 cf	30.00'W x 288.43'L x 4.75'H Field A 41,102 cf Overall - 12,183 cf Embedded = 28,919 cf x 40.0% Voids
#2A	270.80'	12,183 cf	ADS_StormTech SC-800 +Cap x 240 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 240 Chambers in 6 Rows Cap Storage= 3.4 cf x 2 x 6 rows = 41.0 cf
		23,751 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	273.05'	12.0" Round Culvert L= 97.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 273.05' / 272.00' S= 0.0108 ' /' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	274.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Discarded	269.80'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=1.66 cfs @ 11.81 hrs HW=269.85' (Free Discharge)
3=Exfiltration (Exfiltration Controls 1.66 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=269.80' TW=269.87' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Link SP1: On Site Flow To Perennial Stream

Inflow Area = 210,774 sf, 10.22% Impervious, Inflow Depth = 0.07" for 2 Year event
Inflow = 0.83 cfs @ 12.16 hrs, Volume= 1,145 cf
Primary = 0.83 cfs @ 12.16 hrs, Volume= 1,145 cf, Atten= 0%, Lag= 0.0 min
Routed to Pond LP-2 : Low Point - MAHW

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP2: On Site Flow To Perennial Stream

Inflow Area = 236,963 sf, 10.14% Impervious, Inflow Depth = 0.09" for 2 Year event
Inflow = 0.78 cfs @ 12.23 hrs, Volume= 1,796 cf
Primary = 0.78 cfs @ 12.23 hrs, Volume= 1,796 cf, Atten= 0%, Lag= 0.0 min
Routed to Link SP3 : On Site Flow To Perennial Stream

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP3: On Site Flow To Perennial Stream

Inflow Area = 867,482 sf, 36.39% Impervious, Inflow Depth = 0.16" for 2 Year event
Inflow = 3.08 cfs @ 12.17 hrs, Volume= 11,850 cf
Primary = 3.08 cfs @ 12.17 hrs, Volume= 11,850 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment CB1: Subcat CB1	Runoff Area=10,622 sf 86.84% Impervious Runoff Depth=4.36" Tc=6.0 min CN=WQ Runoff=1.08 cfs 3,859 cf
Subcatchment CB10A: Subcat CB10A	Runoff Area=2,889 sf 81.34% Impervious Runoff Depth=4.10" Tc=6.0 min CN=WQ Runoff=0.28 cfs 987 cf
Subcatchment CB10B: Subcat CB10B	Runoff Area=10,414 sf 86.69% Impervious Runoff Depth=4.35" Tc=6.0 min CN=WQ Runoff=1.06 cfs 3,777 cf
Subcatchment CB11: Subcat CB11	Runoff Area=11,505 sf 85.23% Impervious Runoff Depth=4.28" Tc=6.0 min CN=WQ Runoff=1.15 cfs 4,107 cf
Subcatchment CB12: Subcat CB12	Runoff Area=7,149 sf 71.54% Impervious Runoff Depth=3.63" Tc=6.0 min CN=WQ Runoff=0.60 cfs 2,165 cf
Subcatchment CB13: Subcat CB13	Runoff Area=4,141 sf 88.54% Impervious Runoff Depth=4.44" Tc=6.0 min CN=WQ Runoff=0.43 cfs 1,532 cf
Subcatchment CB14: Subcat CB14	Runoff Area=6,167 sf 87.00% Impervious Runoff Depth=4.37" Tc=6.0 min CN=WQ Runoff=0.63 cfs 2,244 cf
Subcatchment CB15: Subcat CB15	Runoff Area=20,154 sf 44.65% Impervious Runoff Depth=2.36" Tc=6.0 min CN=WQ Runoff=1.06 cfs 3,966 cf
Subcatchment CB16: Subcat CB16	Runoff Area=20,584 sf 35.58% Impervious Runoff Depth=1.93" Tc=6.0 min CN=WQ Runoff=0.86 cfs 3,314 cf
Subcatchment CB17: Subcat CB17	Runoff Area=36,035 sf 27.88% Impervious Runoff Depth=1.57" Tc=6.0 min CN=WQ Runoff=1.18 cfs 4,706 cf
Subcatchment CB2: Subcat CB2	Runoff Area=12,976 sf 8.11% Impervious Runoff Depth=0.63" Tc=6.0 min CN=WQ Runoff=0.12 cfs 682 cf
Subcatchment CB3: Subcat CB3	Runoff Area=9,660 sf 44.27% Impervious Runoff Depth=3.79" Tc=6.0 min CN=WQ Runoff=0.86 cfs 3,053 cf
Subcatchment CB4A: Subcat CB4A	Runoff Area=3,976 sf 91.93% Impervious Runoff Depth=4.60" Tc=6.0 min CN=WQ Runoff=0.43 cfs 1,524 cf
Subcatchment CB4B: Subcat CB4B	Runoff Area=10,218 sf 73.42% Impervious Runoff Depth=3.72" Tc=6.0 min CN=WQ Runoff=0.88 cfs 3,171 cf
Subcatchment CB4C: Subcat CB4C	Runoff Area=10,741 sf 84.88% Impervious Runoff Depth=4.27" Tc=6.0 min CN=WQ Runoff=1.07 cfs 3,819 cf
Subcatchment CB4D: Subcat CB4D	Runoff Area=10,203 sf 79.98% Impervious Runoff Depth=4.03" Tc=6.0 min CN=WQ Runoff=0.96 cfs 3,430 cf

Subcatchment CB5A: Subcat CB5A	Runoff Area=10,459 sf 93.11% Impervious Runoff Depth=4.66" Tc=6.0 min CN=WQ Runoff=1.14 cfs 4,058 cf
Subcatchment CB5B: Subcat CB5B	Runoff Area=3,865 sf 95.02% Impervious Runoff Depth=4.75" Tc=6.0 min CN=WQ Runoff=0.43 cfs 1,529 cf
Subcatchment CB5C: Subcat CB5C	Runoff Area=2,619 sf 91.40% Impervious Runoff Depth=4.58" Tc=6.0 min CN=WQ Runoff=0.28 cfs 999 cf
Subcatchment CB6: Subcat CB6	Runoff Area=12,631 sf 77.41% Impervious Runoff Depth=3.91" Tc=6.0 min CN=WQ Runoff=1.15 cfs 4,119 cf
Subcatchment CB7A: Subcat CB7A	Runoff Area=7,101 sf 79.91% Impervious Runoff Depth=4.03" Tc=6.0 min CN=WQ Runoff=0.67 cfs 2,386 cf
Subcatchment CB7B: Subcat CB7B	Runoff Area=11,340 sf 75.16% Impervious Runoff Depth=3.81" Tc=6.0 min CN=WQ Runoff=1.00 cfs 3,597 cf
Subcatchment CB8: Subcat CB8	Runoff Area=10,991 sf 81.26% Impervious Runoff Depth=4.10" Tc=6.0 min CN=WQ Runoff=1.05 cfs 3,751 cf
Subcatchment CB9A: Subcat CB9A	Runoff Area=11,743 sf 81.25% Impervious Runoff Depth=4.09" Tc=6.0 min CN=WQ Runoff=1.12 cfs 4,007 cf
Subcatchment CB9B: Subcat CB9B	Runoff Area=14,736 sf 67.40% Impervious Runoff Depth=3.44" Tc=6.0 min CN=WQ Runoff=1.16 cfs 4,223 cf
Subcatchment CB9C: Subcat CB9C	Runoff Area=3,708 sf 100.00% Impervious Runoff Depth=4.98" Tc=6.0 min CN=WQ Runoff=0.43 cfs 1,540 cf
Subcatchment CB9D: Subcat CB9D	Runoff Area=1,543 sf 58.43% Impervious Runoff Depth=3.01" Tc=6.0 min CN=WQ Runoff=0.11 cfs 387 cf
Subcatchment P-1: Subcat P-1	Runoff Area=14,881 sf 16.65% Impervious Runoff Depth=1.04" Tc=6.0 min CN=WQ Runoff=0.29 cfs 1,284 cf
Subcatchment P-2: Subcat P-2	Runoff Area=11,308 sf 0.00% Impervious Runoff Depth=1.25" Tc=6.0 min CN=WQ Runoff=0.29 cfs 1,174 cf
Subcatchment P-3: Subcat P-3	Runoff Area=7,515 sf 5.81% Impervious Runoff Depth=0.52" Tc=6.0 min CN=WQ Runoff=0.05 cfs 327 cf
Subcatchment P-4: Subcat P-4	Runoff Area=291,674 sf 12.39% Impervious Runoff Depth=0.74" Tc=12.0 min CN=WQ Runoff=3.73 cfs 17,896 cf
Subcatchment P-5: Subcat P-5	Runoff Area=76,040 sf 11.69% Impervious Runoff Depth=0.80" Tc=8.0 min CN=WQ Runoff=0.97 cfs 5,072 cf
Subcatchment P-6: Subcat P-6	Runoff Area=25,354 sf 10.27% Impervious Runoff Depth=0.73" Tc=6.0 min CN=WQ Runoff=0.31 cfs 1,549 cf

Subcatchment P-7: Subcat P-7	Runoff Area=79,230 sf 5.41% Impervious Runoff Depth=0.50" Tc=11.4 min CN=WQ Runoff=0.42 cfs 3,321 cf
Subcatchment PATIO: Subcat PATIO	Runoff Area=2,833 sf 100.00% Impervious Runoff Depth=4.98" Tc=6.0 min CN=98 Runoff=0.33 cfs 1,176 cf
Subcatchment R-1: Subcat R-1	Runoff Area=5,046 sf 100.00% Impervious Runoff Depth=4.98" Tc=6.0 min CN=98 Runoff=0.59 cfs 2,095 cf
Subcatchment R-2: Subcat R-2	Runoff Area=18,290 sf 100.00% Impervious Runoff Depth=4.98" Tc=6.0 min CN=98 Runoff=2.15 cfs 7,595 cf
Subcatchment R-3: Subcat R-3	Runoff Area=18,290 sf 100.00% Impervious Runoff Depth=4.98" Tc=6.0 min CN=98 Runoff=2.15 cfs 7,595 cf
Subcatchment R-4: Subcat R-4	Runoff Area=18,290 sf 100.00% Impervious Runoff Depth=4.98" Tc=6.0 min CN=98 Runoff=2.15 cfs 7,595 cf
Subcatchment R-5: Subcat R-5	Runoff Area=20,565 sf 100.00% Impervious Runoff Depth=4.98" Tc=6.0 min CN=98 Runoff=2.41 cfs 8,539 cf
Pond 1: Catch Basin	Peak Elev=272.85' Inflow=1.08 cfs 3,859 cf 12.0" Round Culvert n=0.012 L=84.0' S=0.0060 ' / ' Outflow=1.08 cfs 3,859 cf
Pond 2: Catch Basin	Peak Elev=273.96' Inflow=0.12 cfs 682 cf 12.0" Round Culvert n=0.012 L=47.0' S=0.0057 ' / ' Outflow=0.13 cfs 682 cf
Pond 3: Catch Basin	Peak Elev=274.02' Inflow=0.86 cfs 3,053 cf 12.0" Round Culvert n=0.012 L=77.0' S=0.0052 ' / ' Outflow=0.86 cfs 3,053 cf
Pond 4A: Catch Basin	Peak Elev=273.04' Inflow=0.43 cfs 1,524 cf 12.0" Round Culvert n=0.012 L=7.0' S=0.0100 ' / ' Outflow=0.43 cfs 1,524 cf
Pond 4B: Catch Basin	Peak Elev=273.15' Inflow=0.88 cfs 3,171 cf 12.0" Round Culvert n=0.012 L=27.0' S=0.0100 ' / ' Outflow=0.88 cfs 3,171 cf
Pond 4C: Catch Basin	Peak Elev=273.91' Inflow=1.07 cfs 3,819 cf 12.0" Round Culvert n=0.012 L=61.0' S=0.0077 ' / ' Outflow=1.07 cfs 3,819 cf
Pond 4D: Catch Basin	Peak Elev=273.80' Inflow=0.96 cfs 3,430 cf 12.0" Round Culvert n=0.012 L=38.0' S=0.0100 ' / ' Outflow=0.96 cfs 3,430 cf
Pond 5A: Catch Basin	Peak Elev=274.12' Inflow=1.14 cfs 4,058 cf 12.0" Round Culvert n=0.012 L=28.0' S=0.0100 ' / ' Outflow=1.14 cfs 4,058 cf
Pond 5B: Catch Basin	Peak Elev=274.05' Inflow=0.43 cfs 1,529 cf 12.0" Round Culvert n=0.012 L=45.0' S=0.0100 ' / ' Outflow=0.43 cfs 1,529 cf
Pond 5C: Catch Basin	Peak Elev=274.19' Inflow=0.28 cfs 999 cf 12.0" Round Culvert n=0.012 L=41.0' S=0.0100 ' / ' Outflow=0.28 cfs 999 cf

Pond 6: Catch Basin	Peak Elev=273.76' Inflow=1.15 cfs 4,119 cf 12.0" Round Culvert n=0.012 L=26.0' S=0.0100 'l' Outflow=1.15 cfs 4,119 cf
Pond 7A: Catch Basin	Peak Elev=273.76' Inflow=0.67 cfs 2,386 cf 12.0" Round Culvert n=0.012 L=10.0' S=0.0100 'l' Outflow=0.67 cfs 2,386 cf
Pond 7B: Catch Basin	Peak Elev=273.82' Inflow=1.00 cfs 3,597 cf 12.0" Round Culvert n=0.012 L=40.0' S=0.0100 'l' Outflow=1.00 cfs 3,597 cf
Pond 8: Catch Basin	Peak Elev=274.70' Inflow=1.05 cfs 3,751 cf 12.0" Round Culvert n=0.012 L=27.0' S=0.0100 'l' Outflow=1.05 cfs 3,751 cf
Pond 9A: Catch Basin	Peak Elev=274.70' Inflow=1.12 cfs 4,007 cf 12.0" Round Culvert n=0.012 L=14.0' S=0.0050 'l' Outflow=1.12 cfs 4,007 cf
Pond 9B: Catch Basin	Peak Elev=274.70' Inflow=1.16 cfs 4,223 cf 12.0" Round Culvert n=0.012 L=64.0' S=0.0059 'l' Outflow=1.16 cfs 4,223 cf
Pond 9C: Catch Basin	Peak Elev=274.69' Inflow=0.43 cfs 1,540 cf 12.0" Round Culvert n=0.012 L=7.0' S=0.0114 'l' Outflow=0.43 cfs 1,540 cf
Pond 9D: Catch Basin	Peak Elev=274.69' Inflow=0.11 cfs 387 cf 12.0" Round Culvert n=0.012 L=49.0' S=0.0090 'l' Outflow=0.11 cfs 387 cf
Pond 10A: Catch Basin	Peak Elev=271.97' Inflow=0.28 cfs 987 cf 12.0" Round Culvert n=0.012 L=87.0' S=0.0100 'l' Outflow=0.28 cfs 987 cf
Pond 10B: Catch Basin	Peak Elev=271.97' Inflow=1.06 cfs 3,777 cf 12.0" Round Culvert n=0.012 L=15.0' S=0.0100 'l' Outflow=1.06 cfs 3,777 cf
Pond 11: Catch Basin	Peak Elev=271.97' Inflow=1.15 cfs 4,107 cf 12.0" Round Culvert n=0.012 L=15.0' S=0.0100 'l' Outflow=1.15 cfs 4,107 cf
Pond 12: Catch Basin	Peak Elev=271.98' Inflow=0.60 cfs 2,165 cf 12.0" Round Culvert n=0.012 L=14.0' S=0.0100 'l' Outflow=0.60 cfs 2,165 cf
Pond 13: Catch Basin	Peak Elev=272.23' Inflow=0.43 cfs 1,532 cf 12.0" Round Culvert n=0.012 L=87.0' S=0.0100 'l' Outflow=0.43 cfs 1,532 cf
Pond 14: Catch Basin	Peak Elev=272.01' Inflow=0.63 cfs 2,244 cf 12.0" Round Culvert n=0.012 L=33.0' S=0.0100 'l' Outflow=0.63 cfs 2,244 cf
Pond 15: Catch Basin	Peak Elev=273.16' Inflow=1.06 cfs 3,966 cf 12.0" Round Culvert n=0.012 L=19.0' S=0.0100 'l' Outflow=1.06 cfs 3,966 cf
Pond 16: Catch Basin	Peak Elev=274.87' Inflow=0.86 cfs 3,314 cf 12.0" Round Culvert n=0.012 L=19.0' S=0.0100 'l' Outflow=0.86 cfs 3,314 cf
Pond 17: Catch Basin	Peak Elev=275.54' Inflow=1.18 cfs 4,706 cf 15.0" Round Culvert n=0.012 L=143.0' S=0.0061 'l' Outflow=1.18 cfs 4,706 cf

Pond LP-1: Low Point - MAHW	Peak Elev=268.26' Storage=285 cf Inflow=2.26 cfs 3,742 cf Outflow=2.24 cfs 3,647 cf
Pond LP-2: Low Point - MAHW	Peak Elev=267.36' Storage=545 cf Inflow=2.50 cfs 4,930 cf 24.0" Round Culvert n=0.011 L=30.0' S=-0.0160 'l' Outflow=2.08 cfs 4,931 cf
Pond MH1: Manhole	Peak Elev=272.83' Inflow=1.08 cfs 3,859 cf 24.0" Round Culvert n=0.012 L=10.0' S=0.0000 'l' Outflow=1.08 cfs 3,859 cf
Pond MH11: Manhole	Peak Elev=274.68' Inflow=2.21 cfs 7,974 cf 24.0" Round Culvert n=0.012 L=4.0' S=0.0000 'l' Outflow=2.21 cfs 7,974 cf
Pond MH12: Manhole	Peak Elev=274.69' Inflow=1.66 cfs 5,934 cf 15.0" Round Culvert n=0.012 L=28.0' S=0.0057 'l' Outflow=1.66 cfs 5,934 cf
Pond MH13: Manhole	Peak Elev=274.68' Inflow=1.66 cfs 5,934 cf 24.0" Round Culvert n=0.012 L=10.0' S=0.0000 'l' Outflow=1.66 cfs 5,934 cf
Pond MH15: Manhole	Peak Elev=271.97' Inflow=1.33 cfs 4,764 cf 12.0" Round Culvert n=0.012 L=14.0' S=0.0100 'l' Outflow=1.33 cfs 4,764 cf
Pond MH16: Manhole	Peak Elev=271.97' Inflow=1.33 cfs 4,764 cf 24.0" Round Culvert n=0.012 L=5.0' S=0.0000 'l' Outflow=1.33 cfs 4,764 cf
Pond MH17: Manhole	Peak Elev=271.97' Inflow=1.15 cfs 4,107 cf 24.0" Round Culvert n=0.012 L=7.0' S=0.0000 'l' Outflow=1.15 cfs 4,107 cf
Pond MH19: Manhole	Peak Elev=269.87' Inflow=0.00 cfs 0 cf 18.0" Round Culvert n=0.012 L=51.0' S=0.0069 'l' Outflow=0.00 cfs 0 cf
Pond MH21: Manhole	Peak Elev=271.97' Inflow=4.75 cfs 17,927 cf 24.0" Round Culvert n=0.012 L=20.0' S=0.0100 'l' Outflow=4.75 cfs 17,927 cf
Pond MH22: Manhole	Peak Elev=271.98' Inflow=4.75 cfs 17,927 cf 24.0" Round Culvert n=0.012 L=22.0' S=0.0100 'l' Outflow=4.75 cfs 17,927 cf
Pond MH23: Manhole	Peak Elev=272.99' Inflow=3.09 cfs 11,986 cf 18.0" Round Culvert n=0.012 L=121.0' S=0.0100 'l' Outflow=3.09 cfs 11,986 cf
Pond MH24: Manhole	Peak Elev=274.64' Inflow=2.04 cfs 8,019 cf 18.0" Round Culvert n=0.012 L=176.0' S=0.0100 'l' Outflow=2.04 cfs 8,019 cf
Pond MH26: Manhole	Peak Elev=273.03' Inflow=3.33 cfs 11,944 cf 18.0" Round Culvert n=0.012 L=43.0' S=0.0100 'l' Outflow=3.33 cfs 11,944 cf
Pond MH27: Manhole	Peak Elev=273.96' Inflow=0.99 cfs 3,735 cf 15.0" Round Culvert n=0.012 L=8.0' S=0.0050 'l' Outflow=0.99 cfs 3,735 cf
Pond MH28: Manhole	Peak Elev=273.81' Inflow=1.85 cfs 6,586 cf 15.0" Round Culvert n=0.012 L=59.0' S=0.0100 'l' Outflow=1.85 cfs 6,586 cf

Pond MH29: Manhole	Peak Elev=273.22' Inflow=2.03 cfs 7,249 cf 15.0" Round Culvert n=0.012 L=11.0' S=0.0100 'l' Outflow=2.03 cfs 7,249 cf
Pond MH30: Manhole	Peak Elev=273.55' Inflow=2.03 cfs 7,249 cf 15.0" Round Culvert n=0.012 L=34.0' S=0.0100 'l' Outflow=2.03 cfs 7,249 cf
Pond MH31: Manhole	Peak Elev=273.86' Inflow=0.28 cfs 999 cf 12.0" Round Culvert n=0.012 L=20.0' S=0.0100 'l' Outflow=0.28 cfs 999 cf
Pond MH32: Manhole	Peak Elev=274.69' Inflow=1.22 cfs 4,394 cf 12.0" Round Culvert n=0.012 L=48.0' S=0.0052 'l' Outflow=1.22 cfs 4,394 cf
Pond MH33: Manhole	Peak Elev=274.69' Inflow=2.21 cfs 7,974 cf 15.0" Round Culvert n=0.012 L=20.0' S=0.0100 'l' Outflow=2.21 cfs 7,974 cf
Pond MH6: Manhole	Peak Elev=273.76' Inflow=1.85 cfs 6,586 cf 24.0" Round Culvert n=0.012 L=6.0' S=0.0000 'l' Outflow=1.85 cfs 6,586 cf
Pond MH7: Manhole	Peak Elev=273.75' Inflow=1.15 cfs 4,119 cf 24.0" Round Culvert n=0.012 L=6.0' S=0.0000 'l' Outflow=1.15 cfs 4,119 cf
Pond MH8: Manhole	Peak Elev=273.76' Inflow=1.67 cfs 5,982 cf 15.0" Round Culvert n=0.012 L=22.0' S=0.0100 'l' Outflow=1.67 cfs 5,982 cf
Pond MH9: Manhole	Peak Elev=273.75' Inflow=1.67 cfs 5,982 cf 24.0" Round Culvert n=0.012 L=8.0' S=0.0000 'l' Outflow=1.67 cfs 5,982 cf
Pond RD1: Roof Drain	Peak Elev=272.83' Inflow=0.92 cfs 3,271 cf 12.0" Round Culvert n=0.012 L=21.0' S=0.0100 'l' Outflow=0.92 cfs 3,271 cf
Pond RD2: Roof Drain	Peak Elev=273.77' Inflow=2.15 cfs 7,595 cf 12.0" Round Culvert n=0.012 L=31.0' S=0.0100 'l' Outflow=2.15 cfs 7,595 cf
Pond RD3: Roof Drain	Peak Elev=273.76' Inflow=2.15 cfs 7,595 cf 12.0" Round Culvert n=0.012 L=14.5' S=0.0103 'l' Outflow=2.15 cfs 7,595 cf
Pond RD4: Roof Drain	Peak Elev=271.98' Inflow=2.15 cfs 7,595 cf 12.0" Round Culvert n=0.012 L=24.0' S=0.0104 'l' Outflow=2.15 cfs 7,595 cf
Pond RD5: Roof Drain	Peak Elev=271.97' Inflow=2.41 cfs 8,539 cf 15.0" Round Culvert n=0.012 L=18.0' S=0.0100 'l' Outflow=2.41 cfs 8,539 cf
Pond TD1: Stone Trench Drain	Peak Elev=272.12' Storage=192 cf Inflow=1.66 cfs 9,942 cf Discarded=0.20 cfs 7,234 cf Primary=1.40 cfs 2,709 cf Outflow=1.60 cfs 9,942 cf
Pond TD2: Stone Trench Drain	Peak Elev=272.78' Storage=173 cf Inflow=0.29 cfs 1,174 cf Discarded=0.08 cfs 1,174 cf Primary=0.00 cfs 0 cf Outflow=0.08 cfs 1,174 cf
Pond UIS1: Underground Chamber #1	Peak Elev=272.83' Storage=4,939 cf Inflow=5.34 cfs 19,074 cf Discarded=0.84 cfs 19,074 cf Primary=0.00 cfs 0 cf Outflow=0.84 cfs 19,074 cf

Pond UIS2: Underground Chamber #2	Peak Elev=273.93' Storage=542 cf Inflow=0.99 cfs 3,735 cf Discarded=0.09 cfs 3,029 cf Primary=0.88 cfs 707 cf Outflow=0.97 cfs 3,735 cf
Pond UIS3: Underground Chamber #3	Peak Elev=273.75' Storage=8,422 cf Inflow=8.96 cfs 31,877 cf Discarded=1.33 cfs 31,507 cf Primary=0.23 cfs 370 cf Outflow=1.56 cfs 31,877 cf
Pond UIS4: Underground Chamber #4	Peak Elev=274.68' Storage=3,618 cf Inflow=3.87 cfs 13,908 cf Discarded=0.46 cfs 12,941 cf Primary=0.58 cfs 966 cf Outflow=1.04 cfs 13,908 cf
Pond UIS5: Underground Chamber #5	Peak Elev=271.97' Storage=11,673 cf Inflow=11.79 cfs 42,932 cf Discarded=1.66 cfs 42,932 cf Primary=0.00 cfs 0 cf Outflow=1.66 cfs 42,932 cf
Link SP1: On Site Flow To Perennial Stream	Inflow=2.24 cfs 3,647 cf Primary=2.24 cfs 3,647 cf
Link SP2: On Site Flow To Perennial Stream	Inflow=2.08 cfs 4,931 cf Primary=2.08 cfs 4,931 cf
Link SP3: On Site Flow To Perennial Stream	Inflow=5.77 cfs 24,162 cf Primary=5.77 cfs 24,162 cf

Total Runoff Area = 867,482 sf Runoff Volume = 142,148 cf Average Runoff Depth = 1.97"
63.61% Pervious = 551,848 sf 36.39% Impervious = 315,634 sf

Summary for Subcatchment CB1: Subcat CB1

Runoff = 1.08 cfs @ 12.08 hrs, Volume= 3,859 cf, Depth= 4.36"
Routed to Pond 1 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
1,398	39	>75% Grass cover, Good, HSG A
9,224	98	Paved parking, HSG A
10,622		Weighted Average
1,398	39	13.16% Pervious Area
9,224	98	86.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB10A: Subcat CB10A

Runoff = 0.28 cfs @ 12.08 hrs, Volume= 987 cf, Depth= 4.10"
Routed to Pond 10A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
539	39	>75% Grass cover, Good, HSG A
2,350	98	Paved parking, HSG A
2,889		Weighted Average
539	39	18.66% Pervious Area
2,350	98	81.34% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB10B: Subcat CB10B

Runoff = 1.06 cfs @ 12.08 hrs, Volume= 3,777 cf, Depth= 4.35"
Routed to Pond 10B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
1,386	39	>75% Grass cover, Good, HSG A
9,027	98	Paved parking, HSG A
10,414		Weighted Average
1,386	39	13.31% Pervious Area
9,027	98	86.69% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB11: Subcat CB11

Runoff = 1.15 cfs @ 12.08 hrs, Volume= 4,107 cf, Depth= 4.28"
Routed to Pond 11 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
1,699	39	>75% Grass cover, Good, HSG A
9,806	98	Paved parking, HSG A
11,505		Weighted Average
1,699	39	14.77% Pervious Area
9,806	98	85.23% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB12: Subcat CB12

Runoff = 0.60 cfs @ 12.08 hrs, Volume= 2,165 cf, Depth= 3.63"
Routed to Pond 12 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
2,035	39	>75% Grass cover, Good, HSG A
5,114	98	Paved parking, HSG A
7,149		Weighted Average
2,035	39	28.46% Pervious Area
5,114	98	71.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB13: Subcat CB13

Runoff = 0.43 cfs @ 12.08 hrs, Volume= 1,532 cf, Depth= 4.44"
Routed to Pond 13 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
474	39	>75% Grass cover, Good, HSG A
3,666	98	Paved parking, HSG A
4,141		Weighted Average
474	39	11.46% Pervious Area
3,666	98	88.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB14: Subcat CB14

Runoff = 0.63 cfs @ 12.08 hrs, Volume= 2,244 cf, Depth= 4.37"
Routed to Pond 14 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
801	39	>75% Grass cover, Good, HSG A
4,765	98	Paved parking, HSG A
600	98	Roofs, HSG A
6,167		Weighted Average
801	39	13.00% Pervious Area
5,365	98	87.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB15: Subcat CB15

Runoff = 1.06 cfs @ 12.08 hrs, Volume= 3,966 cf, Depth= 2.36"
Routed to Pond 15 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
11,154	39	>75% Grass cover, Good, HSG A
8,024	98	Paved parking, HSG A
975	98	Roofs, HSG A
20,154		Weighted Average
11,154	39	55.35% Pervious Area
9,000	98	44.65% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB16: Subcat CB16

Runoff = 0.86 cfs @ 12.08 hrs, Volume= 3,314 cf, Depth= 1.93"
Routed to Pond 16 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
13,260	39	>75% Grass cover, Good, HSG A
7,324	98	Paved parking, HSG A
20,584		Weighted Average
13,260	39	64.42% Pervious Area
7,324	98	35.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB17: Subcat CB17

Runoff = 1.18 cfs @ 12.08 hrs, Volume= 4,706 cf, Depth= 1.57"
Routed to Pond 17 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
25,990	39	>75% Grass cover, Good, HSG A
8,281	98	Paved parking, HSG A
1,764	98	Roofs, HSG A
36,035		Weighted Average
25,990	39	72.12% Pervious Area
10,045	98	27.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB2: Subcat CB2

Runoff = 0.12 cfs @ 12.00 hrs, Volume= 682 cf, Depth= 0.63"
Routed to Pond 2 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
11,923	39	>75% Grass cover, Good, HSG A
1,053	98	Paved parking, HSG A
12,976		Weighted Average
11,923	39	91.89% Pervious Area
1,053	98	8.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB3: Subcat CB3

Runoff = 0.86 cfs @ 12.08 hrs, Volume= 3,053 cf, Depth= 3.79"
Routed to Pond 3 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
2,275	39	>75% Grass cover, Good, HSG A
3,109	96	Gravel surface, HSG A
4,276	98	Paved parking, HSG A
9,660		Weighted Average
5,384	72	55.73% Pervious Area
4,276	98	44.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4A: Subcat CB4A

Runoff = 0.43 cfs @ 12.08 hrs, Volume= 1,524 cf, Depth= 4.60"
Routed to Pond 4A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
321	39	>75% Grass cover, Good, HSG A
3,655	98	Paved parking, HSG A
3,976		Weighted Average
321	39	8.07% Pervious Area
3,655	98	91.93% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4B: Subcat CB4B

Runoff = 0.88 cfs @ 12.08 hrs, Volume= 3,171 cf, Depth= 3.72"
Routed to Pond 4B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
2,716	39	>75% Grass cover, Good, HSG A
7,501	98	Paved parking, HSG A
10,218		Weighted Average
2,716	39	26.58% Pervious Area
7,501	98	73.42% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4C: Subcat CB4C

Runoff = 1.07 cfs @ 12.08 hrs, Volume= 3,819 cf, Depth= 4.27"
Routed to Pond 4C : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
1,624	39	>75% Grass cover, Good, HSG A
7,101	98	Paved parking, HSG A
2,016	98	Roofs, HSG A
10,741		Weighted Average
1,624	39	15.12% Pervious Area
9,117	98	84.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4D: Subcat CB4D

Runoff = 0.96 cfs @ 12.08 hrs, Volume= 3,430 cf, Depth= 4.03"
Routed to Pond 4D : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
2,043	39	>75% Grass cover, Good, HSG A
8,160	98	Paved parking, HSG A
10,203		Weighted Average
2,043	39	20.02% Pervious Area
8,160	98	79.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB5A: Subcat CB5A

Runoff = 1.14 cfs @ 12.08 hrs, Volume= 4,058 cf, Depth= 4.66"
Routed to Pond 5A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
721	39	>75% Grass cover, Good, HSG A
9,224	98	Paved parking, HSG A
514	98	Roofs, HSG A
10,459		Weighted Average
721	39	6.89% Pervious Area
9,738	98	93.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB5B: Subcat CB5B

Runoff = 0.43 cfs @ 12.08 hrs, Volume= 1,529 cf, Depth= 4.75"
Routed to Pond 5B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
193	39	>75% Grass cover, Good, HSG A
2,913	98	Paved parking, HSG A
760	98	Roofs, HSG A
3,865		Weighted Average
193	39	4.98% Pervious Area
3,673	98	95.02% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB5C: Subcat CB5C

Runoff = 0.28 cfs @ 12.08 hrs, Volume= 999 cf, Depth= 4.58"
Routed to Pond 5C : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
225	39	>75% Grass cover, Good, HSG A
2,394	98	Paved parking, HSG A
2,619		Weighted Average
225	39	8.60% Pervious Area
2,394	98	91.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB6: Subcat CB6

Runoff = 1.15 cfs @ 12.08 hrs, Volume= 4,119 cf, Depth= 3.91"
Routed to Pond 6 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
2,854	39	>75% Grass cover, Good, HSG A
9,777	98	Paved parking, HSG A
12,631		Weighted Average
2,854	39	22.59% Pervious Area
9,777	98	77.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment CB7A: Subcat CB7A

Runoff = 0.67 cfs @ 12.08 hrs, Volume= 2,386 cf, Depth= 4.03"
Routed to Pond 7A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
1,426	39	>75% Grass cover, Good, HSG A
5,674	98	Paved parking, HSG A
7,101		Weighted Average
1,426	39	20.09% Pervious Area
5,674	98	79.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB7B: Subcat CB7B

Runoff = 1.00 cfs @ 12.08 hrs, Volume= 3,597 cf, Depth= 3.81"
Routed to Pond 7B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
2,817	39	>75% Grass cover, Good, HSG A
7,718	98	Paved parking, HSG A
805	98	Roofs, HSG A
11,340		Weighted Average
2,817	39	24.84% Pervious Area
8,523	98	75.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB8: Subcat CB8

Runoff = 1.05 cfs @ 12.08 hrs, Volume= 3,751 cf, Depth= 4.10"
Routed to Pond 8 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
2,060	39	>75% Grass cover, Good, HSG A
8,538	98	Paved parking, HSG A
393	98	Roofs, HSG A
10,991		Weighted Average
2,060	39	18.74% Pervious Area
8,931	98	81.26% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9A: Subcat CB9A

Runoff = 1.12 cfs @ 12.08 hrs, Volume= 4,007 cf, Depth= 4.09"
Routed to Pond 9A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
2,202	39	>75% Grass cover, Good, HSG A
7,525	98	Paved parking, HSG A
2,016	98	Roofs, HSG A
11,743		Weighted Average
2,202	39	18.75% Pervious Area
9,541	98	81.25% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9B: Subcat CB9B

Runoff = 1.16 cfs @ 12.08 hrs, Volume= 4,223 cf, Depth= 3.44"
Routed to Pond 9B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
4,805	39	>75% Grass cover, Good, HSG A
9,931	98	Paved parking, HSG A
14,736		Weighted Average
4,805	39	32.60% Pervious Area
9,931	98	67.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9C: Subcat CB9C

Runoff = 0.43 cfs @ 12.08 hrs, Volume= 1,540 cf, Depth= 4.98"
Routed to Pond 9C : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
0	39	>75% Grass cover, Good, HSG A
2,337	98	Paved parking, HSG A
1,371	98	Roofs, HSG A
3,708		Weighted Average
0	39	0.00% Pervious Area
3,708	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9D: Subcat CB9D

Runoff = 0.11 cfs @ 12.08 hrs, Volume= 387 cf, Depth= 3.01"
Routed to Pond 9D : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
641	39	>75% Grass cover, Good, HSG A
901	98	Paved parking, HSG A
1,543		Weighted Average
641	39	41.57% Pervious Area
901	98	58.43% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-1: Subcat P-1

Runoff = 0.29 cfs @ 12.08 hrs, Volume= 1,284 cf, Depth= 1.04"
Routed to Pond LP-2 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
12,404	39	>75% Grass cover, Good, HSG A
0	98	Paved parking, HSG A
2,477	98	Water Surface, HSG A
14,881		Weighted Average
12,404	39	83.35% Pervious Area
2,477	98	16.65% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-2: Subcat P-2

Runoff = 0.29 cfs @ 12.08 hrs, Volume= 1,174 cf, Depth= 1.25"
Routed to Pond TD2 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
8,801	39	>75% Grass cover, Good, HSG A
2,507	96	Gravel surface, HSG A
0	98	Paved parking, HSG A
11,308		Weighted Average
11,308	52	100.00% Pervious Area
0	98	0.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-3: Subcat P-3

Runoff = 0.05 cfs @ 12.08 hrs, Volume= 327 cf, Depth= 0.52"
Routed to Pond LP-1 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
7,078	39	>75% Grass cover, Good, HSG A
5	98	Paved parking, HSG A
431	98	Water Surface, HSG A
7,515		Weighted Average
7,078	39	94.19% Pervious Area
436	98	5.81% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-4: Subcat P-4

Runoff = 3.73 cfs @ 12.16 hrs, Volume= 17,896 cf, Depth= 0.74"
Routed to Link SP3 : On Site Flow To Perennial Stream

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
85,817	39	>75% Grass cover, Good, HSG A
2,376	96	Gravel surface, HSG A
5,773	98	Paved parking, HSG A
30,379	98	Water Surface, HSG A
167,329	30	Woods, Good, HSG A
291,674		Weighted Average
255,522	34	87.61% Pervious Area
36,152	98	12.39% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.0					Direct Entry, Copied from EWS

Summary for Subcatchment P-5: Subcat P-5

Runoff = 0.97 cfs @ 12.11 hrs, Volume= 5,072 cf, Depth= 0.80"
Routed to Pond TD1 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
67,151	39	>75% Grass cover, Good, HSG A
8,890	98	Paved parking, HSG A
76,040		Weighted Average
67,151	39	88.31% Pervious Area
8,890	98	11.69% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, Copied From EWS

Summary for Subcatchment P-6: Subcat P-6

Runoff = 0.31 cfs @ 12.08 hrs, Volume= 1,549 cf, Depth= 0.73"
Routed to Pond TD1 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
22,749	39	>75% Grass cover, Good, HSG A
2,605	98	Paved parking, HSG A
25,354		Weighted Average
22,749	39	89.73% Pervious Area
2,605	98	10.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Copied from EWS

Summary for Subcatchment P-7: Subcat P-7

Runoff = 0.42 cfs @ 12.15 hrs, Volume= 3,321 cf, Depth= 0.50"
Routed to Pond TD1 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
74,943	39	>75% Grass cover, Good, HSG A
4,287	98	Paved parking, HSG A
79,230		Weighted Average
74,943	39	94.59% Pervious Area
4,287	98	5.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.4					Direct Entry, Copied From EWS

Summary for Subcatchment PATIO: Subcat PATIO

Runoff = 0.33 cfs @ 12.08 hrs, Volume= 1,176 cf, Depth= 4.98"
Routed to Pond RD1 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
2,833	98	Paved parking, HSG A
2,833	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-1: Subcat R-1

Runoff = 0.59 cfs @ 12.08 hrs, Volume= 2,095 cf, Depth= 4.98"
Routed to Pond RD1 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
5,046	98	Roofs, HSG A
5,046	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-2: Subcat R-2

Runoff = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf, Depth= 4.98"
Routed to Pond RD2 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
18,290	98	Roofs, HSG A
18,290	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-3: Subcat R-3

Runoff = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf, Depth= 4.98"
Routed to Pond RD3 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
18,290	98	Roofs, HSG A
18,290	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-4: Subcat R-4

Runoff = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf, Depth= 4.98"
Routed to Pond RD4 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
18,290	98	Roofs, HSG A
18,290	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-5: Subcat R-5

Runoff = 2.41 cfs @ 12.08 hrs, Volume= 8,539 cf, Depth= 4.98"
Routed to Pond RD5 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 10 Year Rainfall=5.22"

Area (sf)	CN	Description
20,565	98	Roofs, HSG A
20,565	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Pond 1: Catch Basin

Inflow Area = 10,622 sf, 86.84% Impervious, Inflow Depth = 4.36" for 10 Year event
Inflow = 1.08 cfs @ 12.08 hrs, Volume= 3,859 cf
Outflow = 1.08 cfs @ 12.08 hrs, Volume= 3,859 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.08 cfs @ 12.08 hrs, Volume= 3,859 cf
Routed to Pond MH1 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 272.85' @ 12.52 hrs
Flood Elev= 274.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.20'	12.0" Round Culvert L= 84.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.20' / 271.70' S= 0.0060 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.08 cfs @ 12.08 hrs HW=272.82' TW=272.26' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.08 cfs @ 3.03 fps)

Summary for Pond 2: Catch Basin

Inflow Area = 12,976 sf, 8.11% Impervious, Inflow Depth = 0.63" for 10 Year event
Inflow = 0.12 cfs @ 12.08 hrs, Volume= 682 cf
Outflow = 0.13 cfs @ 12.09 hrs, Volume= 682 cf, Atten= 0%, Lag= 0.4 min
Primary = 0.13 cfs @ 12.09 hrs, Volume= 682 cf
Routed to Pond MH27 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.96' @ 12.10 hrs
Flood Elev= 274.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.61'	12.0" Round Culvert L= 47.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.61' / 271.34' S= 0.0057 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.13 cfs @ 12.09 hrs HW=273.95' TW=273.95' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.13 cfs @ 0.16 fps)

Summary for Pond 3: Catch Basin

Inflow Area = 9,660 sf, 44.27% Impervious, Inflow Depth = 3.79" for 10 Year event
Inflow = 0.86 cfs @ 12.08 hrs, Volume= 3,053 cf
Outflow = 0.86 cfs @ 12.08 hrs, Volume= 3,053 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.86 cfs @ 12.08 hrs, Volume= 3,053 cf
Routed to Pond MH27 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.02' @ 12.10 hrs
Flood Elev= 274.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.74'	12.0" Round Culvert L= 77.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.74' / 271.34' S= 0.0052 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.86 cfs @ 12.08 hrs HW=274.00' TW=273.93' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.86 cfs @ 1.09 fps)

Summary for Pond 4A: Catch Basin

Inflow Area = 3,976 sf, 91.93% Impervious, Inflow Depth = 4.60" for 10 Year event
Inflow = 0.43 cfs @ 12.08 hrs, Volume= 1,524 cf
Outflow = 0.43 cfs @ 12.08 hrs, Volume= 1,524 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.43 cfs @ 12.08 hrs, Volume= 1,524 cf
Routed to Pond MH26 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.04' @ 12.08 hrs
Flood Elev= 275.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.19'	12.0" Round Culvert L= 7.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.19' / 272.12' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.43 cfs @ 12.08 hrs HW=273.04' TW=273.03' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.43 cfs @ 0.81 fps)

Summary for Pond 4B: Catch Basin

Inflow Area = 10,218 sf, 73.42% Impervious, Inflow Depth = 3.72" for 10 Year event
Inflow = 0.88 cfs @ 12.08 hrs, Volume= 3,171 cf
Outflow = 0.88 cfs @ 12.08 hrs, Volume= 3,171 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.88 cfs @ 12.08 hrs, Volume= 3,171 cf
Routed to Pond MH26 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.15' @ 12.08 hrs
Flood Elev= 275.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.40'	12.0" Round Culvert L= 27.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.40' / 272.13' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.88 cfs @ 12.08 hrs HW=273.15' TW=273.03' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.88 cfs @ 1.93 fps)

Summary for Pond 4C: Catch Basin

Inflow Area = 10,741 sf, 84.88% Impervious, Inflow Depth = 4.27" for 10 Year event
Inflow = 1.07 cfs @ 12.08 hrs, Volume= 3,819 cf
Outflow = 1.07 cfs @ 12.08 hrs, Volume= 3,819 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.07 cfs @ 12.08 hrs, Volume= 3,819 cf
Routed to Pond MH30 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.91' @ 12.08 hrs
Flood Elev= 275.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.25'	12.0" Round Culvert L= 61.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.25' / 272.78' S= 0.0077 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.07 cfs @ 12.08 hrs HW=273.91' TW=273.55' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.07 cfs @ 2.74 fps)

Summary for Pond 4D: Catch Basin

Inflow Area = 10,203 sf, 79.98% Impervious, Inflow Depth = 4.03" for 10 Year event
Inflow = 0.96 cfs @ 12.08 hrs, Volume= 3,430 cf
Outflow = 0.96 cfs @ 12.08 hrs, Volume= 3,430 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.96 cfs @ 12.08 hrs, Volume= 3,430 cf
Routed to Pond MH30 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.80' @ 12.08 hrs
Flood Elev= 275.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.16'	12.0" Round Culvert L= 38.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.16' / 272.78' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.95 cfs @ 12.08 hrs HW=273.80' TW=273.55' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.95 cfs @ 2.54 fps)

Summary for Pond 5A: Catch Basin

Inflow Area = 10,459 sf, 93.11% Impervious, Inflow Depth = 4.66" for 10 Year event
Inflow = 1.14 cfs @ 12.08 hrs, Volume= 4,058 cf
Outflow = 1.14 cfs @ 12.08 hrs, Volume= 4,058 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.14 cfs @ 12.08 hrs, Volume= 4,058 cf
Routed to Pond MH28 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.12' @ 12.08 hrs
Flood Elev= 276.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.47'	12.0" Round Culvert L= 28.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.47' / 273.19' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.14 cfs @ 12.08 hrs HW=274.12' TW=273.81' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.14 cfs @ 2.98 fps)

Summary for Pond 5B: Catch Basin

Inflow Area = 3,865 sf, 95.02% Impervious, Inflow Depth = 4.75" for 10 Year event
Inflow = 0.43 cfs @ 12.08 hrs, Volume= 1,529 cf
Outflow = 0.43 cfs @ 12.08 hrs, Volume= 1,529 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.43 cfs @ 12.08 hrs, Volume= 1,529 cf
Routed to Pond MH28 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.05' @ 12.08 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.64'	12.0" Round Culvert L= 45.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.64' / 273.19' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.43 cfs @ 12.08 hrs HW=274.05' TW=273.81' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.43 cfs @ 2.11 fps)

Summary for Pond 5C: Catch Basin

Inflow Area = 2,619 sf, 91.40% Impervious, Inflow Depth = 4.58" for 10 Year event
Inflow = 0.28 cfs @ 12.08 hrs, Volume= 999 cf
Outflow = 0.28 cfs @ 12.08 hrs, Volume= 999 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.28 cfs @ 12.08 hrs, Volume= 999 cf
Routed to Pond MH31 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.19' @ 12.08 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.90'	12.0" Round Culvert L= 41.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.90' / 273.49' S= 0.0100 ' /' Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.28 cfs @ 12.08 hrs HW=274.19' TW=273.86' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.28 cfs @ 2.26 fps)

Summary for Pond 6: Catch Basin

Inflow Area = 12,631 sf, 77.41% Impervious, Inflow Depth = 3.91" for 10 Year event
Inflow = 1.15 cfs @ 12.08 hrs, Volume= 4,119 cf
Outflow = 1.15 cfs @ 12.08 hrs, Volume= 4,119 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.15 cfs @ 12.08 hrs, Volume= 4,119 cf
Routed to Pond MH7 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.76' @ 12.53 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.76'	12.0" Round Culvert L= 26.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.76' / 272.50' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.15 cfs @ 12.08 hrs HW=273.41' TW=273.09' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.15 cfs @ 3.03 fps)

Summary for Pond 7A: Catch Basin

Inflow Area = 7,101 sf, 79.91% Impervious, Inflow Depth = 4.03" for 10 Year event
Inflow = 0.67 cfs @ 12.08 hrs, Volume= 2,386 cf
Outflow = 0.67 cfs @ 12.08 hrs, Volume= 2,386 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.67 cfs @ 12.08 hrs, Volume= 2,386 cf
Routed to Pond MH8 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.76' @ 12.54 hrs
Flood Elev= 276.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.92'	12.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.92' / 272.82' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.66 cfs @ 12.08 hrs HW=273.56' TW=273.48' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.66 cfs @ 1.77 fps)

Summary for Pond 7B: Catch Basin

Inflow Area = 11,340 sf, 75.16% Impervious, Inflow Depth = 3.81" for 10 Year event
Inflow = 1.00 cfs @ 12.08 hrs, Volume= 3,597 cf
Outflow = 1.00 cfs @ 12.08 hrs, Volume= 3,597 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.00 cfs @ 12.08 hrs, Volume= 3,597 cf
Routed to Pond MH8 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.82' @ 12.08 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.22'	12.0" Round Culvert L= 40.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.22' / 272.82' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.00 cfs @ 12.08 hrs HW=273.82' TW=273.48' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.00 cfs @ 2.89 fps)

Summary for Pond 8: Catch Basin

Inflow Area = 10,991 sf, 81.26% Impervious, Inflow Depth = 4.10" for 10 Year event
Inflow = 1.05 cfs @ 12.08 hrs, Volume= 3,751 cf
Outflow = 1.05 cfs @ 12.08 hrs, Volume= 3,751 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.05 cfs @ 12.08 hrs, Volume= 3,751 cf
Routed to Pond MH33 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.70' @ 12.43 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.47'	12.0" Round Culvert L= 27.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.47' / 273.20' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.95 cfs @ 12.08 hrs HW=274.28' TW=274.16' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.95 cfs @ 1.90 fps)

Summary for Pond 9A: Catch Basin

Inflow Area = 11,743 sf, 81.25% Impervious, Inflow Depth = 4.09" for 10 Year event
Inflow = 1.12 cfs @ 12.08 hrs, Volume= 4,007 cf
Outflow = 1.12 cfs @ 12.08 hrs, Volume= 4,007 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.12 cfs @ 12.08 hrs, Volume= 4,007 cf
Routed to Pond MH32 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.70' @ 12.43 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.58'	12.0" Round Culvert L= 14.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.58' / 273.51' S= 0.0050 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.12 cfs @ 12.08 hrs HW=274.39' TW=274.25' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.12 cfs @ 2.24 fps)

Summary for Pond 9B: Catch Basin

Inflow Area = 14,736 sf, 67.40% Impervious, Inflow Depth = 3.44" for 10 Year event
Inflow = 1.16 cfs @ 12.08 hrs, Volume= 4,223 cf
Outflow = 1.16 cfs @ 12.08 hrs, Volume= 4,223 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.16 cfs @ 12.08 hrs, Volume= 4,223 cf
Routed to Pond MH33 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.70' @ 12.43 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.58'	12.0" Round Culvert L= 64.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.58' / 273.20' S= 0.0059 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.10 cfs @ 12.08 hrs HW=274.39' TW=274.16' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.10 cfs @ 2.22 fps)

Summary for Pond 9C: Catch Basin

Inflow Area = 3,708 sf, 100.00% Impervious, Inflow Depth = 4.98" for 10 Year event
Inflow = 0.43 cfs @ 12.08 hrs, Volume= 1,540 cf
Outflow = 0.43 cfs @ 12.08 hrs, Volume= 1,540 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.43 cfs @ 12.08 hrs, Volume= 1,540 cf
Routed to Pond MH12 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.69' @ 12.44 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.24'	12.0" Round Culvert L= 7.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.24' / 273.16' S= 0.0114 '/ Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=274.04' TW=274.05' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)

Summary for Pond 9D: Catch Basin

Inflow Area = 1,543 sf, 58.43% Impervious, Inflow Depth = 3.01" for 10 Year event
Inflow = 0.11 cfs @ 12.08 hrs, Volume= 387 cf
Outflow = 0.11 cfs @ 12.08 hrs, Volume= 387 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.11 cfs @ 12.08 hrs, Volume= 387 cf
Routed to Pond MH32 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.69' @ 12.43 hrs
Flood Elev= 276.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.95'	12.0" Round Culvert L= 49.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.95' / 273.51' S= 0.0090 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.11 cfs @ 12.08 hrs HW=274.28' TW=274.25' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.11 cfs @ 0.70 fps)

Summary for Pond 10A: Catch Basin

Inflow Area = 2,889 sf, 81.34% Impervious, Inflow Depth = 4.10" for 10 Year event
Inflow = 0.28 cfs @ 12.08 hrs, Volume= 987 cf
Outflow = 0.28 cfs @ 12.08 hrs, Volume= 987 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.28 cfs @ 12.08 hrs, Volume= 987 cf
Routed to Pond MH15 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.97' @ 12.60 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.41'	12.0" Round Culvert L= 87.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.41' / 270.54' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.27 cfs @ 12.08 hrs HW=271.72' TW=271.31' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.27 cfs @ 1.96 fps)

Summary for Pond 10B: Catch Basin

Inflow Area = 10,414 sf, 86.69% Impervious, Inflow Depth = 4.35" for 10 Year event
Inflow = 1.06 cfs @ 12.08 hrs, Volume= 3,777 cf
Outflow = 1.06 cfs @ 12.08 hrs, Volume= 3,777 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.06 cfs @ 12.08 hrs, Volume= 3,777 cf
Routed to Pond MH15 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.97' @ 12.60 hrs
Flood Elev= 276.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.69'	12.0" Round Culvert L= 15.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.69' / 270.54' S= 0.0100 ' / ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.96 cfs @ 12.08 hrs HW=271.44' TW=271.31' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.96 cfs @ 2.10 fps)

Summary for Pond 11: Catch Basin

Inflow Area = 11,505 sf, 85.23% Impervious, Inflow Depth = 4.28" for 10 Year event
Inflow = 1.15 cfs @ 12.08 hrs, Volume= 4,107 cf
Outflow = 1.15 cfs @ 12.08 hrs, Volume= 4,107 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.15 cfs @ 12.08 hrs, Volume= 4,107 cf
Routed to Pond MH17 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.97' @ 12.59 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.55'	12.0" Round Culvert L= 15.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.55' / 270.40' S= 0.0100 ' / ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.15 cfs @ 12.08 hrs HW=271.31' TW=271.13' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.15 cfs @ 2.48 fps)

Summary for Pond 12: Catch Basin

Inflow Area = 7,149 sf, 71.54% Impervious, Inflow Depth = 3.63" for 10 Year event
Inflow = 0.60 cfs @ 12.08 hrs, Volume= 2,165 cf
Outflow = 0.60 cfs @ 12.08 hrs, Volume= 2,165 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.60 cfs @ 12.08 hrs, Volume= 2,165 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.98' @ 12.57 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.74'	12.0" Round Culvert L= 14.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.74' / 270.60' S= 0.0100 ' / ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.58 cfs @ 12.08 hrs HW=271.97' TW=271.95' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.58 cfs @ 0.74 fps)

Summary for Pond 13: Catch Basin

Inflow Area = 4,141 sf, 88.54% Impervious, Inflow Depth = 4.44" for 10 Year event
Inflow = 0.43 cfs @ 12.08 hrs, Volume= 1,532 cf
Outflow = 0.43 cfs @ 12.08 hrs, Volume= 1,532 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.43 cfs @ 12.08 hrs, Volume= 1,532 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.23' @ 12.08 hrs
Flood Elev= 275.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.79'	12.0" Round Culvert L= 87.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.79' / 270.92' S= 0.0100 ' / ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.43 cfs @ 12.08 hrs HW=272.23' TW=271.95' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.43 cfs @ 1.90 fps)

Summary for Pond 14: Catch Basin

Inflow Area = 6,167 sf, 87.00% Impervious, Inflow Depth = 4.37" for 10 Year event
Inflow = 0.63 cfs @ 12.08 hrs, Volume= 2,244 cf
Outflow = 0.63 cfs @ 12.08 hrs, Volume= 2,244 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.63 cfs @ 12.08 hrs, Volume= 2,244 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.01' @ 12.08 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.25'	12.0" Round Culvert L= 33.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.25' / 270.92' S= 0.0100 ' / ' Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.62 cfs @ 12.08 hrs HW=272.01' TW=271.95' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.62 cfs @ 1.33 fps)

Summary for Pond 15: Catch Basin

Inflow Area = 20,154 sf, 44.65% Impervious, Inflow Depth = 2.36" for 10 Year event
Inflow = 1.06 cfs @ 12.08 hrs, Volume= 3,966 cf
Outflow = 1.06 cfs @ 12.08 hrs, Volume= 3,966 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.06 cfs @ 12.08 hrs, Volume= 3,966 cf
Routed to Pond MH23 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.16' @ 12.08 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.42'	12.0" Round Culvert L= 19.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.42' / 272.23' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.05 cfs @ 12.08 hrs HW=273.16' TW=272.99' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.05 cfs @ 2.36 fps)

Summary for Pond 16: Catch Basin

Inflow Area = 20,584 sf, 35.58% Impervious, Inflow Depth = 1.93" for 10 Year event
Inflow = 0.86 cfs @ 12.08 hrs, Volume= 3,314 cf
Outflow = 0.86 cfs @ 12.08 hrs, Volume= 3,314 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.86 cfs @ 12.08 hrs, Volume= 3,314 cf
Routed to Pond MH24 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.87' @ 12.08 hrs
Flood Elev= 277.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	274.29'	12.0" Round Culvert L= 19.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 274.29' / 274.10' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.86 cfs @ 12.08 hrs HW=274.87' TW=274.64' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.86 cfs @ 2.63 fps)

Summary for Pond 17: Catch Basin

Inflow Area = 36,035 sf, 27.88% Impervious, Inflow Depth = 1.57" for 10 Year event
Inflow = 1.18 cfs @ 12.08 hrs, Volume= 4,706 cf
Outflow = 1.18 cfs @ 12.08 hrs, Volume= 4,706 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.18 cfs @ 12.08 hrs, Volume= 4,706 cf
Routed to Pond MH24 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.54' @ 12.08 hrs
Flood Elev= 277.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	274.97'	15.0" Round Culvert L= 143.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 274.97' / 274.10' S= 0.0061 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.18 cfs @ 12.08 hrs HW=275.54' TW=274.64' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.18 cfs @ 3.20 fps)

Summary for Pond LP-1: Low Point - MAHW

Inflow Area = 210,774 sf, 10.22% Impervious, Inflow Depth = 0.21" for 10 Year event
Inflow = 2.26 cfs @ 12.12 hrs, Volume= 3,742 cf
Outflow = 2.24 cfs @ 12.13 hrs, Volume= 3,647 cf, Atten= 1%, Lag= 0.8 min
Primary = 2.24 cfs @ 12.13 hrs, Volume= 3,647 cf
Routed to Link SP1 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 268.26' @ 12.13 hrs Surf.Area= 279 sf Storage= 285 cf

Plug-Flow detention time= 19.7 min calculated for 3,647 cf (97% of inflow)
Center-of-Mass det. time= 9.1 min (760.9 - 751.8)

Volume	Invert	Avail.Storage	Storage Description		
#1	267.00'	902 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
267.00	175	160.0	0	0	175
268.00	258	168.0	215	215	441
269.00	344	172.0	300	515	642
270.00	431	176.0	387	902	848

Device	Routing	Invert	Outlet Devices
#1	Primary	267.93'	24.0" Round RCP_Round 24" L= 59.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 267.72' / 267.93' S= -0.0036 '/' Cc= 0.900 n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf

#2 Primary 267.48' **24.0" Round RCP_Round 24"**
L= 59.0' RCP, square edge headwall, Ke= 0.500
Inlet / Outlet Invert= 267.48' / 267.48' S= 0.0000 '/ S= 0.0000 ' Cc= 0.900
n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf

Primary OutFlow Max=2.24 cfs @ 12.13 hrs HW=268.26' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Inlet Controls 0.66 cfs @ 1.95 fps)
2=RCP_Round 24" (Barrel Controls 1.58 cfs @ 2.06 fps)

Summary for Pond LP-2: Low Point - MAHW

Inflow Area = 236,963 sf, 10.14% Impervious, Inflow Depth = 0.25" for 10 Year event
Inflow = 2.50 cfs @ 12.12 hrs, Volume= 4,930 cf
Outflow = 2.08 cfs @ 12.19 hrs, Volume= 4,931 cf, Atten= 17%, Lag= 3.7 min
Primary = 2.08 cfs @ 12.19 hrs, Volume= 4,931 cf
Routed to Link SP2 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 267.36' @ 12.19 hrs Surf.Area= 1,410 sf Storage= 545 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time= 4.5 min (774.8 - 770.3)

Volume	Invert	Avail.Storage	Storage Description			
#1	266.85'	5,666 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
266.85	0	0.0	0	0	0	
267.00	1,276	387.0	64	64	11,918	
268.00	1,668	396.0	1,468	1,531	12,602	
269.00	2,069	404.0	1,865	3,396	13,250	
270.00	2,477	412.0	2,270	5,666	13,910	

Device	Routing	Invert	Outlet Devices
#1	Primary	266.85'	24.0" Round RCP_Round 24" L= 30.0' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 266.37' / 266.85' S= -0.0160 '/ S= -0.0160 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf

Primary OutFlow Max=2.08 cfs @ 12.19 hrs HW=267.36' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Inlet Controls 2.08 cfs @ 3.31 fps)

Summary for Pond MH1: Manhole

Inflow Area = 10,622 sf, 86.84% Impervious, Inflow Depth = 4.36" for 10 Year event
Inflow = 1.08 cfs @ 12.08 hrs, Volume= 3,859 cf
Outflow = 1.08 cfs @ 12.08 hrs, Volume= 3,859 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.08 cfs @ 12.08 hrs, Volume= 3,859 cf
Routed to Pond UIS1 : Underground Chamber #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.83' @ 12.56 hrs
Flood Elev= 275.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.70'	24.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.70' / 271.70' S= 0.0000 '/ S= 0.0000 ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.09 cfs @ 12.08 hrs HW=272.26' TW=272.10' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.09 cfs @ 2.26 fps)

Summary for Pond MH11: Manhole

Inflow Area = 25,727 sf, 73.32% Impervious, Inflow Depth = 3.72" for 10 Year event
Inflow = 2.21 cfs @ 12.08 hrs, Volume= 7,974 cf
Outflow = 2.21 cfs @ 12.08 hrs, Volume= 7,974 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.21 cfs @ 12.08 hrs, Volume= 7,974 cf
Routed to Pond UIS4 : Underground Chamber #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.68' @ 12.43 hrs
Flood Elev= 276.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.00'	24.0" Round Culvert L= 4.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.00' / 273.00' S= 0.0000 '/ S= 0.0000 ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=2.21 cfs @ 12.08 hrs HW=273.90' TW=273.76' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.21 cfs @ 2.37 fps)

Summary for Pond MH12: Manhole

Inflow Area = 16,994 sf, 83.27% Impervious, Inflow Depth = 4.19" for 10 Year event
Inflow = 1.66 cfs @ 12.08 hrs, Volume= 5,934 cf
Outflow = 1.66 cfs @ 12.08 hrs, Volume= 5,934 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.66 cfs @ 12.08 hrs, Volume= 5,934 cf
Routed to Pond MH13 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.69' @ 12.43 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.16'	15.0" Round Culvert L= 28.0' CPP, square edge headwall, Ke= 0.500

Inlet / Outlet Invert= 273.16' / 273.00' S= 0.0057 '/' Cc= 0.900
n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.66 cfs @ 12.08 hrs HW=274.05' TW=273.86' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.66 cfs @ 2.48 fps)

Summary for Pond MH13: Manhole

Inflow Area = 16,994 sf, 83.27% Impervious, Inflow Depth = 4.19" for 10 Year event
Inflow = 1.66 cfs @ 12.08 hrs, Volume= 5,934 cf
Outflow = 1.66 cfs @ 12.08 hrs, Volume= 5,934 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.66 cfs @ 12.08 hrs, Volume= 5,934 cf
Routed to Pond UIS4 : Underground Chamber #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.68' @ 12.43 hrs
Flood Elev= 277.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.00'	24.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.00' / 273.00' S= 0.0000 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.66 cfs @ 12.08 hrs HW=273.86' TW=273.76' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.66 cfs @ 1.91 fps)

Summary for Pond MH15: Manhole

Inflow Area = 13,302 sf, 85.53% Impervious, Inflow Depth = 4.30" for 10 Year event
Inflow = 1.33 cfs @ 12.08 hrs, Volume= 4,764 cf
Outflow = 1.33 cfs @ 12.08 hrs, Volume= 4,764 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.33 cfs @ 12.08 hrs, Volume= 4,764 cf
Routed to Pond MH16 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.97' @ 12.59 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.44'	12.0" Round Culvert L= 14.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.44' / 270.30' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.33 cfs @ 12.08 hrs HW=271.31' TW=271.15' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.33 cfs @ 2.44 fps)

Summary for Pond MH16: Manhole

Inflow Area = 13,302 sf, 85.53% Impervious, Inflow Depth = 4.30" for 10 Year event
Inflow = 1.33 cfs @ 12.08 hrs, Volume= 4,764 cf
Outflow = 1.33 cfs @ 12.08 hrs, Volume= 4,764 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.33 cfs @ 12.08 hrs, Volume= 4,764 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.97' @ 12.59 hrs
Flood Elev= 276.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.30'	24.0" Round Culvert L= 5.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.30' / 270.30' S= 0.0000 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.33 cfs @ 12.08 hrs HW=271.15' TW=271.09' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.33 cfs @ 1.56 fps)

Summary for Pond MH17: Manhole

Inflow Area = 11,505 sf, 85.23% Impervious, Inflow Depth = 4.28" for 10 Year event
Inflow = 1.15 cfs @ 12.08 hrs, Volume= 4,107 cf
Outflow = 1.15 cfs @ 12.08 hrs, Volume= 4,107 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.15 cfs @ 12.08 hrs, Volume= 4,107 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.97' @ 12.59 hrs
Flood Elev= 277.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.30'	24.0" Round Culvert L= 7.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.30' / 270.30' S= 0.0000 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.15 cfs @ 12.08 hrs HW=271.13' TW=271.09' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.15 cfs @ 1.37 fps)

Summary for Pond MH19: Manhole

Inflow Area = 157,891 sf, 63.68% Impervious, Inflow Depth = 0.00" for 10 Year event
Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 269.87' @ 0.00 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	269.87'	18.0" Round Culvert L= 51.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 269.87' / 269.52' S= 0.0069 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=269.87' TW=0.00' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)

Summary for Pond MH21: Manhole

Inflow Area = 94,229 sf, 43.00% Impervious, Inflow Depth = 2.28" for 10 Year event
Inflow = 4.75 cfs @ 12.08 hrs, Volume= 17,927 cf
Outflow = 4.75 cfs @ 12.08 hrs, Volume= 17,927 cf, Atten= 0%, Lag= 0.0 min
Primary = 4.75 cfs @ 12.08 hrs, Volume= 17,927 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.97' @ 12.58 hrs
Flood Elev= 277.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.50'	24.0" Round Culvert L= 20.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.50' / 270.30' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=4.78 cfs @ 12.08 hrs HW=271.56' TW=271.09' (Dynamic Tailwater)
1=Culvert (Barrel Controls 4.78 cfs @ 4.12 fps)

Summary for Pond MH22: Manhole

Inflow Area = 94,229 sf, 43.00% Impervious, Inflow Depth = 2.28" for 10 Year event
Inflow = 4.75 cfs @ 12.08 hrs, Volume= 17,927 cf
Outflow = 4.75 cfs @ 12.08 hrs, Volume= 17,927 cf, Atten= 0%, Lag= 0.0 min
Primary = 4.75 cfs @ 12.08 hrs, Volume= 17,927 cf
Routed to Pond MH21 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.98' @ 12.56 hrs
Flood Elev= 277.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.82'	24.0" Round Culvert L= 22.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.82' / 270.60' S= 0.0100 '/ Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=4.74 cfs @ 12.08 hrs HW=271.95' TW=271.56' (Dynamic Tailwater)
1=Culvert (Outlet Controls 4.74 cfs @ 3.76 fps)

Summary for Pond MH23: Manhole

Inflow Area = 76,773 sf, 34.35% Impervious, Inflow Depth = 1.87" for 10 Year event
Inflow = 3.09 cfs @ 12.08 hrs, Volume= 11,986 cf
Outflow = 3.09 cfs @ 12.08 hrs, Volume= 11,986 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.09 cfs @ 12.08 hrs, Volume= 11,986 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.99' @ 12.08 hrs
Flood Elev= 277.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.13'	18.0" Round Culvert L= 121.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.13' / 270.92' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=3.08 cfs @ 12.08 hrs HW=272.99' TW=271.95' (Dynamic Tailwater)
1=Culvert (Outlet Controls 3.08 cfs @ 4.27 fps)

Summary for Pond MH24: Manhole

Inflow Area = 56,619 sf, 30.68% Impervious, Inflow Depth = 1.70" for 10 Year event
Inflow = 2.04 cfs @ 12.08 hrs, Volume= 8,019 cf
Outflow = 2.04 cfs @ 12.08 hrs, Volume= 8,019 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.04 cfs @ 12.08 hrs, Volume= 8,019 cf
Routed to Pond MH23 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.64' @ 12.08 hrs
Flood Elev= 278.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.99'	18.0" Round Culvert L= 176.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.99' / 272.23' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=2.03 cfs @ 12.08 hrs HW=274.64' TW=272.99' (Dynamic Tailwater)
1=Culvert (Inlet Controls 2.03 cfs @ 2.75 fps)

Summary for Pond MH26: Manhole

Inflow Area = 35,138 sf, 80.92% Impervious, Inflow Depth = 4.08" for 10 Year event
Inflow = 3.33 cfs @ 12.08 hrs, Volume= 11,944 cf
Outflow = 3.33 cfs @ 12.08 hrs, Volume= 11,944 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.33 cfs @ 12.08 hrs, Volume= 11,944 cf
Routed to Pond UIS1 : Underground Chamber #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.03' @ 12.08 hrs
Flood Elev= 275.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.13'	18.0" Round Culvert L= 43.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.13' / 271.70' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=3.33 cfs @ 12.08 hrs HW=273.03' TW=272.10' (Dynamic Tailwater)
1=Culvert (Barrel Controls 3.33 cfs @ 4.34 fps)

Summary for Pond MH27: Manhole

Inflow Area = 22,636 sf, 23.54% Impervious, Inflow Depth = 1.98" for 10 Year event
Inflow = 0.99 cfs @ 12.09 hrs, Volume= 3,735 cf
Outflow = 0.99 cfs @ 12.09 hrs, Volume= 3,735 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.99 cfs @ 12.09 hrs, Volume= 3,735 cf
Routed to Pond UIS2 : Underground Chamber #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.96' @ 12.10 hrs
Flood Elev= 275.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.34'	15.0" Round Culvert L= 8.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.34' / 272.30' S= 0.0050 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=0.98 cfs @ 12.09 hrs HW=273.95' TW=273.92' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.98 cfs @ 0.80 fps)

Summary for Pond MH28: Manhole

Inflow Area = 16,944 sf, 93.28% Impervious, Inflow Depth = 4.66" for 10 Year event
Inflow = 1.85 cfs @ 12.08 hrs, Volume= 6,586 cf
Outflow = 1.85 cfs @ 12.08 hrs, Volume= 6,586 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.85 cfs @ 12.08 hrs, Volume= 6,586 cf
Routed to Pond MH6 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.81' @ 12.08 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.09'	15.0" Round Culvert L= 59.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.09' / 272.50' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.85 cfs @ 12.08 hrs HW=273.81' TW=273.22' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.85 cfs @ 3.65 fps)

Summary for Pond MH29: Manhole

Inflow Area = 20,944 sf, 82.49% Impervious, Inflow Depth = 4.15" for 10 Year event
Inflow = 2.03 cfs @ 12.08 hrs, Volume= 7,249 cf
Outflow = 2.03 cfs @ 12.08 hrs, Volume= 7,249 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.03 cfs @ 12.08 hrs, Volume= 7,249 cf
Routed to Pond MH26 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.22' @ 12.08 hrs
Flood Elev= 275.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.24'	15.0" Round Culvert L= 11.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.24' / 272.13' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.02 cfs @ 12.08 hrs HW=273.22' TW=273.03' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.02 cfs @ 2.70 fps)

Summary for Pond MH30: Manhole

Inflow Area = 20,944 sf, 82.49% Impervious, Inflow Depth = 4.15" for 10 Year event
Inflow = 2.03 cfs @ 12.08 hrs, Volume= 7,249 cf
Outflow = 2.03 cfs @ 12.08 hrs, Volume= 7,249 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.03 cfs @ 12.08 hrs, Volume= 7,249 cf
Routed to Pond MH29 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.55' @ 12.08 hrs
Flood Elev= 275.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.68'	15.0" Round Culvert L= 34.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.68' / 272.34' S= 0.0100 '/ Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.02 cfs @ 12.08 hrs HW=273.55' TW=273.22' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.02 cfs @ 3.14 fps)

Summary for Pond MH31: Manhole

Inflow Area = 2,619 sf, 91.40% Impervious, Inflow Depth = 4.58" for 10 Year event
Inflow = 0.28 cfs @ 12.08 hrs, Volume= 999 cf
Outflow = 0.28 cfs @ 12.08 hrs, Volume= 999 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.28 cfs @ 12.08 hrs, Volume= 999 cf
Routed to Pond MH28 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.86' @ 12.08 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.39'	12.0" Round Culvert L= 20.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.39' / 273.19' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.28 cfs @ 12.08 hrs HW=273.86' TW=273.81' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.28 cfs @ 1.15 fps)

Summary for Pond MH32: Manhole

Inflow Area = 13,286 sf, 78.60% Impervious, Inflow Depth = 3.97" for 10 Year event
Inflow = 1.22 cfs @ 12.08 hrs, Volume= 4,394 cf
Outflow = 1.22 cfs @ 12.08 hrs, Volume= 4,394 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.22 cfs @ 12.08 hrs, Volume= 4,394 cf
Routed to Pond MH12 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.69' @ 12.43 hrs
Flood Elev= 276.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.41'	12.0" Round Culvert L= 48.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.41' / 273.16' S= 0.0052 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.14 cfs @ 12.08 hrs HW=274.25' TW=274.05' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.14 cfs @ 2.20 fps)

Summary for Pond MH33: Manhole

Inflow Area = 25,727 sf, 73.32% Impervious, Inflow Depth = 3.72" for 10 Year event
Inflow = 2.21 cfs @ 12.08 hrs, Volume= 7,974 cf
Outflow = 2.21 cfs @ 12.08 hrs, Volume= 7,974 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.21 cfs @ 12.08 hrs, Volume= 7,974 cf
Routed to Pond MH11 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.69' @ 12.42 hrs
Flood Elev= 277.05'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.20'	15.0" Round Culvert L= 20.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.20' / 273.00' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.21 cfs @ 12.08 hrs HW=274.16' TW=273.90' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.21 cfs @ 3.01 fps)

Summary for Pond MH6: Manhole

Inflow Area = 16,944 sf, 93.28% Impervious, Inflow Depth = 4.66" for 10 Year event
Inflow = 1.85 cfs @ 12.08 hrs, Volume= 6,586 cf
Outflow = 1.85 cfs @ 12.08 hrs, Volume= 6,586 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.85 cfs @ 12.08 hrs, Volume= 6,586 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.76' @ 12.54 hrs
Flood Elev= 277.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.50'	24.0" Round Culvert L= 6.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.50' / 272.50' S= 0.0000 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.85 cfs @ 12.08 hrs HW=273.22' TW=272.97' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.85 cfs @ 2.68 fps)

Summary for Pond MH7: Manhole

Inflow Area = 12,631 sf, 77.41% Impervious, Inflow Depth = 3.91" for 10 Year event
Inflow = 1.15 cfs @ 12.08 hrs, Volume= 4,119 cf
Outflow = 1.15 cfs @ 12.08 hrs, Volume= 4,119 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.15 cfs @ 12.08 hrs, Volume= 4,119 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.75' @ 12.54 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.50'	24.0" Round Culvert L= 6.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.50' / 272.50' S= 0.0000 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.15 cfs @ 12.08 hrs HW=273.09' TW=272.97' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.15 cfs @ 2.21 fps)

Summary for Pond MH8: Manhole

Inflow Area = 18,441 sf, 76.99% Impervious, Inflow Depth = 3.89" for 10 Year event
Inflow = 1.67 cfs @ 12.08 hrs, Volume= 5,982 cf
Outflow = 1.67 cfs @ 12.08 hrs, Volume= 5,982 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.67 cfs @ 12.08 hrs, Volume= 5,982 cf
Routed to Pond MH9 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.76' @ 12.53 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.72'	15.0" Round Culvert L= 22.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.72' / 272.50' S= 0.0100 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.66 cfs @ 12.08 hrs HW=273.48' TW=273.19' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.66 cfs @ 3.06 fps)

Summary for Pond MH9: Manhole

Inflow Area = 18,441 sf, 76.99% Impervious, Inflow Depth = 3.89" for 10 Year event
Inflow = 1.67 cfs @ 12.08 hrs, Volume= 5,982 cf
Outflow = 1.67 cfs @ 12.08 hrs, Volume= 5,982 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.67 cfs @ 12.08 hrs, Volume= 5,982 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.75' @ 12.54 hrs
Flood Elev= 277.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.50'	24.0" Round Culvert L= 8.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.50' / 272.50' S= 0.0000 '/ S Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.66 cfs @ 12.08 hrs HW=273.19' TW=272.97' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.66 cfs @ 2.58 fps)

Summary for Pond RD1: Roof Drain

Inflow Area = 7,878 sf, 100.00% Impervious, Inflow Depth = 4.98" for 10 Year event
Inflow = 0.92 cfs @ 12.08 hrs, Volume= 3,271 cf
Outflow = 0.92 cfs @ 12.08 hrs, Volume= 3,271 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.92 cfs @ 12.08 hrs, Volume= 3,271 cf
Routed to Pond UIS1 : Underground Chamber #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.83' @ 12.55 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.91'	12.0" Round Culvert L= 21.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.91' / 271.70' S= 0.0100 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.93 cfs @ 12.08 hrs HW=272.44' TW=272.10' (Dynamic Tailwater)
1=Culvert (Barrel Controls 0.93 cfs @ 3.18 fps)

Summary for Pond RD2: Roof Drain

Inflow Area = 18,290 sf, 100.00% Impervious, Inflow Depth = 4.98" for 10 Year event
Inflow = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf
Outflow = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.77' @ 12.51 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.81'	12.0" Round Culvert L= 31.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.81' / 272.50' S= 0.0100 '/ S Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.14 cfs @ 12.08 hrs HW=273.68' TW=272.97' (Dynamic Tailwater)
1=Culvert (Barrel Controls 2.14 cfs @ 3.94 fps)

Summary for Pond RD3: Roof Drain

Inflow Area = 18,290 sf, 100.00% Impervious, Inflow Depth = 4.98" for 10 Year event
Inflow = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf
Outflow = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.76' @ 12.52 hrs
Flood Elev= 278.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.65'	12.0" Round Culvert L= 14.5' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.65' / 272.50' S= 0.0103 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.14 cfs @ 12.08 hrs HW=273.58' TW=272.97' (Dynamic Tailwater)
1=Culvert (Barrel Controls 2.14 cfs @ 3.68 fps)

Summary for Pond RD4: Roof Drain

Inflow Area = 18,290 sf, 100.00% Impervious, Inflow Depth = 4.98" for 10 Year event
Inflow = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf
Outflow = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.15 cfs @ 12.08 hrs, Volume= 7,595 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.98' @ 12.58 hrs
Flood Elev= 277.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.55'	12.0" Round Culvert L= 24.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.55' / 270.30' S= 0.0104 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.14 cfs @ 12.08 hrs HW=271.49' TW=271.09' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.14 cfs @ 3.61 fps)

Summary for Pond RD5: Roof Drain

Inflow Area = 20,565 sf, 100.00% Impervious, Inflow Depth = 4.98" for 10 Year event
Inflow = 2.41 cfs @ 12.08 hrs, Volume= 8,539 cf
Outflow = 2.41 cfs @ 12.08 hrs, Volume= 8,539 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.41 cfs @ 12.08 hrs, Volume= 8,539 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.97' @ 12.59 hrs
Flood Elev= 278.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.48'	15.0" Round Culvert L= 18.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.48' / 270.30' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.41 cfs @ 12.08 hrs HW=271.41' TW=271.09' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.41 cfs @ 3.40 fps)

Summary for Pond TD1: Stone Trench Drain

Inflow Area = 180,624 sf, 8.74% Impervious, Inflow Depth = 0.66" for 10 Year event
Inflow = 1.66 cfs @ 12.11 hrs, Volume= 9,942 cf
Outflow = 1.60 cfs @ 12.14 hrs, Volume= 9,942 cf, Atten= 4%, Lag= 1.6 min
Discarded = 0.20 cfs @ 11.51 hrs, Volume= 7,234 cf
Primary = 1.40 cfs @ 12.14 hrs, Volume= 2,709 cf
Routed to Pond LP-1 : Low Point - MAHW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.12' @ 12.14 hrs Surf.Area= 1,050 sf Storage= 192 cf
Flood Elev= 272.90' Surf.Area= 1,050 sf Storage= 475 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time= 0.9 min (835.6 - 834.7)

Volume	Invert	Avail.Storage	Storage Description
#1	271.20'	4 cf	3.50'W x 300.00'L x 1.70'H Prismatic 1,785 cf Overall - 471 cf Embedded = 1,314 cf x 0.3% Voids
#2	271.70'	471 cf	12.0" Round Pipe Storage x 2 Inside #1 L= 300.0'
#3	272.90'	3,296 cf	Custom Stage Data (Irregular) Listed below (Recalc)
		3,771 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
272.90	0	0.0	0	0	0
273.00	2,670	657.2	89	89	34,370
274.00	3,775	679.8	3,207	3,296	36,866

Device	Routing	Invert	Outlet Devices
#1	Primary	271.70'	12.0" Round Culvert X 2.00 L= 60.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.70' / 270.90' S= 0.0133 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Discarded	271.20'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.20 cfs @ 11.51 hrs HW=271.23' (Free Discharge)
2=Exfiltration (Exfiltration Controls 0.20 cfs)

Primary OutFlow Max=1.40 cfs @ 12.14 hrs HW=272.12' TW=268.26' (Dynamic Tailwater)
1=Culvert (Inlet Controls 1.40 cfs @ 2.21 fps)

Summary for Pond TD2: Stone Trench Drain

Inflow Area = 11,308 sf, 0.00% Impervious, Inflow Depth = 1.25" for 10 Year event
Inflow = 0.29 cfs @ 12.08 hrs, Volume= 1,174 cf
Outflow = 0.08 cfs @ 11.86 hrs, Volume= 1,174 cf, Atten= 72%, Lag= 0.0 min
Discarded = 0.08 cfs @ 11.86 hrs, Volume= 1,174 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Pond LP-2 : Low Point - MAHW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.78' @ 12.46 hrs Surf.Area= 427 sf Storage= 173 cf
Flood Elev= 275.00' Surf.Area= 1,466 sf Storage= 536 cf

Plug-Flow detention time= 8.7 min calculated for 1,173 cf (100% of inflow)
Center-of-Mass det. time= 8.7 min (806.2 - 797.6)

Volume	Invert	Avail.Storage	Storage Description
#1	272.00'	167 cf	3.50'W x 122.00'L x 1.75'H Prismatoid 747 cf Overall - 192 cf Embedded = 556 cf x 30.0% Voids
#2	272.25'	192 cf	12.0" Round Pipe Storage x 2 Inside #1 L= 122.0'
#3	273.75'	178 cf	Custom Stage Data (Irregular) Listed below (Recalc)
			536 cf Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
273.75	430	253.0	0	0	430
274.00	1,039	301.0	178	178	2,547

Device	Routing	Invert	Outlet Devices
#1	Primary	273.03'	12.0" Round Culvert L= 60.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.03' / 272.50' S= 0.0088 ' / Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Discarded	272.00'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.08 cfs @ 11.86 hrs HW=272.04' (Free Discharge)
2=Exfiltration (Exfiltration Controls 0.08 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=272.00' TW=266.85' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)

Summary for Pond UIS1: Underground Chamber #1

Inflow Area = 53,638 sf, 84.89% Impervious, Inflow Depth = 4.27" for 10 Year event
Inflow = 5.34 cfs @ 12.08 hrs, Volume= 19,074 cf
Outflow = 0.84 cfs @ 11.71 hrs, Volume= 19,074 cf, Atten= 84%, Lag= 0.0 min
Discarded = 0.84 cfs @ 11.71 hrs, Volume= 19,074 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.83' @ 12.56 hrs Surf.Area= 4,410 sf Storage= 4,939 cf

Plug-Flow detention time= 31.3 min calculated for 19,069 cf (100% of inflow)
Center-of-Mass det. time= 31.3 min (780.7 - 749.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	271.20'	4,137 cf	53.75'W x 82.05'L x 3.75'H Field A 16,538 cf Overall - 6,197 cf Embedded = 10,341 cf x 40.0% Voids
#2A	271.70'	6,197 cf	ADS StormTech SC-800 +Cap x 121 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 121 Chambers in 11 Rows Cap Storage= 3.4 cf x 2 x 11 rows = 75.2 cf
			10,333 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	271.70'	15.0" Round Culvert L= 139.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 271.70' / 269.85' S= 0.0133 ' / Cc= 0.900 n= 0.013, Flow Area= 1.23 sf
#2	Device 1	274.30'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Discarded	271.20'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.84 cfs @ 11.71 hrs HW=271.24' (Free Discharge)
3=Exfiltration (Exfiltration Controls 0.84 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=271.20' TW=0.00' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Summary for Pond UIS2: Underground Chamber #2

Inflow Area = 22,636 sf, 23.54% Impervious, Inflow Depth = 1.98" for 10 Year event
Inflow = 0.99 cfs @ 12.09 hrs, Volume= 3,735 cf
Outflow = 0.97 cfs @ 12.10 hrs, Volume= 3,735 cf, Atten= 2%, Lag= 0.9 min
Discarded = 0.09 cfs @ 11.45 hrs, Volume= 3,029 cf
Primary = 0.88 cfs @ 12.10 hrs, Volume= 707 cf
Routed to Pond LP-1 : Low Point - MAHW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.93' @ 12.10 hrs Surf.Area= 470 sf Storage= 542 cf
Flood Elev= 275.00' Surf.Area= 470 sf Storage= 580 cf

Plug-Flow detention time= 28.6 min calculated for 3,734 cf (100% of inflow)
Center-of-Mass det. time= 28.6 min (800.1 - 771.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	271.80'	344 cf	14.83'W x 31.68'L x 2.33'H Field A 1,096 cf Overall - 236 cf Embedded = 861 cf x 40.0% Voids
#2A	272.30'	236 cf	ADS_StormTech SC-310 +Cap x 16 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 16 Chambers in 4 Rows
			580 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	272.60'	12.0" Round Culvert L= 60.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 272.60' / 272.00' S= 0.0100 ' /' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	273.75'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Discarded	271.80'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.09 cfs @ 11.45 hrs HW=271.83' (Free Discharge)
3=Exfiltration (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.87 cfs @ 12.10 hrs HW=273.93' TW=268.24' (Dynamic Tailwater)
1=Culvert (Passes 0.87 cfs of 2.72 cfs potential flow)
2=Broad-Crested Rectangular Weir (Weir Controls 0.87 cfs @ 1.19 fps)

Summary for Pond UIS3: Underground Chamber #3

Inflow Area = 84,596 sf, 90.26% Impervious, Inflow Depth = 4.52" for 10 Year event
Inflow = 8.96 cfs @ 12.08 hrs, Volume= 31,877 cf
Outflow = 1.56 cfs @ 12.54 hrs, Volume= 31,877 cf, Atten= 83%, Lag= 27.3 min
Discarded = 1.33 cfs @ 11.70 hrs, Volume= 31,507 cf
Primary = 0.23 cfs @ 12.54 hrs, Volume= 370 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.75' @ 12.54 hrs Surf.Area= 6,945 sf Storage= 8,422 cf

Plug-Flow detention time= 33.4 min calculated for 31,868 cf (100% of inflow)
Center-of-Mass det. time= 33.4 min (782.0 - 748.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	272.00'	6,516 cf	30.00'W x 231.50'L x 3.75'H Field A 26,044 cf Overall - 9,755 cf Embedded = 16,289 cf x 40.0% Voids
#2A	272.50'	9,755 cf	ADS_StormTech SC-800 +Cap x 192 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 192 Chambers in 6 Rows Cap Storage= 3.4 cf x 2 x 6 rows = 41.0 cf
			16,270 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	273.29'	12.0" Round Culvert L= 110.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 273.29' / 272.00' S= 0.0117 ' /' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	275.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Device 1	273.50'	9.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	272.00'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=1.33 cfs @ 11.70 hrs HW=272.04' (Free Discharge)
4=Exfiltration (Exfiltration Controls 1.33 cfs)

Primary OutFlow Max=0.23 cfs @ 12.54 hrs HW=273.75' TW=0.00' (Dynamic Tailwater)
1=Culvert (Passes 0.23 cfs of 0.65 cfs potential flow)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
3=Orifice/Grate (Orifice Controls 0.23 cfs @ 1.72 fps)

Summary for Pond UIS4: Underground Chamber #4

Inflow Area = 42,721 sf, 77.28% Impervious, Inflow Depth = 3.91" for 10 Year event
Inflow = 3.87 cfs @ 12.08 hrs, Volume= 13,908 cf
Outflow = 1.04 cfs @ 12.43 hrs, Volume= 13,908 cf, Atten= 73%, Lag= 21.1 min
Discarded = 0.46 cfs @ 11.65 hrs, Volume= 12,941 cf
Primary = 0.58 cfs @ 12.43 hrs, Volume= 966 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.68' @ 12.43 hrs Surf.Area= 2,398 sf Storage= 3,618 cf

Plug-Flow detention time= 38.2 min calculated for 13,904 cf (100% of inflow)
Center-of-Mass det. time= 38.1 min (789.0 - 750.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	272.50'	2,279 cf	39.50'W x 60.70'L x 3.75'H Field A 8,991 cf Overall - 3,293 cf Embedded = 5,699 cf x 40.0% Voids
#2A	273.00'	3,293 cf	ADS_StormTech SC-800 +Cap x 64 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 64 Chambers in 8 Rows Cap Storage= 3.4 cf x 2 x 8 rows = 54.7 cf
		5,572 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	273.00'	12.0" Round Culvert L= 264.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 273.00' / 271.00' S= 0.0076 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	276.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Device 1	274.25'	9.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	272.50'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.46 cfs @ 11.65 hrs HW=272.54' (Free Discharge)
↳**4=Exfiltration** (Exfiltration Controls 0.46 cfs)

Primary OutFlow Max=0.58 cfs @ 12.43 hrs HW=274.68' TW=0.00' (Dynamic Tailwater)
↳**1=Culvert** (Passes 0.58 cfs of 3.23 cfs potential flow)
↳**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)
↳**3=Orifice/Grate** (Orifice Controls 0.58 cfs @ 2.23 fps)

Summary for Pond UIS5: Underground Chamber #5

Inflow Area = 157,891 sf, 63.68% Impervious, Inflow Depth = 3.26" for 10 Year event
Inflow = 11.79 cfs @ 12.08 hrs, Volume= 42,932 cf
Outflow = 1.66 cfs @ 11.70 hrs, Volume= 42,932 cf, Atten= 86%, Lag= 0.0 min
Discarded = 1.66 cfs @ 11.70 hrs, Volume= 42,932 cf
Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf
Routed to Pond MH19 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 271.97' @ 12.59 hrs Surf.Area= 8,653 sf Storage= 11,673 cf

Plug-Flow detention time= 40.0 min calculated for 42,920 cf (100% of inflow)
Center-of-Mass det. time= 40.0 min (794.1 - 754.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	269.80'	11,567 cf	30.00'W x 288.43'L x 4.75'H Field A 41,102 cf Overall - 12,183 cf Embedded = 28,919 cf x 40.0% Voids
#2A	270.80'	12,183 cf	ADS_StormTech SC-800 +Cap x 240 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 240 Chambers in 6 Rows Cap Storage= 3.4 cf x 2 x 6 rows = 41.0 cf
		23,751 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	273.05'	12.0" Round Culvert L= 97.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 273.05' / 272.00' S= 0.0108 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	274.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Discarded	269.80'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=1.66 cfs @ 11.70 hrs HW=269.85' (Free Discharge)
↳**3=Exfiltration** (Exfiltration Controls 1.66 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=269.80' TW=269.87' (Dynamic Tailwater)
↳**1=Culvert** (Controls 0.00 cfs)
↳**2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Summary for Link SP1: On Site Flow To Perennial Stream

Inflow Area = 210,774 sf, 10.22% Impervious, Inflow Depth = 0.21" for 10 Year event
Inflow = 2.24 cfs @ 12.13 hrs, Volume= 3,647 cf
Primary = 2.24 cfs @ 12.13 hrs, Volume= 3,647 cf, Atten= 0%, Lag= 0.0 min
Routed to Pond LP-2 : Low Point - MAHW

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP2: On Site Flow To Perennial Stream

Inflow Area = 236,963 sf, 10.14% Impervious, Inflow Depth = 0.25" for 10 Year event
Inflow = 2.08 cfs @ 12.19 hrs, Volume= 4,931 cf
Primary = 2.08 cfs @ 12.19 hrs, Volume= 4,931 cf, Atten= 0%, Lag= 0.0 min
Routed to Link SP3 : On Site Flow To Perennial Stream

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP3: On Site Flow To Perennial Stream

Inflow Area = 867,482 sf, 36.39% Impervious, Inflow Depth = 0.33" for 10 Year event
Inflow = 5.77 cfs @ 12.17 hrs, Volume= 24,162 cf
Primary = 5.77 cfs @ 12.17 hrs, Volume= 24,162 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points x 3
Runoff by SCS TR-20 method, UH=SCS, Weighted-Q
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment CB1: Subcat CB1	Runoff Area=10,622 sf 86.84% Impervious Runoff Depth=7.04" Tc=6.0 min CN=WQ Runoff=1.72 cfs 6,230 cf
Subcatchment CB10A: Subcat CB10A	Runoff Area=2,889 sf 81.34% Impervious Runoff Depth=6.67" Tc=6.0 min CN=WQ Runoff=0.44 cfs 1,606 cf
Subcatchment CB10B: Subcat CB10B	Runoff Area=10,414 sf 86.69% Impervious Runoff Depth=7.03" Tc=6.0 min CN=WQ Runoff=1.69 cfs 6,100 cf
Subcatchment CB11: Subcat CB11	Runoff Area=11,505 sf 85.23% Impervious Runoff Depth=6.93" Tc=6.0 min CN=WQ Runoff=1.84 cfs 6,645 cf
Subcatchment CB12: Subcat CB12	Runoff Area=7,149 sf 71.54% Impervious Runoff Depth=6.01" Tc=6.0 min CN=WQ Runoff=0.98 cfs 3,583 cf
Subcatchment CB13: Subcat CB13	Runoff Area=4,141 sf 88.54% Impervious Runoff Depth=7.15" Tc=6.0 min CN=WQ Runoff=0.68 cfs 2,468 cf
Subcatchment CB14: Subcat CB14	Runoff Area=6,167 sf 87.00% Impervious Runoff Depth=7.05" Tc=6.0 min CN=WQ Runoff=1.00 cfs 3,623 cf
Subcatchment CB15: Subcat CB15	Runoff Area=20,154 sf 44.65% Impervious Runoff Depth=4.21" Tc=6.0 min CN=WQ Runoff=1.88 cfs 7,078 cf
Subcatchment CB16: Subcat CB16	Runoff Area=20,584 sf 35.58% Impervious Runoff Depth=3.61" Tc=6.0 min CN=WQ Runoff=1.62 cfs 6,187 cf
Subcatchment CB17: Subcat CB17	Runoff Area=36,035 sf 27.88% Impervious Runoff Depth=3.09" Tc=6.0 min CN=WQ Runoff=2.38 cfs 9,283 cf
Subcatchment CB2: Subcat CB2	Runoff Area=12,976 sf 8.11% Impervious Runoff Depth=1.77" Tc=6.0 min CN=WQ Runoff=0.45 cfs 1,912 cf
Subcatchment CB3: Subcat CB3	Runoff Area=9,660 sf 44.27% Impervious Runoff Depth=6.27" Tc=6.0 min CN=WQ Runoff=1.40 cfs 5,044 cf
Subcatchment CB4A: Subcat CB4A	Runoff Area=3,976 sf 91.93% Impervious Runoff Depth=7.38" Tc=6.0 min CN=WQ Runoff=0.68 cfs 2,445 cf
Subcatchment CB4B: Subcat CB4B	Runoff Area=10,218 sf 73.42% Impervious Runoff Depth=6.14" Tc=6.0 min CN=WQ Runoff=1.43 cfs 5,228 cf
Subcatchment CB4C: Subcat CB4C	Runoff Area=10,741 sf 84.88% Impervious Runoff Depth=6.91" Tc=6.0 min CN=WQ Runoff=1.71 cfs 6,183 cf
Subcatchment CB4D: Subcat CB4D	Runoff Area=10,203 sf 79.98% Impervious Runoff Depth=6.58" Tc=6.0 min CN=WQ Runoff=1.54 cfs 5,594 cf

Subcatchment CB5A: Subcat CB5A	Runoff Area=10,459 sf 93.11% Impervious Runoff Depth=7.46" Tc=6.0 min CN=WQ Runoff=1.81 cfs 6,501 cf
Subcatchment CB5B: Subcat CB5B	Runoff Area=3,865 sf 95.02% Impervious Runoff Depth=7.59" Tc=6.0 min CN=WQ Runoff=0.68 cfs 2,444 cf
Subcatchment CB5C: Subcat CB5C	Runoff Area=2,619 sf 91.40% Impervious Runoff Depth=7.34" Tc=6.0 min CN=WQ Runoff=0.44 cfs 1,603 cf
Subcatchment CB6: Subcat CB6	Runoff Area=12,631 sf 77.41% Impervious Runoff Depth=6.41" Tc=6.0 min CN=WQ Runoff=1.86 cfs 6,744 cf
Subcatchment CB7A: Subcat CB7A	Runoff Area=7,101 sf 79.91% Impervious Runoff Depth=6.58" Tc=6.0 min CN=WQ Runoff=1.07 cfs 3,891 cf
Subcatchment CB7B: Subcat CB7B	Runoff Area=11,340 sf 75.16% Impervious Runoff Depth=6.26" Tc=6.0 min CN=WQ Runoff=1.62 cfs 5,913 cf
Subcatchment CB8: Subcat CB8	Runoff Area=10,991 sf 81.26% Impervious Runoff Depth=6.67" Tc=6.0 min CN=WQ Runoff=1.68 cfs 6,105 cf
Subcatchment CB9A: Subcat CB9A	Runoff Area=11,743 sf 81.25% Impervious Runoff Depth=6.66" Tc=6.0 min CN=WQ Runoff=1.80 cfs 6,522 cf
Subcatchment CB9B: Subcat CB9B	Runoff Area=14,736 sf 67.40% Impervious Runoff Depth=5.74" Tc=6.0 min CN=WQ Runoff=1.92 cfs 7,045 cf
Subcatchment CB9C: Subcat CB9C	Runoff Area=3,708 sf 100.00% Impervious Runoff Depth=7.92" Tc=6.0 min CN=WQ Runoff=0.68 cfs 2,448 cf
Subcatchment CB9D: Subcat CB9D	Runoff Area=1,543 sf 58.43% Impervious Runoff Depth=5.14" Tc=6.0 min CN=WQ Runoff=0.18 cfs 660 cf
Subcatchment P-1: Subcat P-1	Runoff Area=14,881 sf 16.65% Impervious Runoff Depth=2.34" Tc=6.0 min CN=WQ Runoff=0.71 cfs 2,901 cf
Subcatchment P-2: Subcat P-2	Runoff Area=11,308 sf 0.00% Impervious Runoff Depth=2.66" Tc=6.0 min CN=WQ Runoff=0.64 cfs 2,503 cf
Subcatchment P-3: Subcat P-3	Runoff Area=7,515 sf 5.81% Impervious Runoff Depth=1.61" Tc=6.0 min CN=WQ Runoff=0.23 cfs 1,011 cf
Subcatchment P-4: Subcat P-4	Runoff Area=291,674 sf 12.39% Impervious Runoff Depth=1.67" Tc=12.0 min CN=WQ Runoff=7.28 cfs 40,483 cf
Subcatchment P-5: Subcat P-5	Runoff Area=76,040 sf 11.69% Impervious Runoff Depth=2.01" Tc=8.0 min CN=WQ Runoff=2.83 cfs 12,721 cf
Subcatchment P-6: Subcat P-6	Runoff Area=25,354 sf 10.27% Impervious Runoff Depth=1.91" Tc=6.0 min CN=WQ Runoff=0.96 cfs 4,041 cf

Subcatchment P-7: Subcat P-7	Runoff Area=79,230 sf 5.41% Impervious Runoff Depth=1.59" Tc=11.4 min CN=WQ Runoff=1.98 cfs 10,478 cf
Subcatchment PATIO: Subcat PATIO	Runoff Area=2,833 sf 100.00% Impervious Runoff Depth=7.92" Tc=6.0 min CN=98 Runoff=0.52 cfs 1,870 cf
Subcatchment R-1: Subcat R-1	Runoff Area=5,046 sf 100.00% Impervious Runoff Depth=7.92" Tc=6.0 min CN=98 Runoff=0.93 cfs 3,330 cf
Subcatchment R-2: Subcat R-2	Runoff Area=18,290 sf 100.00% Impervious Runoff Depth=7.92" Tc=6.0 min CN=98 Runoff=3.36 cfs 12,072 cf
Subcatchment R-3: Subcat R-3	Runoff Area=18,290 sf 100.00% Impervious Runoff Depth=7.92" Tc=6.0 min CN=98 Runoff=3.36 cfs 12,072 cf
Subcatchment R-4: Subcat R-4	Runoff Area=18,290 sf 100.00% Impervious Runoff Depth=7.92" Tc=6.0 min CN=98 Runoff=3.36 cfs 12,072 cf
Subcatchment R-5: Subcat R-5	Runoff Area=20,565 sf 100.00% Impervious Runoff Depth=7.92" Tc=6.0 min CN=98 Runoff=3.78 cfs 13,573 cf
Pond 1: Catch Basin	Peak Elev=274.47' Inflow=1.72 cfs 6,230 cf 12.0" Round Culvert n=0.012 L=84.0' S=0.0060 '/' Outflow=1.72 cfs 6,230 cf
Pond 2: Catch Basin	Peak Elev=274.15' Inflow=0.45 cfs 1,912 cf 12.0" Round Culvert n=0.012 L=47.0' S=0.0057 '/' Outflow=0.45 cfs 1,912 cf
Pond 3: Catch Basin	Peak Elev=274.31' Inflow=1.40 cfs 5,044 cf 12.0" Round Culvert n=0.012 L=77.0' S=0.0052 '/' Outflow=1.40 cfs 5,044 cf
Pond 4A: Catch Basin	Peak Elev=274.47' Inflow=0.68 cfs 2,445 cf 12.0" Round Culvert n=0.012 L=7.0' S=0.0100 '/' Outflow=0.68 cfs 2,445 cf
Pond 4B: Catch Basin	Peak Elev=274.48' Inflow=1.43 cfs 5,228 cf 12.0" Round Culvert n=0.012 L=27.0' S=0.0100 '/' Outflow=1.43 cfs 5,228 cf
Pond 4C: Catch Basin	Peak Elev=274.50' Inflow=1.71 cfs 6,183 cf 12.0" Round Culvert n=0.012 L=61.0' S=0.0077 '/' Outflow=1.71 cfs 6,183 cf
Pond 4D: Catch Basin	Peak Elev=274.50' Inflow=1.54 cfs 5,594 cf 12.0" Round Culvert n=0.012 L=38.0' S=0.0100 '/' Outflow=1.54 cfs 5,594 cf
Pond 5A: Catch Basin	Peak Elev=274.91' Inflow=1.81 cfs 6,501 cf 12.0" Round Culvert n=0.012 L=28.0' S=0.0100 '/' Outflow=1.81 cfs 6,501 cf
Pond 5B: Catch Basin	Peak Elev=274.90' Inflow=0.68 cfs 2,444 cf 12.0" Round Culvert n=0.012 L=45.0' S=0.0100 '/' Outflow=0.68 cfs 2,444 cf
Pond 5C: Catch Basin	Peak Elev=274.90' Inflow=0.44 cfs 1,603 cf 12.0" Round Culvert n=0.012 L=41.0' S=0.0100 '/' Outflow=0.44 cfs 1,603 cf

Pond 6: Catch Basin	Peak Elev=274.90' Inflow=1.86 cfs 6,744 cf 12.0" Round Culvert n=0.012 L=26.0' S=0.0100 'l' Outflow=1.86 cfs 6,744 cf
Pond 7A: Catch Basin	Peak Elev=274.90' Inflow=1.07 cfs 3,891 cf 12.0" Round Culvert n=0.012 L=10.0' S=0.0100 'l' Outflow=1.07 cfs 3,891 cf
Pond 7B: Catch Basin	Peak Elev=274.91' Inflow=1.62 cfs 5,913 cf 12.0" Round Culvert n=0.012 L=40.0' S=0.0100 'l' Outflow=1.62 cfs 5,913 cf
Pond 8: Catch Basin	Peak Elev=275.87' Inflow=1.68 cfs 6,105 cf 12.0" Round Culvert n=0.012 L=27.0' S=0.0100 'l' Outflow=1.68 cfs 6,105 cf
Pond 9A: Catch Basin	Peak Elev=275.89' Inflow=1.80 cfs 6,522 cf 12.0" Round Culvert n=0.012 L=14.0' S=0.0050 'l' Outflow=1.80 cfs 6,522 cf
Pond 9B: Catch Basin	Peak Elev=275.90' Inflow=1.92 cfs 7,045 cf 12.0" Round Culvert n=0.012 L=64.0' S=0.0059 'l' Outflow=1.92 cfs 7,045 cf
Pond 9C: Catch Basin	Peak Elev=275.80' Inflow=0.68 cfs 2,448 cf 12.0" Round Culvert n=0.012 L=7.0' S=0.0114 'l' Outflow=0.68 cfs 2,448 cf
Pond 9D: Catch Basin	Peak Elev=275.85' Inflow=0.18 cfs 660 cf 12.0" Round Culvert n=0.012 L=49.0' S=0.0090 'l' Outflow=0.18 cfs 660 cf
Pond 10A: Catch Basin	Peak Elev=274.31' Inflow=0.44 cfs 1,606 cf 12.0" Round Culvert n=0.012 L=87.0' S=0.0100 'l' Outflow=0.44 cfs 1,606 cf
Pond 10B: Catch Basin	Peak Elev=274.31' Inflow=1.69 cfs 6,100 cf 12.0" Round Culvert n=0.012 L=15.0' S=0.0100 'l' Outflow=1.69 cfs 6,100 cf
Pond 11: Catch Basin	Peak Elev=274.30' Inflow=1.84 cfs 6,645 cf 12.0" Round Culvert n=0.012 L=15.0' S=0.0100 'l' Outflow=1.84 cfs 6,645 cf
Pond 12: Catch Basin	Peak Elev=274.32' Inflow=0.98 cfs 3,583 cf 12.0" Round Culvert n=0.012 L=14.0' S=0.0100 'l' Outflow=0.98 cfs 3,583 cf
Pond 13: Catch Basin	Peak Elev=274.32' Inflow=0.68 cfs 2,468 cf 12.0" Round Culvert n=0.012 L=87.0' S=0.0100 'l' Outflow=0.68 cfs 2,468 cf
Pond 14: Catch Basin	Peak Elev=274.32' Inflow=1.00 cfs 3,623 cf 12.0" Round Culvert n=0.012 L=33.0' S=0.0100 'l' Outflow=1.00 cfs 3,623 cf
Pond 15: Catch Basin	Peak Elev=274.36' Inflow=1.88 cfs 7,078 cf 12.0" Round Culvert n=0.012 L=19.0' S=0.0100 'l' Outflow=1.88 cfs 7,078 cf
Pond 16: Catch Basin	Peak Elev=275.20' Inflow=1.62 cfs 6,187 cf 12.0" Round Culvert n=0.012 L=19.0' S=0.0100 'l' Outflow=1.62 cfs 6,187 cf
Pond 17: Catch Basin	Peak Elev=275.84' Inflow=2.38 cfs 9,283 cf 15.0" Round Culvert n=0.012 L=143.0' S=0.0061 'l' Outflow=2.38 cfs 9,283 cf

Pond LP-1: Low Point - MAHW	Peak Elev=268.73' Storage=425 cf Inflow=6.90 cfs 19,447 cf Outflow=6.88 cfs 19,352 cf
Pond LP-2: Low Point - MAHW	Peak Elev=267.86' Storage=1,302 cf Inflow=7.92 cfs 22,678 cf 24.0" Round Culvert n=0.011 L=30.0' S=-0.0160 'l' Outflow=7.42 cfs 22,678 cf
Pond MH1: Manhole	Peak Elev=274.46' Inflow=1.72 cfs 6,230 cf 24.0" Round Culvert n=0.012 L=10.0' S=0.0000 'l' Outflow=1.72 cfs 6,230 cf
Pond MH11: Manhole	Peak Elev=275.77' Inflow=3.61 cfs 13,150 cf 24.0" Round Culvert n=0.012 L=4.0' S=0.0000 'l' Outflow=3.61 cfs 13,150 cf
Pond MH12: Manhole	Peak Elev=275.80' Inflow=2.66 cfs 9,630 cf 15.0" Round Culvert n=0.012 L=28.0' S=0.0057 'l' Outflow=2.66 cfs 9,630 cf
Pond MH13: Manhole	Peak Elev=275.76' Inflow=2.66 cfs 9,630 cf 24.0" Round Culvert n=0.012 L=10.0' S=0.0000 'l' Outflow=2.66 cfs 9,630 cf
Pond MH15: Manhole	Peak Elev=274.31' Inflow=2.13 cfs 7,705 cf 12.0" Round Culvert n=0.012 L=14.0' S=0.0100 'l' Outflow=2.13 cfs 7,705 cf
Pond MH16: Manhole	Peak Elev=274.30' Inflow=2.13 cfs 7,705 cf 24.0" Round Culvert n=0.012 L=5.0' S=0.0000 'l' Outflow=2.13 cfs 7,705 cf
Pond MH17: Manhole	Peak Elev=274.30' Inflow=1.84 cfs 6,645 cf 24.0" Round Culvert n=0.012 L=7.0' S=0.0000 'l' Outflow=1.84 cfs 6,640 cf
Pond MH19: Manhole	Peak Elev=270.54' Inflow=1.84 cfs 2,546 cf 18.0" Round Culvert n=0.012 L=51.0' S=0.0069 'l' Outflow=1.84 cfs 2,546 cf
Pond MH21: Manhole	Peak Elev=274.31' Inflow=8.54 cfs 32,222 cf 24.0" Round Culvert n=0.012 L=20.0' S=0.0100 'l' Outflow=8.54 cfs 32,222 cf
Pond MH22: Manhole	Peak Elev=274.32' Inflow=8.54 cfs 32,222 cf 24.0" Round Culvert n=0.012 L=22.0' S=0.0100 'l' Outflow=8.54 cfs 32,222 cf
Pond MH23: Manhole	Peak Elev=274.34' Inflow=5.88 cfs 22,548 cf 18.0" Round Culvert n=0.012 L=121.0' S=0.0100 'l' Outflow=5.88 cfs 22,548 cf
Pond MH24: Manhole	Peak Elev=274.97' Inflow=4.00 cfs 15,470 cf 18.0" Round Culvert n=0.012 L=176.0' S=0.0100 'l' Outflow=4.00 cfs 15,470 cf
Pond MH26: Manhole	Peak Elev=274.47' Inflow=5.36 cfs 19,450 cf 18.0" Round Culvert n=0.012 L=43.0' S=0.0100 'l' Outflow=5.36 cfs 19,450 cf
Pond MH27: Manhole	Peak Elev=274.13' Inflow=1.84 cfs 6,956 cf 15.0" Round Culvert n=0.012 L=8.0' S=0.0050 'l' Outflow=1.84 cfs 6,956 cf
Pond MH28: Manhole	Peak Elev=274.90' Inflow=2.93 cfs 10,548 cf 15.0" Round Culvert n=0.012 L=59.0' S=0.0100 'l' Outflow=2.93 cfs 10,548 cf

Pond MH29: Manhole	Peak Elev=274.48' Inflow=3.25 cfs 11,777 cf 15.0" Round Culvert n=0.012 L=11.0' S=0.0100 '/' Outflow=3.25 cfs 11,777 cf
Pond MH30: Manhole	Peak Elev=274.49' Inflow=3.25 cfs 11,777 cf 15.0" Round Culvert n=0.012 L=34.0' S=0.0100 '/' Outflow=3.25 cfs 11,777 cf
Pond MH31: Manhole	Peak Elev=274.90' Inflow=0.44 cfs 1,603 cf 12.0" Round Culvert n=0.012 L=20.0' S=0.0100 '/' Outflow=0.44 cfs 1,603 cf
Pond MH32: Manhole	Peak Elev=275.85' Inflow=1.98 cfs 7,182 cf 12.0" Round Culvert n=0.012 L=48.0' S=0.0052 '/' Outflow=1.98 cfs 7,182 cf
Pond MH33: Manhole	Peak Elev=275.84' Inflow=3.61 cfs 13,150 cf 15.0" Round Culvert n=0.012 L=20.0' S=0.0100 '/' Outflow=3.61 cfs 13,150 cf
Pond MH6: Manhole	Peak Elev=274.88' Inflow=2.93 cfs 10,548 cf 24.0" Round Culvert n=0.012 L=6.0' S=0.0000 '/' Outflow=2.93 cfs 10,548 cf
Pond MH7: Manhole	Peak Elev=274.88' Inflow=1.86 cfs 6,744 cf 24.0" Round Culvert n=0.012 L=6.0' S=0.0000 '/' Outflow=1.86 cfs 6,744 cf
Pond MH8: Manhole	Peak Elev=274.89' Inflow=2.70 cfs 9,803 cf 15.0" Round Culvert n=0.012 L=22.0' S=0.0100 '/' Outflow=2.70 cfs 9,803 cf
Pond MH9: Manhole	Peak Elev=274.88' Inflow=2.70 cfs 9,803 cf 24.0" Round Culvert n=0.012 L=8.0' S=0.0000 '/' Outflow=2.70 cfs 9,803 cf
Pond RD1: Roof Drain	Peak Elev=274.46' Inflow=1.45 cfs 5,200 cf 12.0" Round Culvert n=0.012 L=21.0' S=0.0100 '/' Outflow=1.45 cfs 5,200 cf
Pond RD2: Roof Drain	Peak Elev=274.93' Inflow=3.36 cfs 12,072 cf 12.0" Round Culvert n=0.012 L=31.0' S=0.0100 '/' Outflow=3.36 cfs 12,072 cf
Pond RD3: Roof Drain	Peak Elev=274.93' Inflow=3.36 cfs 12,072 cf 12.0" Round Culvert n=0.012 L=14.5' S=0.0103 '/' Outflow=3.36 cfs 12,072 cf
Pond RD4: Roof Drain	Peak Elev=274.32' Inflow=3.36 cfs 12,072 cf 12.0" Round Culvert n=0.012 L=24.0' S=0.0104 '/' Outflow=3.36 cfs 12,072 cf
Pond RD5: Roof Drain	Peak Elev=274.31' Inflow=3.78 cfs 13,573 cf 15.0" Round Culvert n=0.012 L=18.0' S=0.0100 '/' Outflow=3.78 cfs 13,573 cf
Pond TD1: Stone Trench Drain	Peak Elev=272.67' Storage=471 cf Inflow=5.49 cfs 27,240 cf Discarded=0.20 cfs 11,376 cf Primary=5.24 cfs 15,864 cf Outflow=5.44 cfs 27,240 cf
Pond TD2: Stone Trench Drain	Peak Elev=273.36' Storage=308 cf Inflow=0.64 cfs 2,503 cf Discarded=0.08 cfs 2,077 cf Primary=0.44 cfs 425 cf Outflow=0.52 cfs 2,503 cf
Pond UIS1: Underground Chamber #1	Peak Elev=274.46' Storage=9,465 cf Inflow=8.54 cfs 30,881 cf Discarded=0.84 cfs 30,230 cf Primary=0.70 cfs 650 cf Outflow=1.54 cfs 30,881 cf

Pond UIS2: Underground Chamber #2	Peak Elev=274.04' Storage=562 cf Inflow=1.84 cfs 6,956 cf Discarded=0.09 cfs 4,383 cf Primary=1.74 cfs 2,573 cf Outflow=1.83 cfs 6,956 cf
Pond UIS3: Underground Chamber #3	Peak Elev=274.88' Storage=13,690 cf Inflow=14.21 cfs 51,238 cf Discarded=1.33 cfs 43,519 cf Primary=2.13 cfs 7,720 cf Outflow=3.46 cfs 51,238 cf
Pond UIS4: Underground Chamber #4	Peak Elev=275.75' Storage=5,097 cf Inflow=6.27 cfs 22,780 cf Discarded=0.46 cfs 17,563 cf Primary=2.26 cfs 5,216 cf Outflow=2.72 cfs 22,780 cf
Pond UIS5: Underground Chamber #5	Peak Elev=274.30' Storage=22,870 cf Inflow=19.65 cfs 72,212 cf Discarded=1.66 cfs 69,666 cf Primary=1.84 cfs 2,546 cf Outflow=3.49 cfs 72,212 cf
Link SP1: On Site Flow To Perennial Stream	Inflow=6.88 cfs 19,352 cf Primary=6.88 cfs 19,352 cf
Link SP2: On Site Flow To Perennial Stream	Inflow=7.42 cfs 22,678 cf Primary=7.42 cfs 22,678 cf
Link SP3: On Site Flow To Perennial Stream	Inflow=18.24 cfs 79,294 cf Primary=18.24 cfs 79,294 cf
Total Runoff Area = 867,482 sf Runoff Volume = 258,209 cf Average Runoff Depth = 3.57" 63.61% Pervious = 551,848 sf 36.39% Impervious = 315,634 sf	

Summary for Subcatchment CB1: Subcat CB1

Runoff = 1.72 cfs @ 12.08 hrs, Volume= 6,230 cf, Depth= 7.04"
Routed to Pond 1 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
1,398	39	>75% Grass cover, Good, HSG A
9,224	98	Paved parking, HSG A
10,622		Weighted Average
1,398	39	13.16% Pervious Area
9,224	98	86.84% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB10A: Subcat CB10A

Runoff = 0.44 cfs @ 12.08 hrs, Volume= 1,606 cf, Depth= 6.67"
Routed to Pond 10A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
539	39	>75% Grass cover, Good, HSG A
2,350	98	Paved parking, HSG A
2,889		Weighted Average
539	39	18.66% Pervious Area
2,350	98	81.34% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB10B: Subcat CB10B

Runoff = 1.69 cfs @ 12.08 hrs, Volume= 6,100 cf, Depth= 7.03"
Routed to Pond 10B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
1,386	39	>75% Grass cover, Good, HSG A
9,027	98	Paved parking, HSG A
10,414		Weighted Average
1,386	39	13.31% Pervious Area
9,027	98	86.69% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB11: Subcat CB11

Runoff = 1.84 cfs @ 12.08 hrs, Volume= 6,645 cf, Depth= 6.93"
Routed to Pond 11 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
1,699	39	>75% Grass cover, Good, HSG A
9,806	98	Paved parking, HSG A
11,505		Weighted Average
1,699	39	14.77% Pervious Area
9,806	98	85.23% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB12: Subcat CB12

Runoff = 0.98 cfs @ 12.08 hrs, Volume= 3,583 cf, Depth= 6.01"
Routed to Pond 12 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
2,035	39	>75% Grass cover, Good, HSG A
5,114	98	Paved parking, HSG A
7,149		Weighted Average
2,035	39	28.46% Pervious Area
5,114	98	71.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB13: Subcat CB13

Runoff = 0.68 cfs @ 12.08 hrs, Volume= 2,468 cf, Depth= 7.15"
Routed to Pond 13 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
474	39	>75% Grass cover, Good, HSG A
3,666	98	Paved parking, HSG A
4,141		Weighted Average
474	39	11.46% Pervious Area
3,666	98	88.54% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB14: Subcat CB14

Runoff = 1.00 cfs @ 12.08 hrs, Volume= 3,623 cf, Depth= 7.05"
Routed to Pond 14 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
801	39	>75% Grass cover, Good, HSG A
4,765	98	Paved parking, HSG A
600	98	Roofs, HSG A
6,167		Weighted Average
801	39	13.00% Pervious Area
5,365	98	87.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB15: Subcat CB15

Runoff = 1.88 cfs @ 12.09 hrs, Volume= 7,078 cf, Depth= 4.21"
Routed to Pond 15 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
11,154	39	>75% Grass cover, Good, HSG A
8,024	98	Paved parking, HSG A
975	98	Roofs, HSG A
20,154		Weighted Average
11,154	39	55.35% Pervious Area
9,000	98	44.65% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB16: Subcat CB16

Runoff = 1.62 cfs @ 12.09 hrs, Volume= 6,187 cf, Depth= 3.61"
Routed to Pond 16 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
13,260	39	>75% Grass cover, Good, HSG A
7,324	98	Paved parking, HSG A
20,584		Weighted Average
13,260	39	64.42% Pervious Area
7,324	98	35.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB17: Subcat CB17

Runoff = 2.38 cfs @ 12.09 hrs, Volume= 9,283 cf, Depth= 3.09"
Routed to Pond 17 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
25,990	39	>75% Grass cover, Good, HSG A
8,281	98	Paved parking, HSG A
1,764	98	Roofs, HSG A
36,035		Weighted Average
25,990	39	72.12% Pervious Area
10,045	98	27.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB2: Subcat CB2

Runoff = 0.45 cfs @ 12.10 hrs, Volume= 1,912 cf, Depth= 1.77"
Routed to Pond 2 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
11,923	39	>75% Grass cover, Good, HSG A
1,053	98	Paved parking, HSG A
12,976		Weighted Average
11,923	39	91.89% Pervious Area
1,053	98	8.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB3: Subcat CB3

Runoff = 1.40 cfs @ 12.08 hrs, Volume= 5,044 cf, Depth= 6.27"
Routed to Pond 3 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
2,275	39	>75% Grass cover, Good, HSG A
3,109	96	Gravel surface, HSG A
4,276	98	Paved parking, HSG A
9,660		Weighted Average
5,384	72	55.73% Pervious Area
4,276	98	44.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4A: Subcat CB4A

Runoff = 0.68 cfs @ 12.08 hrs, Volume= 2,445 cf, Depth= 7.38"
Routed to Pond 4A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
321	39	>75% Grass cover, Good, HSG A
3,655	98	Paved parking, HSG A
3,976		Weighted Average
321	39	8.07% Pervious Area
3,655	98	91.93% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4B: Subcat CB4B

Runoff = 1.43 cfs @ 12.08 hrs, Volume= 5,228 cf, Depth= 6.14"
Routed to Pond 4B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
2,716	39	>75% Grass cover, Good, HSG A
7,501	98	Paved parking, HSG A
10,218		Weighted Average
2,716	39	26.58% Pervious Area
7,501	98	73.42% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4C: Subcat CB4C

Runoff = 1.71 cfs @ 12.08 hrs, Volume= 6,183 cf, Depth= 6.91"
Routed to Pond 4C : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
1,624	39	>75% Grass cover, Good, HSG A
7,101	98	Paved parking, HSG A
2,016	98	Roofs, HSG A
10,741		Weighted Average
1,624	39	15.12% Pervious Area
9,117	98	84.88% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB4D: Subcat CB4D

Runoff = 1.54 cfs @ 12.08 hrs, Volume= 5,594 cf, Depth= 6.58"
Routed to Pond 4D : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
2,043	39	>75% Grass cover, Good, HSG A
8,160	98	Paved parking, HSG A
10,203		Weighted Average
2,043	39	20.02% Pervious Area
8,160	98	79.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB5A: Subcat CB5A

Runoff = 1.81 cfs @ 12.08 hrs, Volume= 6,501 cf, Depth= 7.46"
Routed to Pond 5A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
721	39	>75% Grass cover, Good, HSG A
9,224	98	Paved parking, HSG A
514	98	Roofs, HSG A
10,459		Weighted Average
721	39	6.89% Pervious Area
9,738	98	93.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB5B: Subcat CB5B

Runoff = 0.68 cfs @ 12.08 hrs, Volume= 2,444 cf, Depth= 7.59"
Routed to Pond 5B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
193	39	>75% Grass cover, Good, HSG A
2,913	98	Paved parking, HSG A
760	98	Roofs, HSG A
3,865		Weighted Average
193	39	4.98% Pervious Area
3,673	98	95.02% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB5C: Subcat CB5C

Runoff = 0.44 cfs @ 12.08 hrs, Volume= 1,603 cf, Depth= 7.34"
Routed to Pond 5C : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
225	39	>75% Grass cover, Good, HSG A
2,394	98	Paved parking, HSG A
2,619		Weighted Average
225	39	8.60% Pervious Area
2,394	98	91.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB6: Subcat CB6

Runoff = 1.86 cfs @ 12.08 hrs, Volume= 6,744 cf, Depth= 6.41"
Routed to Pond 6 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
2,854	39	>75% Grass cover, Good, HSG A
9,777	98	Paved parking, HSG A
12,631		Weighted Average
2,854	39	22.59% Pervious Area
9,777	98	77.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry,

Summary for Subcatchment CB7A: Subcat CB7A

Runoff = 1.07 cfs @ 12.08 hrs, Volume= 3,891 cf, Depth= 6.58"
Routed to Pond 7A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
1,426	39	>75% Grass cover, Good, HSG A
5,674	98	Paved parking, HSG A
7,101		Weighted Average
1,426	39	20.09% Pervious Area
5,674	98	79.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB7B: Subcat CB7B

Runoff = 1.62 cfs @ 12.08 hrs, Volume= 5,913 cf, Depth= 6.26"
Routed to Pond 7B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
2,817	39	>75% Grass cover, Good, HSG A
7,718	98	Paved parking, HSG A
805	98	Roofs, HSG A
11,340		Weighted Average
2,817	39	24.84% Pervious Area
8,523	98	75.16% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB8: Subcat CB8

Runoff = 1.68 cfs @ 12.08 hrs, Volume= 6,105 cf, Depth= 6.67"
Routed to Pond 8 : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
2,060	39	>75% Grass cover, Good, HSG A
8,538	98	Paved parking, HSG A
393	98	Roofs, HSG A
10,991		Weighted Average
2,060	39	18.74% Pervious Area
8,931	98	81.26% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9A: Subcat CB9A

Runoff = 1.80 cfs @ 12.08 hrs, Volume= 6,522 cf, Depth= 6.66"
Routed to Pond 9A : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
2,202	39	>75% Grass cover, Good, HSG A
7,525	98	Paved parking, HSG A
2,016	98	Roofs, HSG A
11,743		Weighted Average
2,202	39	18.75% Pervious Area
9,541	98	81.25% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9B: Subcat CB9B

Runoff = 1.92 cfs @ 12.09 hrs, Volume= 7,045 cf, Depth= 5.74"
Routed to Pond 9B : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
4,805	39	>75% Grass cover, Good, HSG A
9,931	98	Paved parking, HSG A
14,736		Weighted Average
4,805	39	32.60% Pervious Area
9,931	98	67.40% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9C: Subcat CB9C

Runoff = 0.68 cfs @ 12.08 hrs, Volume= 2,448 cf, Depth= 7.92"
Routed to Pond 9C : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
0	39	>75% Grass cover, Good, HSG A
2,337	98	Paved parking, HSG A
1,371	98	Roofs, HSG A
3,708		Weighted Average
0	39	0.00% Pervious Area
3,708	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment CB9D: Subcat CB9D

Runoff = 0.18 cfs @ 12.09 hrs, Volume= 660 cf, Depth= 5.14"
Routed to Pond 9D : Catch Basin

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
641	39	>75% Grass cover, Good, HSG A
901	98	Paved parking, HSG A
1,543		Weighted Average
641	39	41.57% Pervious Area
901	98	58.43% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-1: Subcat P-1

Runoff = 0.71 cfs @ 12.10 hrs, Volume= 2,901 cf, Depth= 2.34"
Routed to Pond LP-2 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
12,404	39	>75% Grass cover, Good, HSG A
0	98	Paved parking, HSG A
2,477	98	Water Surface, HSG A
14,881		Weighted Average
12,404	39	83.35% Pervious Area
2,477	98	16.65% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-2: Subcat P-2

Runoff = 0.64 cfs @ 12.09 hrs, Volume= 2,503 cf, Depth= 2.66"
Routed to Pond TD2 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
8,801	39	>75% Grass cover, Good, HSG A
2,507	96	Gravel surface, HSG A
0	98	Paved parking, HSG A
11,308		Weighted Average
11,308	52	100.00% Pervious Area
0	98	0.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-3: Subcat P-3

Runoff = 0.23 cfs @ 12.11 hrs, Volume= 1,011 cf, Depth= 1.61"
Routed to Pond LP-1 : Low Point - MAHW

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
7,078	39	>75% Grass cover, Good, HSG A
5	98	Paved parking, HSG A
431	98	Water Surface, HSG A
7,515		Weighted Average
7,078	39	94.19% Pervious Area
436	98	5.81% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment P-4: Subcat P-4

Runoff = 7.28 cfs @ 12.17 hrs, Volume= 40,483 cf, Depth= 1.67"
Routed to Link SP3 : On Site Flow To Perennial Stream

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
85,817	39	>75% Grass cover, Good, HSG A
2,376	96	Gravel surface, HSG A
5,773	98	Paved parking, HSG A
30,379	98	Water Surface, HSG A
167,329	30	Woods, Good, HSG A
291,674		Weighted Average
255,522	34	87.61% Pervious Area
36,152	98	12.39% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
12.0					Direct Entry, Copied from EWS

Summary for Subcatchment P-5: Subcat P-5

Runoff = 2.83 cfs @ 12.13 hrs, Volume= 12,721 cf, Depth= 2.01"
Routed to Pond TD1 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
67,151	39	>75% Grass cover, Good, HSG A
8,890	98	Paved parking, HSG A
76,040		Weighted Average
67,151	39	88.31% Pervious Area
8,890	98	11.69% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
8.0					Direct Entry, Copied From EWS

Summary for Subcatchment P-6: Subcat P-6

Runoff = 0.96 cfs @ 12.10 hrs, Volume= 4,041 cf, Depth= 1.91"
Routed to Pond TD1 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
22,749	39	>75% Grass cover, Good, HSG A
2,605	98	Paved parking, HSG A
25,354		Weighted Average
22,749	39	89.73% Pervious Area
2,605	98	10.27% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Copied from EWS

Summary for Subcatchment P-7: Subcat P-7

Runoff = 1.98 cfs @ 12.19 hrs, Volume= 10,478 cf, Depth= 1.59"
Routed to Pond TD1 : Stone Trench Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
74,943	39	>75% Grass cover, Good, HSG A
4,287	98	Paved parking, HSG A
79,230		Weighted Average
74,943	39	94.59% Pervious Area
4,287	98	5.41% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.4					Direct Entry, Copied From EWS

Summary for Subcatchment PATIO: Subcat PATIO

Runoff = 0.52 cfs @ 12.08 hrs, Volume= 1,870 cf, Depth= 7.92"
Routed to Pond RD1 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
2,833	98	Paved parking, HSG A
2,833	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-1: Subcat R-1

Runoff = 0.93 cfs @ 12.08 hrs, Volume= 3,330 cf, Depth= 7.92"
Routed to Pond RD1 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
5,046	98	Roofs, HSG A
5,046	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-2: Subcat R-2

Runoff = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf, Depth= 7.92"
Routed to Pond RD2 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
18,290	98	Roofs, HSG A
18,290	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-3: Subcat R-3

Runoff = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf, Depth= 7.92"
Routed to Pond RD3 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
18,290	98	Roofs, HSG A
18,290	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-4: Subcat R-4

Runoff = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf, Depth= 7.92"
Routed to Pond RD4 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
18,290	98	Roofs, HSG A
18,290	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Subcatchment R-5: Subcat R-5

Runoff = 3.78 cfs @ 12.08 hrs, Volume= 13,573 cf, Depth= 7.92"
Routed to Pond RD5 : Roof Drain

Runoff by SCS TR-20 method, UH=SCS, Weighted-Q, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs
Type III 24-hr 100 Year Rainfall=8.16"

Area (sf)	CN	Description
20,565	98	Roofs, HSG A
20,565	98	100.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					Direct Entry, Min. TC

Summary for Pond 1: Catch Basin

Inflow Area = 10,622 sf, 86.84% Impervious, Inflow Depth = 7.04" for 100 Year event
Inflow = 1.72 cfs @ 12.08 hrs, Volume= 6,230 cf
Outflow = 1.72 cfs @ 12.08 hrs, Volume= 6,230 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.72 cfs @ 12.08 hrs, Volume= 6,230 cf
Routed to Pond MH1 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3

Peak Elev= 274.47' @ 12.53 hrs
Flood Elev= 274.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.20'	12.0" Round Culvert L= 84.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.20' / 271.70' S= 0.0060 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.72 cfs @ 12.08 hrs HW=273.19' TW=272.80' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.72 cfs @ 2.77 fps)

Summary for Pond 2: Catch Basin

Inflow Area = 12,976 sf, 8.11% Impervious, Inflow Depth = 1.77" for 100 Year event
Inflow = 0.45 cfs @ 12.10 hrs, Volume= 1,912 cf
Outflow = 0.45 cfs @ 12.11 hrs, Volume= 1,912 cf, Atten= 0%, Lag= 0.2 min
Primary = 0.45 cfs @ 12.11 hrs, Volume= 1,912 cf
Routed to Pond MH27 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.15' @ 12.09 hrs
Flood Elev= 274.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.61'	12.0" Round Culvert L= 47.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.61' / 271.34' S= 0.0057 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.45 cfs @ 12.11 hrs HW=274.14' TW=274.13' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.45 cfs @ 0.57 fps)

Summary for Pond 3: Catch Basin

Inflow Area = 9,660 sf, 44.27% Impervious, Inflow Depth = 6.27" for 100 Year event
Inflow = 1.40 cfs @ 12.08 hrs, Volume= 5,044 cf
Outflow = 1.40 cfs @ 12.08 hrs, Volume= 5,044 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.40 cfs @ 12.08 hrs, Volume= 5,044 cf
Routed to Pond MH27 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.31' @ 12.09 hrs
Flood Elev= 274.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.74'	12.0" Round Culvert L= 77.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.74' / 271.34' S= 0.0052 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.40 cfs @ 12.08 hrs HW=274.30' TW=274.13' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.40 cfs @ 1.78 fps)

Summary for Pond 4A: Catch Basin

Inflow Area = 3,976 sf, 91.93% Impervious, Inflow Depth = 7.38" for 100 Year event
Inflow = 0.68 cfs @ 12.08 hrs, Volume= 2,445 cf
Outflow = 0.68 cfs @ 12.08 hrs, Volume= 2,445 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.68 cfs @ 12.08 hrs, Volume= 2,445 cf
Routed to Pond MH26 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.47' @ 12.52 hrs
Flood Elev= 275.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.19'	12.0" Round Culvert L= 7.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.19' / 272.12' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.68 cfs @ 12.08 hrs HW=273.43' TW=273.40' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.68 cfs @ 0.86 fps)

Summary for Pond 4B: Catch Basin

Inflow Area = 10,218 sf, 73.42% Impervious, Inflow Depth = 6.14" for 100 Year event
Inflow = 1.43 cfs @ 12.08 hrs, Volume= 5,228 cf
Outflow = 1.43 cfs @ 12.08 hrs, Volume= 5,228 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.43 cfs @ 12.08 hrs, Volume= 5,228 cf
Routed to Pond MH26 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.48' @ 12.52 hrs
Flood Elev= 275.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.40'	12.0" Round Culvert L= 27.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.40' / 272.13' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.43 cfs @ 12.08 hrs HW=273.55' TW=273.41' (Dynamic Tailwater)
1=Culvert (Inlet Controls 1.43 cfs @ 1.82 fps)

Summary for Pond 4C: Catch Basin

Inflow Area = 10,741 sf, 84.88% Impervious, Inflow Depth = 6.91" for 100 Year event
Inflow = 1.71 cfs @ 12.08 hrs, Volume= 6,183 cf
Outflow = 1.71 cfs @ 12.08 hrs, Volume= 6,183 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.71 cfs @ 12.08 hrs, Volume= 6,183 cf
Routed to Pond MH30 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.50' @ 12.52 hrs
Flood Elev= 275.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.25'	12.0" Round Culvert L= 61.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.25' / 272.78' S= 0.0077 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.70 cfs @ 12.08 hrs HW=274.28' TW=273.98' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.70 cfs @ 2.62 fps)

Summary for Pond 4D: Catch Basin

Inflow Area = 10,203 sf, 79.98% Impervious, Inflow Depth = 6.58" for 100 Year event
Inflow = 1.54 cfs @ 12.08 hrs, Volume= 5,594 cf
Outflow = 1.54 cfs @ 12.08 hrs, Volume= 5,594 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.54 cfs @ 12.08 hrs, Volume= 5,594 cf
Routed to Pond MH30 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.50' @ 12.52 hrs
Flood Elev= 275.70'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.16'	12.0" Round Culvert L= 38.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.16' / 272.78' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.53 cfs @ 12.08 hrs HW=274.18' TW=273.98' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.53 cfs @ 2.37 fps)

Summary for Pond 5A: Catch Basin

Inflow Area = 10,459 sf, 93.11% Impervious, Inflow Depth = 7.46" for 100 Year event
Inflow = 1.81 cfs @ 12.08 hrs, Volume= 6,501 cf
Outflow = 1.81 cfs @ 12.08 hrs, Volume= 6,501 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.81 cfs @ 12.08 hrs, Volume= 6,501 cf
Routed to Pond MH28 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.91' @ 12.45 hrs
Flood Elev= 276.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.47'	12.0" Round Culvert L= 28.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.47' / 273.19' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.69 cfs @ 12.08 hrs HW=274.44' TW=274.20' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.69 cfs @ 2.74 fps)

Summary for Pond 5B: Catch Basin

Inflow Area = 3,865 sf, 95.02% Impervious, Inflow Depth = 7.59" for 100 Year event
Inflow = 0.68 cfs @ 12.08 hrs, Volume= 2,444 cf
Outflow = 0.68 cfs @ 12.08 hrs, Volume= 2,444 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.68 cfs @ 12.08 hrs, Volume= 2,444 cf
Routed to Pond MH28 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.90' @ 12.46 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.64'	12.0" Round Culvert L= 45.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.64' / 273.19' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.58 cfs @ 12.08 hrs HW=274.30' TW=274.20' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.58 cfs @ 1.51 fps)

Summary for Pond 5C: Catch Basin

Inflow Area = 2,619 sf, 91.40% Impervious, Inflow Depth = 7.34" for 100 Year event
Inflow = 0.44 cfs @ 12.08 hrs, Volume= 1,603 cf
Outflow = 0.44 cfs @ 12.08 hrs, Volume= 1,603 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.44 cfs @ 12.08 hrs, Volume= 1,603 cf
Routed to Pond MH31 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.90' @ 12.46 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.90'	12.0" Round Culvert L= 41.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.90' / 273.49' S= 0.0100 ' /' Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.44 cfs @ 12.08 hrs HW=274.36' TW=274.19' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.44 cfs @ 1.86 fps)

Summary for Pond 6: Catch Basin

Inflow Area = 12,631 sf, 77.41% Impervious, Inflow Depth = 6.41" for 100 Year event
Inflow = 1.86 cfs @ 12.08 hrs, Volume= 6,744 cf
Outflow = 1.86 cfs @ 12.08 hrs, Volume= 6,744 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.86 cfs @ 12.08 hrs, Volume= 6,744 cf
Routed to Pond MH7 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.90' @ 12.45 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.76'	12.0" Round Culvert L= 26.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.76' / 272.50' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.85 cfs @ 12.08 hrs HW=273.96' TW=273.72' (Dynamic Tailwater)
1=Culvert (Inlet Controls 1.85 cfs @ 2.36 fps)

Summary for Pond 7A: Catch Basin

Inflow Area = 7,101 sf, 79.91% Impervious, Inflow Depth = 6.58" for 100 Year event
Inflow = 1.07 cfs @ 12.08 hrs, Volume= 3,891 cf
Outflow = 1.07 cfs @ 12.08 hrs, Volume= 3,891 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.07 cfs @ 12.08 hrs, Volume= 3,891 cf
Routed to Pond MH8 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.90' @ 12.46 hrs
Flood Elev= 276.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.92'	12.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.92' / 272.82' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.60 cfs @ 12.08 hrs HW=274.00' TW=273.98' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.60 cfs @ 0.77 fps)

Summary for Pond 7B: Catch Basin

Inflow Area = 11,340 sf, 75.16% Impervious, Inflow Depth = 6.26" for 100 Year event
Inflow = 1.62 cfs @ 12.08 hrs, Volume= 5,913 cf
Outflow = 1.62 cfs @ 12.08 hrs, Volume= 5,913 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.62 cfs @ 12.08 hrs, Volume= 5,913 cf
Routed to Pond MH8 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.91' @ 12.46 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.22'	12.0" Round Culvert L= 40.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.22' / 272.82' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.44 cfs @ 12.08 hrs HW=274.18' TW=273.98' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.44 cfs @ 2.37 fps)

Summary for Pond 8: Catch Basin

Inflow Area = 10,991 sf, 81.26% Impervious, Inflow Depth = 6.67" for 100 Year event
Inflow = 1.68 cfs @ 12.08 hrs, Volume= 6,105 cf
Outflow = 1.68 cfs @ 12.08 hrs, Volume= 6,105 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.68 cfs @ 12.08 hrs, Volume= 6,105 cf
Routed to Pond MH33 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.87' @ 12.26 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.47'	12.0" Round Culvert L= 27.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.47' / 273.20' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.15 cfs @ 12.08 hrs HW=275.32' TW=275.22' (Dynamic Tailwater)
1=Culvert (Inlet Controls 1.15 cfs @ 1.47 fps)

Summary for Pond 9A: Catch Basin

Inflow Area = 11,743 sf, 81.25% Impervious, Inflow Depth = 6.66" for 100 Year event
Inflow = 1.80 cfs @ 12.08 hrs, Volume= 6,522 cf
Outflow = 1.80 cfs @ 12.08 hrs, Volume= 6,522 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.80 cfs @ 12.08 hrs, Volume= 6,522 cf
Routed to Pond MH32 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.89' @ 12.26 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.58'	12.0" Round Culvert L= 14.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.58' / 273.51' S= 0.0050 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.79 cfs @ 12.08 hrs HW=275.42' TW=275.19' (Dynamic Tailwater)
1=Culvert (Inlet Controls 1.79 cfs @ 2.28 fps)

Summary for Pond 9B: Catch Basin

Inflow Area = 14,736 sf, 67.40% Impervious, Inflow Depth = 5.74" for 100 Year event
Inflow = 1.92 cfs @ 12.09 hrs, Volume= 7,045 cf
Outflow = 1.92 cfs @ 12.09 hrs, Volume= 7,045 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.92 cfs @ 12.09 hrs, Volume= 7,045 cf
Routed to Pond MH33 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.90' @ 12.26 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.58'	12.0" Round Culvert L= 64.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.58' / 273.20' S= 0.0059 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.55 cfs @ 12.09 hrs HW=275.43' TW=275.23' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.55 cfs @ 1.97 fps)

Summary for Pond 9C: Catch Basin

Inflow Area = 3,708 sf, 100.00% Impervious, Inflow Depth = 7.92" for 100 Year event
Inflow = 0.68 cfs @ 12.08 hrs, Volume= 2,448 cf
Outflow = 0.68 cfs @ 12.08 hrs, Volume= 2,448 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.68 cfs @ 12.08 hrs, Volume= 2,448 cf
Routed to Pond MH12 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.80' @ 12.27 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.24'	12.0" Round Culvert L= 7.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.24' / 273.16' S= 0.0114 '/ Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=274.94' TW=275.02' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)

Summary for Pond 9D: Catch Basin

Inflow Area = 1,543 sf, 58.43% Impervious, Inflow Depth = 5.14" for 100 Year event
Inflow = 0.18 cfs @ 12.09 hrs, Volume= 660 cf
Outflow = 0.18 cfs @ 12.09 hrs, Volume= 660 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.18 cfs @ 12.09 hrs, Volume= 660 cf
Routed to Pond MH32 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.85' @ 12.27 hrs
Flood Elev= 276.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.95'	12.0" Round Culvert L= 49.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.95' / 273.51' S= 0.0090 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.17 cfs @ 12.09 hrs HW=275.22' TW=275.22' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.17 cfs @ 0.21 fps)

Summary for Pond 10A: Catch Basin

Inflow Area = 2,889 sf, 81.34% Impervious, Inflow Depth = 6.67" for 100 Year event
Inflow = 0.44 cfs @ 12.08 hrs, Volume= 1,606 cf
Outflow = 0.44 cfs @ 12.08 hrs, Volume= 1,606 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.44 cfs @ 12.08 hrs, Volume= 1,606 cf
Routed to Pond MH15 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.31' @ 12.55 hrs
Flood Elev= 276.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.41'	12.0" Round Culvert L= 87.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.41' / 270.54' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=272.18' TW=272.22' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)

Summary for Pond 10B: Catch Basin

Inflow Area = 10,414 sf, 86.69% Impervious, Inflow Depth = 7.03" for 100 Year event
Inflow = 1.69 cfs @ 12.08 hrs, Volume= 6,100 cf
Outflow = 1.69 cfs @ 12.08 hrs, Volume= 6,100 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.69 cfs @ 12.08 hrs, Volume= 6,100 cf
Routed to Pond MH15 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.31' @ 12.55 hrs
Flood Elev= 276.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.69'	12.0" Round Culvert L= 15.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.69' / 270.54' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.24 cfs @ 12.08 hrs HW=272.33' TW=272.22' (Dynamic Tailwater)
1=Culvert (Inlet Controls 1.24 cfs @ 1.58 fps)

Summary for Pond 11: Catch Basin

Inflow Area = 11,505 sf, 85.23% Impervious, Inflow Depth = 6.93" for 100 Year event
Inflow = 1.84 cfs @ 12.08 hrs, Volume= 6,645 cf
Outflow = 1.84 cfs @ 12.08 hrs, Volume= 6,645 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.84 cfs @ 12.08 hrs, Volume= 6,645 cf
Routed to Pond MH17 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.30' @ 12.54 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.55'	12.0" Round Culvert L= 15.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.55' / 270.40' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.83 cfs @ 12.08 hrs HW=272.13' TW=271.90' (Dynamic Tailwater)
1=Culvert (Inlet Controls 1.83 cfs @ 2.34 fps)

Summary for Pond 12: Catch Basin

Inflow Area = 7,149 sf, 71.54% Impervious, Inflow Depth = 6.01" for 100 Year event
Inflow = 0.98 cfs @ 12.08 hrs, Volume= 3,583 cf
Outflow = 0.98 cfs @ 12.08 hrs, Volume= 3,583 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.98 cfs @ 12.08 hrs, Volume= 3,583 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.32' @ 12.54 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.74'	12.0" Round Culvert L= 14.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.74' / 270.60' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.61 cfs @ 12.08 hrs HW=272.66' TW=272.63' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.61 cfs @ 0.77 fps)

Summary for Pond 13: Catch Basin

Inflow Area = 4,141 sf, 88.54% Impervious, Inflow Depth = 7.15" for 100 Year event
Inflow = 0.68 cfs @ 12.08 hrs, Volume= 2,468 cf
Outflow = 0.68 cfs @ 12.08 hrs, Volume= 2,468 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.68 cfs @ 12.08 hrs, Volume= 2,468 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.32' @ 12.54 hrs
Flood Elev= 275.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.79'	12.0" Round Culvert L= 87.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.79' / 270.92' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.48 cfs @ 12.08 hrs HW=272.67' TW=272.63' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.48 cfs @ 0.87 fps)

Summary for Pond 14: Catch Basin

Inflow Area = 6,167 sf, 87.00% Impervious, Inflow Depth = 7.05" for 100 Year event
Inflow = 1.00 cfs @ 12.08 hrs, Volume= 3,623 cf
Outflow = 1.00 cfs @ 12.08 hrs, Volume= 3,623 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.00 cfs @ 12.08 hrs, Volume= 3,623 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.32' @ 12.54 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.25'	12.0" Round Culvert L= 33.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.25' / 270.92' S= 0.0100 ' /' Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.64 cfs @ 12.08 hrs HW=272.66' TW=272.63' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.64 cfs @ 0.82 fps)

Summary for Pond 15: Catch Basin

Inflow Area = 20,154 sf, 44.65% Impervious, Inflow Depth = 4.21" for 100 Year event
Inflow = 1.88 cfs @ 12.09 hrs, Volume= 7,078 cf
Outflow = 1.88 cfs @ 12.09 hrs, Volume= 7,078 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.88 cfs @ 12.09 hrs, Volume= 7,078 cf
Routed to Pond MH23 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.36' @ 12.53 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.42'	12.0" Round Culvert L= 19.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.42' / 272.23' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.88 cfs @ 12.09 hrs HW=273.78' TW=273.53' (Dynamic Tailwater)
1=Culvert (Inlet Controls 1.88 cfs @ 2.39 fps)

Summary for Pond 16: Catch Basin

Inflow Area = 20,584 sf, 35.58% Impervious, Inflow Depth = 3.61" for 100 Year event
Inflow = 1.62 cfs @ 12.09 hrs, Volume= 6,187 cf
Outflow = 1.62 cfs @ 12.09 hrs, Volume= 6,187 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.62 cfs @ 12.09 hrs, Volume= 6,187 cf
Routed to Pond MH24 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.20' @ 12.09 hrs
Flood Elev= 277.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	274.29'	12.0" Round Culvert L= 19.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 274.29' / 274.10' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.62 cfs @ 12.09 hrs HW=275.20' TW=274.97' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.62 cfs @ 2.81 fps)

Summary for Pond 17: Catch Basin

Inflow Area = 36,035 sf, 27.88% Impervious, Inflow Depth = 3.09" for 100 Year event
Inflow = 2.38 cfs @ 12.09 hrs, Volume= 9,283 cf
Outflow = 2.38 cfs @ 12.09 hrs, Volume= 9,283 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.38 cfs @ 12.09 hrs, Volume= 9,283 cf
Routed to Pond MH24 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.84' @ 12.09 hrs
Flood Elev= 277.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	274.97'	15.0" Round Culvert L= 143.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 274.97' / 274.10' S= 0.0061 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.38 cfs @ 12.09 hrs HW=275.84' TW=274.97' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.38 cfs @ 3.65 fps)

Summary for Pond LP-1: Low Point - MAHW

Inflow Area = 210,774 sf, 10.22% Impervious, Inflow Depth = 1.11" for 100 Year event
Inflow = 6.90 cfs @ 12.14 hrs, Volume= 19,447 cf
Outflow = 6.88 cfs @ 12.15 hrs, Volume= 19,352 cf, Atten= 0%, Lag= 0.5 min
Primary = 6.88 cfs @ 12.15 hrs, Volume= 19,352 cf
Routed to Link SP1 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 268.73' @ 12.15 hrs Surf.Area= 319 sf Storage= 425 cf

Plug-Flow detention time= 6.3 min calculated for 19,347 cf (99% of inflow)
Center-of-Mass det. time= 3.9 min (794.7 - 790.8)

Volume	Invert	Avail.Storage	Storage Description		
#1	267.00'	902 cf	Custom Stage Data (Irregular) Listed below (Recalc)		
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
267.00	175	160.0	0	0	175
268.00	258	168.0	215	215	441
269.00	344	172.0	300	515	642
270.00	431	176.0	387	902	848

Device	Routing	Invert	Outlet Devices
#1	Primary	267.93'	24.0" Round RCP_Round 24" L= 59.0' RCP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 267.72' / 267.93' S= -0.0036 '/' Cc= 0.900 n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf

#2 Primary 267.48' **24.0" Round RCP_Round 24"**
L= 59.0' RCP, square edge headwall, Ke= 0.500
Inlet / Outlet Invert= 267.48' / 267.48' S= 0.0000 '/ S= 0.0000 ' Cc= 0.900
n= 0.014 Concrete pipe, finished, Flow Area= 3.14 sf

Primary OutFlow Max=6.88 cfs @ 12.15 hrs HW=268.73' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Barrel Controls 2.71 cfs @ 2.49 fps)
2=RCP_Round 24" (Barrel Controls 4.17 cfs @ 2.89 fps)

Summary for Pond LP-2: Low Point - MAHW

Inflow Area = 236,963 sf, 10.14% Impervious, Inflow Depth = 1.15" for 100 Year event
Inflow = 7.92 cfs @ 12.15 hrs, Volume= 22,678 cf
Outflow = 7.42 cfs @ 12.18 hrs, Volume= 22,678 cf, Atten= 6%, Lag= 2.2 min
Primary = 7.42 cfs @ 12.18 hrs, Volume= 22,678 cf
Routed to Link SP2 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 267.86' @ 12.18 hrs Surf.Area= 1,610 sf Storage= 1,302 cf

Plug-Flow detention time= 4.3 min calculated for 22,672 cf (100% of inflow)
Center-of-Mass det. time= 4.3 min (800.5 - 796.2)

Volume	Invert	Avail.Storage	Storage Description			
#1	266.85'	5,666 cf	Custom Stage Data (Irregular) Listed below (Recalc)			
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
266.85	0	0.0	0	0	0	
267.00	1,276	387.0	64	64	11,918	
268.00	1,668	396.0	1,468	1,531	12,602	
269.00	2,069	404.0	1,865	3,396	13,250	
270.00	2,477	412.0	2,270	5,666	13,910	

Device	Routing	Invert	Outlet Devices			
#1	Primary	266.85'	24.0" Round RCP_Round 24" L= 30.0' RCP, rounded edge headwall, Ke= 0.100 Inlet / Outlet Invert= 266.37' / 266.85' S= -0.0160 '/ S= -0.0160 ' Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 3.14 sf			

Primary OutFlow Max=7.41 cfs @ 12.18 hrs HW=267.86' TW=0.00' (Dynamic Tailwater)
1=RCP_Round 24" (Inlet Controls 7.41 cfs @ 4.66 fps)

Summary for Pond MH1: Manhole

Inflow Area = 10,622 sf, 86.84% Impervious, Inflow Depth = 7.04" for 100 Year event
Inflow = 1.72 cfs @ 12.08 hrs, Volume= 6,230 cf
Outflow = 1.72 cfs @ 12.08 hrs, Volume= 6,230 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.72 cfs @ 12.08 hrs, Volume= 6,230 cf
Routed to Pond UIS1 : Underground Chamber #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.46' @ 12.53 hrs
Flood Elev= 275.40'

Device	Routing	Invert	Outlet Devices			
#1	Primary	271.70'	24.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.70' / 271.70' S= 0.0000 '/ S= 0.0000 ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf			

Primary OutFlow Max=1.73 cfs @ 12.08 hrs HW=272.80' TW=272.75' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.73 cfs @ 1.41 fps)

Summary for Pond MH11: Manhole

Inflow Area = 25,727 sf, 73.32% Impervious, Inflow Depth = 6.13" for 100 Year event
Inflow = 3.61 cfs @ 12.08 hrs, Volume= 13,150 cf
Outflow = 3.61 cfs @ 12.08 hrs, Volume= 13,150 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.61 cfs @ 12.08 hrs, Volume= 13,150 cf
Routed to Pond UIS4 : Underground Chamber #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.77' @ 12.27 hrs
Flood Elev= 276.40'

Device	Routing	Invert	Outlet Devices			
#1	Primary	273.00'	24.0" Round Culvert L= 4.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.00' / 273.00' S= 0.0000 '/ S= 0.0000 ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf			

Primary OutFlow Max=3.61 cfs @ 12.08 hrs HW=274.86' TW=274.80' (Dynamic Tailwater)
1=Culvert (Inlet Controls 3.61 cfs @ 1.19 fps)

Summary for Pond MH12: Manhole

Inflow Area = 16,994 sf, 83.27% Impervious, Inflow Depth = 6.80" for 100 Year event
Inflow = 2.66 cfs @ 12.08 hrs, Volume= 9,630 cf
Outflow = 2.66 cfs @ 12.08 hrs, Volume= 9,630 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.66 cfs @ 12.08 hrs, Volume= 9,630 cf
Routed to Pond MH13 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.80' @ 12.26 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices			
#1	Primary	273.16'	15.0" Round Culvert L= 28.0' CPP, square edge headwall, Ke= 0.500			

Inlet / Outlet Invert= 273.16' / 273.00' S= 0.0057 '/' Cc= 0.900
n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.65 cfs @ 12.08 hrs HW=275.03' TW=274.82' (Dynamic Tailwater)
1=Culvert (Inlet Controls 2.65 cfs @ 2.16 fps)

Summary for Pond MH13: Manhole

Inflow Area = 16,994 sf, 83.27% Impervious, Inflow Depth = 6.80" for 100 Year event
Inflow = 2.66 cfs @ 12.08 hrs, Volume= 9,630 cf
Outflow = 2.66 cfs @ 12.08 hrs, Volume= 9,630 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.66 cfs @ 12.08 hrs, Volume= 9,630 cf
Routed to Pond UIS4 : Underground Chamber #4

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.76' @ 12.27 hrs
Flood Elev= 277.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.00'	24.0" Round Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.00' / 273.00' S= 0.0000 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=2.66 cfs @ 12.08 hrs HW=274.82' TW=274.79' (Dynamic Tailwater)
1=Culvert (Inlet Controls 2.66 cfs @ 0.89 fps)

Summary for Pond MH15: Manhole

Inflow Area = 13,302 sf, 85.53% Impervious, Inflow Depth = 6.95" for 100 Year event
Inflow = 2.13 cfs @ 12.08 hrs, Volume= 7,705 cf
Outflow = 2.13 cfs @ 12.08 hrs, Volume= 7,705 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.13 cfs @ 12.08 hrs, Volume= 7,705 cf
Routed to Pond MH16 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.31' @ 12.54 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.44'	12.0" Round Culvert L= 14.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.44' / 270.30' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=2.13 cfs @ 12.08 hrs HW=272.22' TW=271.90' (Dynamic Tailwater)
1=Culvert (Inlet Controls 2.13 cfs @ 2.71 fps)

Summary for Pond MH16: Manhole

Inflow Area = 13,302 sf, 85.53% Impervious, Inflow Depth = 6.95" for 100 Year event
Inflow = 2.13 cfs @ 12.08 hrs, Volume= 7,705 cf
Outflow = 2.13 cfs @ 12.08 hrs, Volume= 7,705 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.13 cfs @ 12.08 hrs, Volume= 7,705 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.30' @ 12.55 hrs
Flood Elev= 276.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.30'	24.0" Round Culvert L= 5.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.30' / 270.30' S= 0.0000 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=2.13 cfs @ 12.08 hrs HW=271.90' TW=271.87' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.13 cfs @ 1.08 fps)

Summary for Pond MH17: Manhole

Inflow Area = 11,505 sf, 85.23% Impervious, Inflow Depth = 6.93" for 100 Year event
Inflow = 1.84 cfs @ 12.08 hrs, Volume= 6,645 cf
Outflow = 1.84 cfs @ 12.08 hrs, Volume= 6,640 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.84 cfs @ 12.08 hrs, Volume= 6,640 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.30' @ 12.55 hrs
Flood Elev= 277.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.30'	24.0" Round Culvert L= 7.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.30' / 270.30' S= 0.0000 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.84 cfs @ 12.08 hrs HW=271.90' TW=271.87' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.84 cfs @ 0.94 fps)

Summary for Pond MH19: Manhole

Inflow Area = 157,891 sf, 63.68% Impervious, Inflow Depth = 0.19" for 100 Year event
Inflow = 1.84 cfs @ 12.55 hrs, Volume= 2,546 cf
Outflow = 1.84 cfs @ 12.55 hrs, Volume= 2,546 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.84 cfs @ 12.55 hrs, Volume= 2,546 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 270.54' @ 12.55 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	269.87'	18.0" Round Culvert L= 51.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 269.87' / 269.52' S= 0.0069 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=1.84 cfs @ 12.55 hrs HW=270.54' TW=0.00' (Dynamic Tailwater)
1=Culvert (Barrel Controls 1.84 cfs @ 3.53 fps)

Summary for Pond MH21: Manhole

Inflow Area = 94,229 sf, 43.00% Impervious, Inflow Depth = 4.10" for 100 Year event
Inflow = 8.54 cfs @ 12.09 hrs, Volume= 32,222 cf
Outflow = 8.54 cfs @ 12.09 hrs, Volume= 32,222 cf, Atten= 0%, Lag= 0.0 min
Primary = 8.54 cfs @ 12.09 hrs, Volume= 32,222 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.31' @ 12.54 hrs
Flood Elev= 277.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.50'	24.0" Round Culvert L= 20.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.50' / 270.30' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=8.53 cfs @ 12.09 hrs HW=272.29' TW=271.92' (Dynamic Tailwater)
1=Culvert (Outlet Controls 8.53 cfs @ 3.80 fps)

Summary for Pond MH22: Manhole

Inflow Area = 94,229 sf, 43.00% Impervious, Inflow Depth = 4.10" for 100 Year event
Inflow = 8.54 cfs @ 12.09 hrs, Volume= 32,222 cf
Outflow = 8.54 cfs @ 12.09 hrs, Volume= 32,222 cf, Atten= 0%, Lag= 0.0 min
Primary = 8.54 cfs @ 12.09 hrs, Volume= 32,222 cf
Routed to Pond MH21 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.32' @ 12.53 hrs
Flood Elev= 277.40'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.82'	24.0" Round Culvert L= 22.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.82' / 270.60' S= 0.0100 '/ Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=8.53 cfs @ 12.09 hrs HW=272.65' TW=272.29' (Dynamic Tailwater)
1=Culvert (Outlet Controls 8.53 cfs @ 3.71 fps)

Summary for Pond MH23: Manhole

Inflow Area = 76,773 sf, 34.35% Impervious, Inflow Depth = 3.52" for 100 Year event
Inflow = 5.88 cfs @ 12.09 hrs, Volume= 22,548 cf
Outflow = 5.88 cfs @ 12.09 hrs, Volume= 22,548 cf, Atten= 0%, Lag= 0.0 min
Primary = 5.88 cfs @ 12.09 hrs, Volume= 22,548 cf
Routed to Pond MH22 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.34' @ 12.53 hrs
Flood Elev= 277.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.13'	18.0" Round Culvert L= 121.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.13' / 270.92' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=5.73 cfs @ 12.09 hrs HW=273.53' TW=272.66' (Dynamic Tailwater)
1=Culvert (Outlet Controls 5.73 cfs @ 4.33 fps)

Summary for Pond MH24: Manhole

Inflow Area = 56,619 sf, 30.68% Impervious, Inflow Depth = 3.28" for 100 Year event
Inflow = 4.00 cfs @ 12.09 hrs, Volume= 15,470 cf
Outflow = 4.00 cfs @ 12.09 hrs, Volume= 15,470 cf, Atten= 0%, Lag= 0.0 min
Primary = 4.00 cfs @ 12.09 hrs, Volume= 15,470 cf
Routed to Pond MH23 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.97' @ 12.09 hrs
Flood Elev= 278.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.99'	18.0" Round Culvert L= 176.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.99' / 272.23' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=3.99 cfs @ 12.09 hrs HW=274.97' TW=273.53' (Dynamic Tailwater)
1=Culvert (Outlet Controls 3.99 cfs @ 4.64 fps)

Summary for Pond MH26: Manhole

Inflow Area = 35,138 sf, 80.92% Impervious, Inflow Depth = 6.64" for 100 Year event
Inflow = 5.36 cfs @ 12.08 hrs, Volume= 19,450 cf
Outflow = 5.36 cfs @ 12.08 hrs, Volume= 19,450 cf, Atten= 0%, Lag= 0.0 min
Primary = 5.36 cfs @ 12.08 hrs, Volume= 19,450 cf
Routed to Pond UIS1 : Underground Chamber #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.47' @ 12.52 hrs
Flood Elev= 275.10'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.13'	18.0" Round Culvert L= 43.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.13' / 271.70' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.77 sf

Primary OutFlow Max=5.36 cfs @ 12.08 hrs HW=273.40' TW=272.75' (Dynamic Tailwater)
1=Culvert (Outlet Controls 5.36 cfs @ 4.51 fps)

Summary for Pond MH27: Manhole

Inflow Area = 22,636 sf, 23.54% Impervious, Inflow Depth = 3.69" for 100 Year event
Inflow = 1.84 cfs @ 12.09 hrs, Volume= 6,956 cf
Outflow = 1.84 cfs @ 12.09 hrs, Volume= 6,956 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.84 cfs @ 12.09 hrs, Volume= 6,956 cf
Routed to Pond UIS2 : Underground Chamber #2

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.13' @ 12.09 hrs
Flood Elev= 275.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.34'	15.0" Round Culvert L= 8.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.34' / 272.30' S= 0.0050 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=1.84 cfs @ 12.09 hrs HW=274.13' TW=274.04' (Dynamic Tailwater)
1=Culvert (Inlet Controls 1.84 cfs @ 1.50 fps)

Summary for Pond MH28: Manhole

Inflow Area = 16,944 sf, 93.28% Impervious, Inflow Depth = 7.47" for 100 Year event
Inflow = 2.93 cfs @ 12.08 hrs, Volume= 10,548 cf
Outflow = 2.93 cfs @ 12.08 hrs, Volume= 10,548 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.93 cfs @ 12.08 hrs, Volume= 10,548 cf
Routed to Pond MH6 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.90' @ 12.45 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.09'	15.0" Round Culvert L= 59.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.09' / 272.50' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.93 cfs @ 12.08 hrs HW=274.20' TW=273.77' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.93 cfs @ 3.37 fps)

Summary for Pond MH29: Manhole

Inflow Area = 20,944 sf, 82.49% Impervious, Inflow Depth = 6.75" for 100 Year event
Inflow = 3.25 cfs @ 12.08 hrs, Volume= 11,777 cf
Outflow = 3.25 cfs @ 12.08 hrs, Volume= 11,777 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.25 cfs @ 12.08 hrs, Volume= 11,777 cf
Routed to Pond MH26 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.48' @ 12.52 hrs
Flood Elev= 275.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.24'	15.0" Round Culvert L= 11.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.24' / 272.13' S= 0.0100 '/ Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.25 cfs @ 12.08 hrs HW=273.70' TW=273.40' (Dynamic Tailwater)
1=Culvert (Inlet Controls 3.25 cfs @ 2.64 fps)

Summary for Pond MH30: Manhole

Inflow Area = 20,944 sf, 82.49% Impervious, Inflow Depth = 6.75" for 100 Year event
Inflow = 3.25 cfs @ 12.08 hrs, Volume= 11,777 cf
Outflow = 3.25 cfs @ 12.08 hrs, Volume= 11,777 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.25 cfs @ 12.08 hrs, Volume= 11,777 cf
Routed to Pond MH29 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.49' @ 12.52 hrs
Flood Elev= 275.35'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.68'	15.0" Round Culvert L= 34.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.68' / 272.34' S= 0.0100 '/ Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.06 cfs @ 12.08 hrs HW=273.98' TW=273.70' (Dynamic Tailwater)
1=Culvert (Outlet Controls 3.06 cfs @ 2.98 fps)

Summary for Pond MH31: Manhole

Inflow Area = 2,619 sf, 91.40% Impervious, Inflow Depth = 7.34" for 100 Year event
Inflow = 0.44 cfs @ 12.08 hrs, Volume= 1,603 cf
Outflow = 0.44 cfs @ 12.08 hrs, Volume= 1,603 cf, Atten= 0%, Lag= 0.0 min
Primary = 0.44 cfs @ 12.08 hrs, Volume= 1,603 cf
Routed to Pond MH28 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.90' @ 12.46 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.39'	12.0" Round Culvert L= 20.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.39' / 273.19' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.00 cfs @ 12.08 hrs HW=274.19' TW=274.20' (Dynamic Tailwater)
1=Culvert (Controls 0.00 cfs)

Summary for Pond MH32: Manhole

Inflow Area = 13,286 sf, 78.60% Impervious, Inflow Depth = 6.49" for 100 Year event
Inflow = 1.98 cfs @ 12.08 hrs, Volume= 7,182 cf
Outflow = 1.98 cfs @ 12.08 hrs, Volume= 7,182 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.98 cfs @ 12.08 hrs, Volume= 7,182 cf
Routed to Pond MH12 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.85' @ 12.27 hrs
Flood Elev= 276.30'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.41'	12.0" Round Culvert L= 48.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.41' / 273.16' S= 0.0052 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.55 cfs @ 12.08 hrs HW=275.20' TW=275.03' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.55 cfs @ 1.97 fps)

Summary for Pond MH33: Manhole

Inflow Area = 25,727 sf, 73.32% Impervious, Inflow Depth = 6.13" for 100 Year event
Inflow = 3.61 cfs @ 12.08 hrs, Volume= 13,150 cf
Outflow = 3.61 cfs @ 12.08 hrs, Volume= 13,150 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.61 cfs @ 12.08 hrs, Volume= 13,150 cf
Routed to Pond MH11 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.84' @ 12.26 hrs
Flood Elev= 277.05'

Device	Routing	Invert	Outlet Devices
#1	Primary	273.20'	15.0" Round Culvert L= 20.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.20' / 273.00' S= 0.0100 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.60 cfs @ 12.08 hrs HW=275.23' TW=274.86' (Dynamic Tailwater)
1=Culvert (Inlet Controls 3.60 cfs @ 2.93 fps)

Summary for Pond MH6: Manhole

Inflow Area = 16,944 sf, 93.28% Impervious, Inflow Depth = 7.47" for 100 Year event
Inflow = 2.93 cfs @ 12.08 hrs, Volume= 10,548 cf
Outflow = 2.93 cfs @ 12.08 hrs, Volume= 10,548 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.93 cfs @ 12.08 hrs, Volume= 10,548 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.88' @ 12.46 hrs
Flood Elev= 277.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.50'	24.0" Round Culvert L= 6.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.50' / 272.50' S= 0.0000 ' /' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=2.93 cfs @ 12.08 hrs HW=273.77' TW=273.67' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.93 cfs @ 1.99 fps)

Summary for Pond MH7: Manhole

Inflow Area = 12,631 sf, 77.41% Impervious, Inflow Depth = 6.41" for 100 Year event
Inflow = 1.86 cfs @ 12.08 hrs, Volume= 6,744 cf
Outflow = 1.86 cfs @ 12.08 hrs, Volume= 6,744 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.86 cfs @ 12.08 hrs, Volume= 6,744 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.88' @ 12.46 hrs
Flood Elev= 276.80'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.50'	24.0" Round Culvert L= 6.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.50' / 272.50' S= 0.0000 '/ S= 0.0000 ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=1.86 cfs @ 12.08 hrs HW=273.72' TW=273.68' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.86 cfs @ 1.32 fps)

Summary for Pond MH8: Manhole

Inflow Area = 18,441 sf, 76.99% Impervious, Inflow Depth = 6.38" for 100 Year event
Inflow = 2.70 cfs @ 12.08 hrs, Volume= 9,803 cf
Outflow = 2.70 cfs @ 12.08 hrs, Volume= 9,803 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.70 cfs @ 12.08 hrs, Volume= 9,803 cf
Routed to Pond MH9 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.89' @ 12.45 hrs
Flood Elev= 276.60'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.72'	15.0" Round Culvert L= 22.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.72' / 272.50' S= 0.0100 '/ S= 0.0100 ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=2.72 cfs @ 12.08 hrs HW=273.98' TW=273.76' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.72 cfs @ 2.74 fps)

Summary for Pond MH9: Manhole

Inflow Area = 18,441 sf, 76.99% Impervious, Inflow Depth = 6.38" for 100 Year event
Inflow = 2.70 cfs @ 12.08 hrs, Volume= 9,803 cf
Outflow = 2.70 cfs @ 12.08 hrs, Volume= 9,803 cf, Atten= 0%, Lag= 0.0 min
Primary = 2.70 cfs @ 12.08 hrs, Volume= 9,803 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.88' @ 12.46 hrs
Flood Elev= 277.20'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.50'	24.0" Round Culvert L= 8.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.50' / 272.50' S= 0.0000 '/ S= 0.0000 ' Cc= 0.900

n= 0.012 Corrugated PP, smooth interior, Flow Area= 3.14 sf

Primary OutFlow Max=2.70 cfs @ 12.08 hrs HW=273.76' TW=273.68' (Dynamic Tailwater)
1=Culvert (Outlet Controls 2.70 cfs @ 1.84 fps)

Summary for Pond RD1: Roof Drain

Inflow Area = 7,878 sf, 100.00% Impervious, Inflow Depth = 7.92" for 100 Year event
Inflow = 1.45 cfs @ 12.08 hrs, Volume= 5,200 cf
Outflow = 1.45 cfs @ 12.08 hrs, Volume= 5,200 cf, Atten= 0%, Lag= 0.0 min
Primary = 1.45 cfs @ 12.08 hrs, Volume= 5,200 cf
Routed to Pond UIS1 : Underground Chamber #1

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.46' @ 12.53 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	271.91'	12.0" Round Culvert L= 21.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.91' / 271.70' S= 0.0100 '/ S= 0.0100 ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.45 cfs @ 12.08 hrs HW=272.90' TW=272.74' (Dynamic Tailwater)
1=Culvert (Outlet Controls 1.45 cfs @ 2.31 fps)

Summary for Pond RD2: Roof Drain

Inflow Area = 18,290 sf, 100.00% Impervious, Inflow Depth = 7.92" for 100 Year event
Inflow = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf
Outflow = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.93' @ 12.43 hrs
Flood Elev= 277.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.81'	12.0" Round Culvert L= 31.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.81' / 272.50' S= 0.0100 '/ S= 0.0100 ' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.36 cfs @ 12.08 hrs HW=274.46' TW=273.67' (Dynamic Tailwater)
1=Culvert (Inlet Controls 3.36 cfs @ 4.28 fps)

Summary for Pond RD3: Roof Drain

Inflow Area = 18,290 sf,100.00% Impervious, Inflow Depth = 7.92" for 100 Year event
Inflow = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf
Outflow = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf
Routed to Pond UIS3 : Underground Chamber #3

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.93' @ 12.43 hrs
Flood Elev= 278.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	272.65'	12.0" Round Culvert L= 14.5' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 272.65' / 272.50' S= 0.0103 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.36 cfs @ 12.08 hrs HW=274.46' TW=273.67' (Dynamic Tailwater)
1=Culvert (Inlet Controls 3.36 cfs @ 4.28 fps)

Summary for Pond RD4: Roof Drain

Inflow Area = 18,290 sf,100.00% Impervious, Inflow Depth = 7.92" for 100 Year event
Inflow = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf
Outflow = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.36 cfs @ 12.08 hrs, Volume= 12,072 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.32' @ 12.53 hrs
Flood Elev= 277.50'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.55'	12.0" Round Culvert L= 24.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.55' / 270.30' S= 0.0104 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=3.36 cfs @ 12.08 hrs HW=272.65' TW=271.87' (Dynamic Tailwater)
1=Culvert (Inlet Controls 3.36 cfs @ 4.28 fps)

Summary for Pond RD5: Roof Drain

Inflow Area = 20,565 sf,100.00% Impervious, Inflow Depth = 7.92" for 100 Year event
Inflow = 3.78 cfs @ 12.08 hrs, Volume= 13,573 cf
Outflow = 3.78 cfs @ 12.08 hrs, Volume= 13,573 cf, Atten= 0%, Lag= 0.0 min
Primary = 3.78 cfs @ 12.08 hrs, Volume= 13,573 cf
Routed to Pond UIS5 : Underground Chamber #5

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.31' @ 12.54 hrs
Flood Elev= 278.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	270.48'	15.0" Round Culvert L= 18.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 270.48' / 270.30' S= 0.0100 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 1.23 sf

Primary OutFlow Max=3.78 cfs @ 12.08 hrs HW=272.27' TW=271.87' (Dynamic Tailwater)
1=Culvert (Inlet Controls 3.78 cfs @ 3.08 fps)

Summary for Pond TD1: Stone Trench Drain

Inflow Area = 180,624 sf, 8.74% Impervious, Inflow Depth = 1.81" for 100 Year event
Inflow = 5.49 cfs @ 12.14 hrs, Volume= 27,240 cf
Outflow = 5.44 cfs @ 12.16 hrs, Volume= 27,240 cf, Atten= 1%, Lag= 0.9 min
Discarded = 0.20 cfs @ 10.94 hrs, Volume= 11,376 cf
Primary = 5.24 cfs @ 12.16 hrs, Volume= 15,864 cf
Routed to Pond LP-1 : Low Point - MAHW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 272.67' @ 12.16 hrs Surf.Area= 1,050 sf Storage= 471 cf
Flood Elev= 272.90' Surf.Area= 1,050 sf Storage= 475 cf

Plug-Flow detention time= 1.5 min calculated for 27,232 cf (100% of inflow)
Center-of-Mass det. time= 1.5 min (849.4 - 847.9)

Volume	Invert	Avail.Storage	Storage Description
#1	271.20'	4 cf	3.50'W x 300.00'L x 1.70'H Prismatoid 1,785 cf Overall - 471 cf Embedded = 1,314 cf x 0.3% Voids
#2	271.70'	471 cf	12.0" Round Pipe Storage x 2 Inside #1 L= 300.0'
#3	272.90'	3,296 cf	Custom Stage Data (Irregular) Listed below (Recalc)
		3,771 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
272.90	0	0.0	0	0	0
273.00	2,670	657.2	89	89	34,370
274.00	3,775	679.8	3,207	3,296	36,866

Device	Routing	Invert	Outlet Devices
#1	Primary	271.70'	12.0" Round Culvert X 2.00 L= 60.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 271.70' / 270.90' S= 0.0133 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Discarded	271.20'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.20 cfs @ 10.94 hrs HW=271.23' (Free Discharge)
2=Exfiltration (Exfiltration Controls 0.20 cfs)

Primary OutFlow Max=5.24 cfs @ 12.16 hrs HW=272.67' TW=268.73' (Dynamic Tailwater)
1=Culvert (Inlet Controls 5.24 cfs @ 3.36 fps)

Summary for Pond TD2: Stone Trench Drain

Inflow Area = 11,308 sf, 0.00% Impervious, Inflow Depth = 2.66" for 100 Year event
Inflow = 0.64 cfs @ 12.09 hrs, Volume= 2,503 cf
Outflow = 0.52 cfs @ 12.16 hrs, Volume= 2,503 cf, Atten= 19%, Lag= 3.6 min
Discarded = 0.08 cfs @ 11.72 hrs, Volume= 2,077 cf
Primary = 0.44 cfs @ 12.16 hrs, Volume= 425 cf
Routed to Pond LP-2 : Low Point - MAHW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 273.36' @ 12.16 hrs Surf.Area= 427 sf Storage= 308 cf
Flood Elev= 275.00' Surf.Area= 1,466 sf Storage= 536 cf

Plug-Flow detention time= 15.5 min calculated for 2,502 cf (100% of inflow)
Center-of-Mass det. time= 15.5 min (824.0 - 808.5)

Volume	Invert	Avail.Storage	Storage Description
#1	272.00'	167 cf	3.50'W x 122.00'L x 1.75'H Prismaoid 747 cf Overall - 192 cf Embedded = 556 cf x 30.0% Voids
#2	272.25'	192 cf	12.0" Round Pipe Storage x 2 Inside #1 L= 122.0'
#3	273.75'	178 cf	Custom Stage Data (Irregular) Listed below (Recalc)
			536 cf Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
273.75	430	253.0	0	0	430
274.00	1,039	301.0	178	178	2,547

Device	Routing	Invert	Outlet Devices
#1	Primary	273.03'	12.0" Round Culvert L= 60.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 273.03' / 272.50' S= 0.0088 '/' Cc= 0.900 n= 0.012 Corrugated PP, smooth interior, Flow Area= 0.79 sf
#2	Discarded	272.00'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.08 cfs @ 11.72 hrs HW=272.03' (Free Discharge)
2=Exfiltration (Exfiltration Controls 0.08 cfs)

Primary OutFlow Max=0.43 cfs @ 12.16 hrs HW=273.36' TW=267.83' (Dynamic Tailwater)
1=Culvert (Barrel Controls 0.43 cfs @ 2.86 fps)

Summary for Pond UIS1: Underground Chamber #1

Inflow Area = 53,638 sf, 84.89% Impervious, Inflow Depth = 6.91" for 100 Year event
Inflow = 8.54 cfs @ 12.08 hrs, Volume= 30,881 cf
Outflow = 1.54 cfs @ 12.53 hrs, Volume= 30,881 cf, Atten= 82%, Lag= 27.0 min
Discarded = 0.84 cfs @ 11.56 hrs, Volume= 30,230 cf
Primary = 0.70 cfs @ 12.53 hrs, Volume= 650 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.46' @ 12.53 hrs Surf.Area= 4,410 sf Storage= 9,465 cf

Plug-Flow detention time= 69.1 min calculated for 30,872 cf (100% of inflow)
Center-of-Mass det. time= 69.1 min (814.5 - 745.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	271.20'	4,137 cf	53.75'W x 82.05'L x 3.75'H Field A 16,538 cf Overall - 6,197 cf Embedded = 10,341 cf x 40.0% Voids
#2A	271.70'	6,197 cf	ADS StormTech SC-800 +Cap x 121 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 121 Chambers in 11 Rows Cap Storage= 3.4 cf x 2 x 11 rows = 75.2 cf
			10,333 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	271.70'	15.0" Round Culvert L= 139.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 271.70' / 269.85' S= 0.0133 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf
#2	Device 1	274.30'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Discarded	271.20'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.84 cfs @ 11.56 hrs HW=271.24' (Free Discharge)
3=Exfiltration (Exfiltration Controls 0.84 cfs)

Primary OutFlow Max=0.70 cfs @ 12.53 hrs HW=274.46' TW=0.00' (Dynamic Tailwater)
1=Culvert (Passes 0.70 cfs of 6.81 cfs potential flow)
2=Broad-Crested Rectangular Weir (Weir Controls 0.70 cfs @ 1.11 fps)

Summary for Pond UIS2: Underground Chamber #2

444 E. Central St Franklin, MA
Type III 24-hr 100 Year Rainfall=8.16"
Prepared by Allen & Major Associates, Inc Printed 7/24/2025
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Inflow Area = 22,636 sf, 23.54% Impervious, Inflow Depth = 3.69" for 100 Year event
Inflow = 1.84 cfs @ 12.09 hrs, Volume= 6,956 cf
Outflow = 1.83 cfs @ 12.10 hrs, Volume= 6,956 cf, Atten= 0%, Lag= 0.3 min
Discarded = 0.09 cfs @ 10.71 hrs, Volume= 4,383 cf
Primary = 1.74 cfs @ 12.10 hrs, Volume= 2,573 cf
Routed to Pond LP-1 : Low Point - MAHW

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.04' @ 12.10 hrs Surf.Area= 470 sf Storage= 562 cf
Flood Elev= 275.00' Surf.Area= 470 sf Storage= 580 cf

Plug-Flow detention time= 27.3 min calculated for 6,956 cf (100% of inflow)
Center-of-Mass det. time= 27.3 min (806.6 - 779.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	271.80'	344 cf	14.83'W x 31.68'L x 2.33'H Field A 1,096 cf Overall - 236 cf Embedded = 861 cf x 40.0% Voids
#2A	272.30'	236 cf	ADS_StormTech SC-310 +Cap x 16 Inside #1 Effective Size= 28.9"W x 16.0"H => 2.07 sf x 7.12'L = 14.7 cf Overall Size= 34.0"W x 16.0"H x 7.56'L with 0.44' Overlap 16 Chambers in 4 Rows
			580 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	272.60'	12.0" Round Culvert L= 60.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 272.60' / 272.00' S= 0.0100 ' /' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	273.75'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Discarded	271.80'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.09 cfs @ 10.71 hrs HW=271.83' (Free Discharge)
3=Exfiltration (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=1.74 cfs @ 12.10 hrs HW=274.04' TW=268.64' (Dynamic Tailwater)
1=Culvert (Passes 1.74 cfs of 2.89 cfs potential flow)
2=Broad-Crested Rectangular Weir (Weir Controls 1.74 cfs @ 1.52 fps)

Summary for Pond UIS3: Underground Chamber #3

Inflow Area = 84,596 sf, 90.26% Impervious, Inflow Depth = 7.27" for 100 Year event
Inflow = 14.21 cfs @ 12.08 hrs, Volume= 51,238 cf
Outflow = 3.46 cfs @ 12.46 hrs, Volume= 51,238 cf, Atten= 76%, Lag= 22.7 min
Discarded = 1.33 cfs @ 11.49 hrs, Volume= 43,519 cf
Primary = 2.13 cfs @ 12.46 hrs, Volume= 7,720 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

444 E. Central St Franklin, MA
Type III 24-hr 100 Year Rainfall=8.16"
Prepared by Allen & Major Associates, Inc Printed 7/24/2025
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Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.88' @ 12.46 hrs Surf.Area= 6,945 sf Storage= 13,690 cf

Plug-Flow detention time= 38.8 min calculated for 51,224 cf (100% of inflow)
Center-of-Mass det. time= 38.8 min (782.5 - 743.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	272.00'	6,516 cf	30.00'W x 231.50'L x 3.75'H Field A 26,044 cf Overall - 9,755 cf Embedded = 16,289 cf x 40.0% Voids
#2A	272.50'	9,755 cf	ADS_StormTech SC-800 +Cap x 192 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 192 Chambers in 6 Rows Cap Storage= 3.4 cf x 2 x 6 rows = 41.0 cf
			16,270 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	273.29'	12.0" Round Culvert L= 110.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 273.29' / 272.00' S= 0.0117 ' /' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	275.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Device 1	273.50'	9.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	272.00'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=1.33 cfs @ 11.49 hrs HW=272.04' (Free Discharge)
4=Exfiltration (Exfiltration Controls 1.33 cfs)

Primary OutFlow Max=2.13 cfs @ 12.46 hrs HW=274.88' TW=0.00' (Dynamic Tailwater)
1=Culvert (Passes 2.13 cfs of 3.12 cfs potential flow)
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
3=Orifice/Grate (Orifice Controls 2.13 cfs @ 4.82 fps)

Summary for Pond UIS4: Underground Chamber #4

Inflow Area = 42,721 sf, 77.28% Impervious, Inflow Depth = 6.40" for 100 Year event
Inflow = 6.27 cfs @ 12.08 hrs, Volume= 22,780 cf
Outflow = 2.72 cfs @ 12.27 hrs, Volume= 22,780 cf, Atten= 57%, Lag= 11.4 min
Discarded = 0.46 cfs @ 11.27 hrs, Volume= 17,563 cf
Primary = 2.26 cfs @ 12.27 hrs, Volume= 5,216 cf
Routed to Link SP3 : On Site Flow To Perennial Stream

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 275.75' @ 12.27 hrs Surf.Area= 2,398 sf Storage= 5,097 cf

Plug-Flow detention time= 36.1 min calculated for 22,780 cf (100% of inflow)
Center-of-Mass det. time= 36.1 min (784.4 - 748.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	272.50'	2,279 cf	39.50'W x 60.70'L x 3.75'H Field A 8,991 cf Overall - 3,293 cf Embedded = 5,699 cf x 40.0% Voids
#2A	273.00'	3,293 cf	ADS_StormTech SC-800 +Cap x 64 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 64 Chambers in 8 Rows Cap Storage= 3.4 cf x 2 x 8 rows = 54.7 cf
		5,572 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	273.00'	12.0" Round Culvert L= 264.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 273.00' / 271.00' S= 0.0076 ' /' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	276.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Device 1	274.25'	9.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Discarded	272.50'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=0.46 cfs @ 11.27 hrs HW=272.54' (Free Discharge)
↳ **4=Exfiltration** (Exfiltration Controls 0.46 cfs)

Primary OutFlow Max=2.26 cfs @ 12.27 hrs HW=275.75' TW=0.00' (Dynamic Tailwater)
↳ **1=Culvert** (Passes 2.26 cfs of 3.83 cfs potential flow)
↳ **2=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)
↳ **3=Orifice/Grate** (Orifice Controls 2.26 cfs @ 5.12 fps)

Summary for Pond UIS5: Underground Chamber #5

Inflow Area = 157,891 sf, 63.68% Impervious, Inflow Depth = 5.49" for 100 Year event
Inflow = 19.65 cfs @ 12.09 hrs, Volume= 72,212 cf
Outflow = 3.49 cfs @ 12.55 hrs, Volume= 72,212 cf, Atten= 82%, Lag= 27.7 min
Discarded = 1.66 cfs @ 11.46 hrs, Volume= 69,666 cf
Primary = 1.84 cfs @ 12.55 hrs, Volume= 2,546 cf
Routed to Pond MH19 : Manhole

Routing by Dyn-Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs / 3
Peak Elev= 274.30' @ 12.55 hrs Surf.Area= 8,653 sf Storage= 22,870 cf

Plug-Flow detention time= 90.3 min calculated for 72,192 cf (100% of inflow)
Center-of-Mass det. time= 90.3 min (844.9 - 754.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	269.80'	11,567 cf	30.00'W x 288.43'L x 4.75'H Field A 41,102 cf Overall - 12,183 cf Embedded = 28,919 cf x 40.0% Voids
#2A	270.80'	12,183 cf	ADS_StormTech SC-800 +Cap x 240 Inside #1 Effective Size= 45.0"W x 33.0"H => 7.11 sf x 7.12'L = 50.6 cf Overall Size= 51.0"W x 33.0"H x 7.55'L with 0.43' Overlap 240 Chambers in 6 Rows Cap Storage= 3.4 cf x 2 x 6 rows = 41.0 cf
		23,751 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	273.05'	12.0" Round Culvert L= 97.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 273.05' / 272.00' S= 0.0108 ' /' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf
#2	Device 1	274.00'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32
#3	Discarded	269.80'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'

Discarded OutFlow Max=1.66 cfs @ 11.46 hrs HW=269.85' (Free Discharge)
↳ **3=Exfiltration** (Exfiltration Controls 1.66 cfs)

Primary OutFlow Max=1.84 cfs @ 12.55 hrs HW=274.30' TW=270.54' (Dynamic Tailwater)
↳ **1=Culvert** (Passes 1.84 cfs of 2.58 cfs potential flow)
↳ **2=Broad-Crested Rectangular Weir** (Weir Controls 1.84 cfs @ 1.55 fps)

Summary for Link SP1: On Site Flow To Perennial Stream

Inflow Area = 210,774 sf, 10.22% Impervious, Inflow Depth = 1.10" for 100 Year event
Inflow = 6.88 cfs @ 12.15 hrs, Volume= 19,352 cf
Primary = 6.88 cfs @ 12.15 hrs, Volume= 19,352 cf, Atten= 0%, Lag= 0.0 min
Routed to Pond LP-2 : Low Point - MAHW

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP2: On Site Flow To Perennial Stream

Inflow Area = 236,963 sf, 10.14% Impervious, Inflow Depth = 1.15" for 100 Year event
Inflow = 7.42 cfs @ 12.18 hrs, Volume= 22,678 cf
Primary = 7.42 cfs @ 12.18 hrs, Volume= 22,678 cf, Atten= 0%, Lag= 0.0 min
Routed to Link SP3 : On Site Flow To Perennial Stream

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

Summary for Link SP3: On Site Flow To Perennial Stream

Inflow Area = 867,482 sf, 36.39% Impervious, Inflow Depth = 1.10" for 100 Year event
Inflow = 18.24 cfs @ 12.19 hrs, Volume= 79,294 cf
Primary = 18.24 cfs @ 12.19 hrs, Volume= 79,294 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs

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**ISSUED FOR
DRAINAGE REPORT**
JULY 21, 2025

PROFESSIONAL ENGINEER FOR
ALLEN & MAJOR ASSOCIATES, INC.

A	7/21/25	REVISED PER NOI PEER REVIEW
A	6/11/25	REVISED PER TOWN COMMENTS
REV	DATE	DESCRIPTION

APPLICANT/OWNER:

TAG CENTRAL LLC
275 REGATTA DRIVE
JUPITER, FL 33477

PROJECT:

40B MULTIFAMILY
444 EAST CENTRAL STREET
FRANKLIN, MA

PROJECT NO. 3317-01 DATE: 02-11-2025

SCALE: 1" = 50' DWG. NAME: C-3317-01

DESIGNED BY: MTB CHECKED BY: CMQ

PREPARED BY:

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DRAWING TITLE:

PROPOSED WATERSHED PLAN

SHEET No.

PWS

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SECTION 6.0 - APPENDIX



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypsak, Dale Urruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerials](#)

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.321 (0.252-0.404)	0.391 (0.307-0.493)	0.506 (0.395-0.640)	0.602 (0.455-0.788)	0.733 (0.551-0.978)	0.832 (0.612-1.13)	0.935 (0.687-1.32)	1.05 (0.709-1.52)	1.21 (0.788-1.83)	1.35 (0.853-2.07)
10-min	0.454 (0.357-0.572)	0.554 (0.435-0.698)	0.717 (0.561-0.908)	0.852 (0.663-1.08)	1.04 (0.780-1.38)	1.18 (0.867-1.60)	1.32 (0.945-1.87)	1.49 (1.00-2.15)	1.72 (1.12-2.59)	1.91 (1.21-2.93)
15-min	0.535 (0.420-0.673)	0.652 (0.512-0.821)	0.843 (0.660-1.07)	1.00 (0.780-1.28)	1.22 (0.915-1.63)	1.39 (1.02-1.89)	1.56 (1.11-2.20)	1.75 (1.18-2.64)	2.02 (1.31-3.04)	2.25 (1.42-3.45)
30-min	0.733 (0.576-0.922)	0.896 (0.702-1.13)	1.16 (0.907-1.48)	1.38 (1.07-1.75)	1.68 (1.26-2.24)	1.90 (1.40-2.59)	2.14 (1.53-3.03)	2.41 (1.63-3.49)	2.78 (1.81-4.19)	3.09 (1.98-4.75)
60-min	0.932 (0.732-1.17)	1.14 (0.893-1.43)	1.47 (1.15-1.85)	1.75 (1.35-2.23)	2.14 (1.60-2.84)	2.42 (1.78-3.30)	2.73 (1.95-3.85)	3.06 (2.07-4.44)	3.55 (2.30-5.34)	3.94 (2.49-6.05)
2-hr	1.19 (0.945-1.48)	1.47 (1.16-1.84)	1.93 (1.52-2.42)	2.30 (1.81-2.91)	2.82 (2.14-3.75)	3.21 (2.39-4.37)	3.62 (2.62-5.15)	4.12 (2.79-5.94)	4.88 (3.17-7.29)	5.52 (3.51-8.42)
3-hr	1.38 (1.10-1.72)	1.71 (1.36-2.12)	2.24 (1.77-2.80)	2.68 (2.11-3.37)	3.29 (2.51-4.35)	3.74 (2.80-5.08)	4.23 (3.08-6.01)	4.83 (3.28-6.94)	5.76 (3.75-8.57)	6.56 (4.17-9.95)
6-hr	1.79 (1.43-2.20)	2.20 (1.76-2.71)	2.87 (2.29-3.58)	3.43 (2.72-4.28)	4.20 (3.22-5.62)	4.76 (3.59-6.43)	5.38 (3.95-7.60)	6.15 (4.19-8.78)	7.34 (4.80-10.8)	8.37 (5.34-12.8)
12-hr	2.29 (1.85-2.80)	2.79 (2.25-3.42)	3.61 (2.90-4.44)	4.29 (3.43-5.31)	5.23 (4.04-6.81)	5.92 (4.48-7.91)	6.68 (4.91-9.32)	7.59 (5.20-10.7)	8.99 (5.91-13.2)	10.2 (6.53-15.2)
24-hr	2.75 (2.24-3.34)	3.37 (2.74-4.09)	4.38 (3.55-5.34)	5.22 (4.20-6.41)	6.38 (4.95-8.25)	7.23 (5.51-9.59)	8.16 (6.04-11.3)	9.30 (6.40-13.0)	11.1 (7.29-16.1)	12.6 (8.07-18.8)
2-day	3.09 (2.54-3.73)	3.86 (3.16-4.65)	5.10 (4.17-6.18)	6.14 (4.98-7.48)	7.56 (5.94-9.74)	8.61 (6.82-11.4)	9.76 (7.31-13.5)	11.2 (7.75-15.8)	13.5 (8.94-19.5)	15.5 (10.0-22.9)
3-day	3.37 (2.78-4.04)	4.19 (3.45-5.03)	5.53 (4.54-6.67)	6.64 (5.42-8.08)	8.18 (6.45-10.5)	9.30 (7.18-12.2)	10.5 (7.92-14.5)	12.1 (8.39-16.8)	14.6 (9.68-21.0)	16.8 (10.8-24.8)
4-day	3.63 (3.01-4.34)	4.48 (3.71-5.37)	5.87 (4.84-7.08)	7.03 (5.75-8.49)	8.62 (6.81-11.0)	9.78 (7.57-12.8)	11.1 (8.32-15.2)	12.7 (8.80-17.5)	15.2 (10.1-21.8)	17.4 (11.3-25.5)
7-day	4.38 (3.65-5.20)	5.28 (4.39-6.28)	6.75 (5.60-8.05)	7.97 (6.59-9.57)	9.65 (7.65-12.2)	10.9 (8.45-14.1)	12.2 (9.20-16.6)	13.9 (9.69-19.0)	16.4 (10.9-23.3)	18.6 (12.0-26.9)
10-day	5.09 (4.28-6.02)	6.02 (5.03-7.13)	7.54 (6.27-8.95)	8.80 (7.27-10.5)	10.5 (8.38-13.2)	11.8 (9.19-15.2)	13.2 (9.91-17.7)	14.8 (10.4-20.3)	17.3 (11.6-24.5)	19.3 (12.6-28.0)
20-day	7.18 (6.05-8.42)	8.17 (6.88-9.60)	9.80 (8.22-11.6)	11.1 (9.28-13.2)	13.0 (10.4-16.1)	14.4 (11.2-18.2)	15.9 (11.9-20.8)	17.5 (12.3-23.8)	19.7 (13.2-27.6)	21.4 (14.0-30.8)
30-day	8.91 (7.58-10.4)	9.96 (8.43-11.6)	11.7 (9.83-13.7)	13.1 (10.9-15.4)	15.0 (12.0-18.4)	16.5 (12.9-20.7)	18.0 (13.5-23.3)	19.5 (13.8-26.3)	21.6 (14.6-30.1)	23.2 (16.1-33.0)
45-day	11.1 (9.45-12.9)	12.2 (10.4-14.2)	14.0 (11.8-16.3)	15.5 (13.0-18.2)	17.5 (14.1-21.3)	19.1 (14.9-23.7)	20.7 (15.4-26.5)	22.1 (16.7-29.8)	24.0 (18.2-33.3)	25.3 (18.6-36.0)
60-day	12.9 (11.0-15.0)	14.1 (12.0-16.3)	15.9 (13.5-18.6)	17.5 (14.8-20.5)	19.6 (15.8-23.8)	21.3 (16.7-26.3)	22.9 (17.1-29.1)	24.3 (17.3-32.4)	26.0 (17.7-36.0)	27.1 (17.8-38.4)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS). Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

Manning's Roughness Coefficients ("n")

Conduit	Manning's Coefficients
Closed Conduits	
Asbestos-Cement Pipe	0.011 to 0.015
Brick	0.013 to 0.017
Cast Iron Pipe	
Cement-lined and seal-coated	0.011 to 0.015
Concrete (Monolithic)	
Smooth forms	0.012 to 0.014
Rough forms	0.015 to 0.017
Concrete Pipe	0.011 to 0.015
Corrugated-Metal Pipe (1/2 - STUL 34470 2 1/2-inch corrgrtn.)	
Plain	0.022 to 0.026
Paved invert	0.018 to 0.022
Spun asphalt-lined	0.011 to 0.015
Plastic Pipe (Smooth)	0.011 to 0.015
Vitrified Clay	
Pipes	0.011 to 0.015
Liner channels	0.013 to 0.017
Open Channels	
Lined Channels	
Asphalt	0.013 to 0.017
Brick	0.012 to 0.018
Concrete	0.011 to 0.020
Rubble or riprap	0.020 to 0.035
Vegetal	0.030 to 0.040
Excavated or Dredged	
Earth, straight and uniform	0.020 to 0.030
Earth, winding, fairly uniform	0.025 to 0.040
Rock	0.030 to 0.045
Unmaintained	0.050 to 0.140
Natural Channels (minor streams, top width at flood state < 100 feet)	
Fairly regular section	0.030 to 0.070
Irregular section with pools	0.040 to 0.100

Source: Design and Construction of Sanitary and Storm Sewers, American Society of Civil Engineers and the Water Pollution Control Federation, 1969.



United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for Norfolk and Suffolk Counties, Massachusetts

444 East Central Street



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

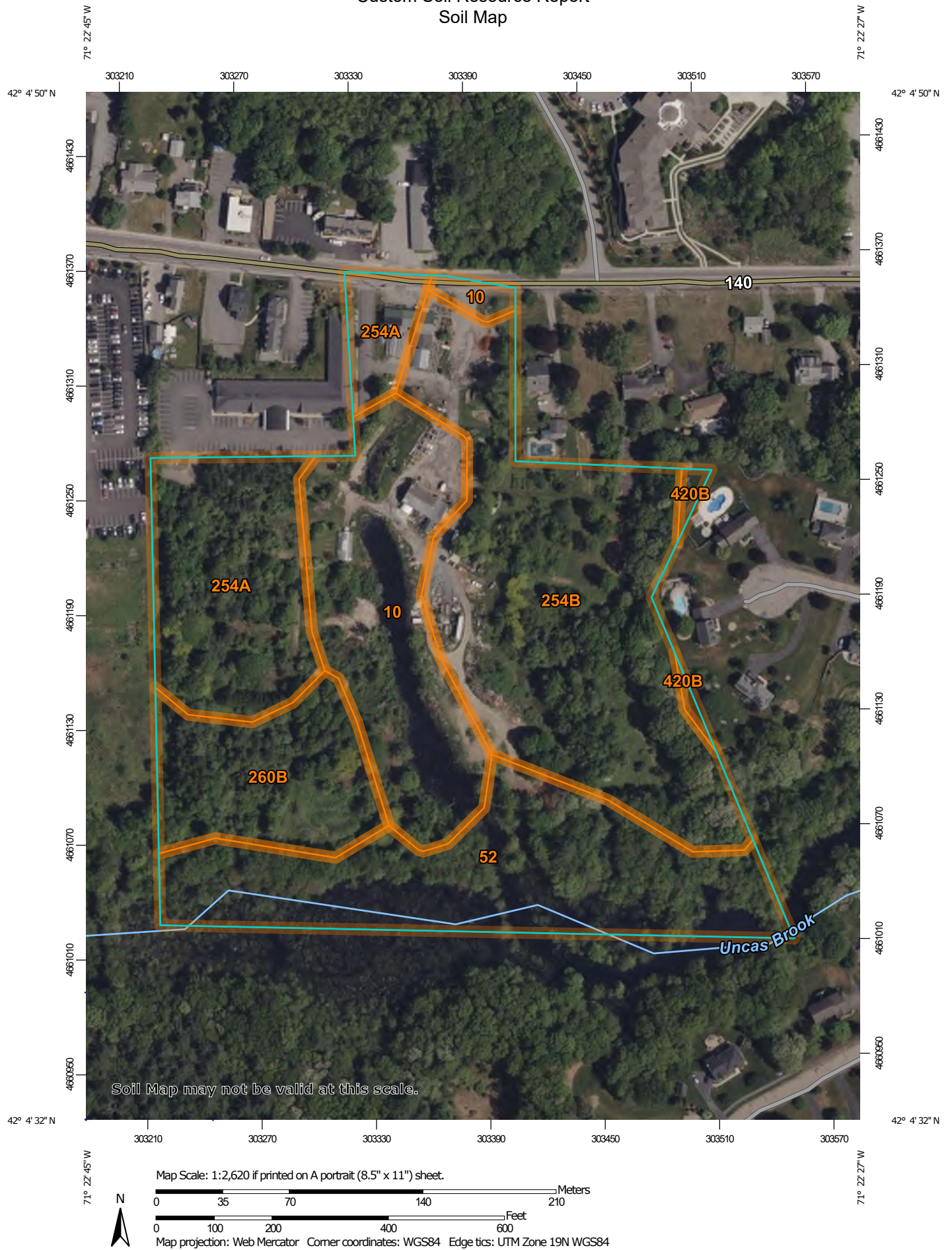
Custom Soil Resource Report

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map



Custom Soil Resource Report

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit

 Clay Spot

 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water

 Perennial Water

 Rock Outcrop

 Saline Spot

 Sandy Spot

 Severely Eroded Spot

 Sinkhole

 Slide or Slip

 Sodic Spot

 Spoil Area

 Stony Spot

 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals

Transportation

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Norfolk and Suffolk Counties, Massachusetts
Survey Area Data: Version 19, Sep 10, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 22, 2022—Jun 5, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
10	Scarboro and Birdsall soils, 0 to 3 percent slopes	3.7	18.8%
52	Freetown muck, 0 to 1 percent slopes	4.3	21.8%
254A	Merrimac fine sandy loam, 0 to 3 percent slopes	3.3	16.4%
254B	Merrimac fine sandy loam, 3 to 8 percent slopes	6.3	31.9%
260B	Sudbury fine sandy loam, 2 to 8 percent slopes	2.1	10.5%
420B	Canton fine sandy loam, 3 to 8 percent slopes	0.1	0.6%
Totals for Area of Interest		19.8	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it

was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Norfolk and Suffolk Counties, Massachusetts

10—Scarboro and Birdsall soils, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: vkxw
Elevation: 0 to 2,100 feet
Mean annual precipitation: 45 to 54 inches
Mean annual air temperature: 43 to 54 degrees F
Frost-free period: 145 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Scarboro and similar soils: 65 percent
Birdsall and similar soils: 25 percent
Minor components: 10 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Scarboro

Setting

Landform: Terraces
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Tread
Down-slope shape: Concave
Across-slope shape: Concave
Parent material: Loose sandy glaciofluvial deposits

Typical profile

H1 - 0 to 9 inches: mucky fine sandy loam
H2 - 9 to 60 inches: stratified loamy fine sand to gravelly coarse sand

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (6.00 to 20.00 in/hr)
Depth to water table: About 0 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Available water supply, 0 to 60 inches: Low (about 5.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 5w
Hydrologic Soil Group: A/D
Ecological site: F144AY031MA - Very Wet Outwash
Hydric soil rating: Yes

Description of Birdsall

Setting

Landform: Terraces
Landform position (two-dimensional): Toeslope

Custom Soil Resource Report

Landform position (three-dimensional): Tread
Down-slope shape: Concave
Across-slope shape: Concave
Parent material: Soft coarse-silty glaciolacustrine deposits

Typical profile

H1 - 0 to 8 inches: very fine sandy loam
H2 - 8 to 16 inches: very fine sandy loam
H3 - 16 to 60 inches: silt loam

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 0 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Available water supply, 0 to 60 inches: Very high (about 12.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 5w
Hydrologic Soil Group: C/D
Ecological site: F144AY031MA - Very Wet Outwash
Hydric soil rating: Yes

Minor Components

Swansea

Percent of map unit: 5 percent
Landform: Bogs
Hydric soil rating: Yes

Raynham

Percent of map unit: 3 percent
Landform: Depressions
Hydric soil rating: Yes

Walpole

Percent of map unit: 2 percent
Landform: Terraces
Hydric soil rating: Yes

52—Freetown muck, 0 to 1 percent slopes

Map Unit Setting

National map unit symbol: 2t2q9
Elevation: 0 to 1,110 feet

Custom Soil Resource Report

Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Not prime farmland

Map Unit Composition

Freetown and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Freetown

Setting

Landform: Depressions, depressions, swamps, kettles, marshes, bogs
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Tread, dip
Down-slope shape: Concave
Across-slope shape: Concave
Parent material: Highly decomposed organic material

Typical profile

Oe - 0 to 2 inches: mucky peat
Oa - 2 to 79 inches: muck

Properties and qualities

Slope: 0 to 1 percent
Surface area covered with cobbles, stones or boulders: 0.0 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high
(0.14 to 14.17 in/hr)
Depth to water table: About 0 to 6 inches
Frequency of flooding: Rare
Frequency of ponding: Frequent
Available water supply, 0 to 60 inches: Very high (about 19.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 5w
Hydrologic Soil Group: B/D
Ecological site: F144AY043MA - Acidic Organic Wetlands
Hydric soil rating: Yes

Minor Components

Scarboro

Percent of map unit: 5 percent
Landform: Drainageways, depressions
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Base slope, tread, dip
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Swansea

Percent of map unit: 5 percent

Custom Soil Resource Report

Landform: Bogs, swamps, marshes, depressions, depressions, kettles
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Tread, dip
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

Whitman

Percent of map unit: 5 percent
Landform: Drainageways, depressions
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Base slope
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

254A—Merrimac fine sandy loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 2tyqr
Elevation: 0 to 1,100 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: All areas are prime farmland

Map Unit Composition

Merrimac and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Merrimac

Setting

Landform: Outwash plains, outwash terraces, moraines, eskers, kames
Landform position (two-dimensional): Summit, shoulder, backslope, footslope
Landform position (three-dimensional): Crest, side slope, riser, tread
Down-slope shape: Convex
Across-slope shape: Convex
Parent material: Loamy glaciofluvial deposits derived from granite, schist, and gneiss over sandy and gravelly glaciofluvial deposits derived from granite, schist, and gneiss

Typical profile

Ap - 0 to 10 inches: fine sandy loam
Bw1 - 10 to 22 inches: fine sandy loam
Bw2 - 22 to 26 inches: stratified gravel to gravelly loamy sand
2C - 26 to 65 inches: stratified gravel to very gravelly sand

Properties and qualities

Slope: 0 to 3 percent

Custom Soil Resource Report

Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 99.90 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 2 percent
Maximum salinity: Nonsaline (0.0 to 1.4 mmhos/cm)
Sodium adsorption ratio, maximum: 1.0
Available water supply, 0 to 60 inches: Low (about 4.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2s
Hydrologic Soil Group: A
Ecological site: F145XY008MA - Dry Outwash
Hydric soil rating: No

Minor Components

Sudbury

Percent of map unit: 5 percent
Landform: Deltas, terraces, outwash plains
Landform position (two-dimensional): Footslope
Landform position (three-dimensional): Tread, dip
Down-slope shape: Concave
Across-slope shape: Linear
Hydric soil rating: No

Hinckley

Percent of map unit: 5 percent
Landform: Deltas, kames, eskers, outwash plains
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Crest, side slope, head slope, nose slope, rise
Down-slope shape: Convex
Across-slope shape: Convex, linear
Hydric soil rating: No

Agawam

Percent of map unit: 3 percent
Landform: Stream terraces, outwash terraces, outwash plains, moraines, eskers, kames
Landform position (three-dimensional): Rise
Down-slope shape: Convex
Across-slope shape: Convex
Hydric soil rating: No

Windsor

Percent of map unit: 2 percent
Landform: Dunes, deltas, outwash terraces, outwash plains
Landform position (two-dimensional): Summit
Landform position (three-dimensional): Tread, riser
Down-slope shape: Convex, linear
Across-slope shape: Convex, linear

Hydric soil rating: No

254B—Merrimac fine sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2tyqs

Elevation: 0 to 1,290 feet

Mean annual precipitation: 36 to 71 inches

Mean annual air temperature: 39 to 55 degrees F

Frost-free period: 140 to 240 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Merrimac and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Merrimac

Setting

Landform: Kames, outwash plains, outwash terraces, moraines, eskers

Landform position (two-dimensional): Summit, shoulder, backslope, footslope

Landform position (three-dimensional): Crest, side slope, riser, tread

Down-slope shape: Convex

Across-slope shape: Convex

Parent material: Loamy glaciofluvial deposits derived from granite, schist, and gneiss over sandy and gravelly glaciofluvial deposits derived from granite, schist, and gneiss

Typical profile

Ap - 0 to 10 inches: fine sandy loam

Bw1 - 10 to 22 inches: fine sandy loam

Bw2 - 22 to 26 inches: stratified gravel to gravelly loamy sand

2C - 26 to 65 inches: stratified gravel to very gravelly sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Somewhat excessively drained

Runoff class: Very low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 99.90 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Calcium carbonate, maximum content: 2 percent

Maximum salinity: Nonsaline (0.0 to 1.4 mmhos/cm)

Sodium adsorption ratio, maximum: 1.0

Available water supply, 0 to 60 inches: Low (about 4.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2s

Hydrologic Soil Group: A

Ecological site: F145XY008MA - Dry Outwash

Hydric soil rating: No

Minor Components

Sudbury

Percent of map unit: 5 percent

Landform: Deltas, terraces, outwash plains

Landform position (two-dimensional): Footslope

Landform position (three-dimensional): Tread, dip

Down-slope shape: Concave

Across-slope shape: Linear

Hydric soil rating: No

Hinckley

Percent of map unit: 5 percent

Landform: Deltas, kames, eskers, outwash plains

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Crest, side slope, head slope, nose slope, rise

Down-slope shape: Convex

Across-slope shape: Convex, linear

Hydric soil rating: No

Windsor

Percent of map unit: 3 percent

Landform: Outwash terraces, dunes, deltas, outwash plains

Landform position (two-dimensional): Shoulder

Landform position (three-dimensional): Tread, riser

Down-slope shape: Linear, convex

Across-slope shape: Linear, convex

Hydric soil rating: No

Agawam

Percent of map unit: 2 percent

Landform: Outwash plains, outwash terraces, moraines, stream terraces, eskers, kames

Landform position (three-dimensional): Rise

Down-slope shape: Convex

Across-slope shape: Convex

Hydric soil rating: No

260B—Sudbury fine sandy loam, 2 to 8 percent slopes

Map Unit Setting

National map unit symbol: vky4

Custom Soil Resource Report

Elevation: 0 to 2,100 feet

Mean annual precipitation: 45 to 54 inches

Mean annual air temperature: 43 to 54 degrees F

Frost-free period: 145 to 240 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Sudbury and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Sudbury

Setting

Landform: Outwash plains

Landform position (two-dimensional): Footslope

Landform position (three-dimensional): Riser

Down-slope shape: Linear

Across-slope shape: Concave

Parent material: Friable coarse-loamy eolian deposits over loose sandy glaciofluvial deposits

Typical profile

H1 - 0 to 11 inches: sandy loam

H2 - 11 to 22 inches: sandy loam

H3 - 22 to 60 inches: gravelly coarse sand

Properties and qualities

Slope: 2 to 8 percent

Depth to restrictive feature: 18 to 36 inches to strongly contrasting textural stratification

Drainage class: Moderately well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)

Depth to water table: About 18 to 36 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 4.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: B

Ecological site: F144AY027MA - Moist Sandy Outwash

Hydric soil rating: No

Minor Components

Merrimac

Percent of map unit: 5 percent

Hydric soil rating: No

Deerfield

Percent of map unit: 5 percent

Landform: Outwash plains

Landform position (two-dimensional): Footslope

Custom Soil Resource Report

Landform position (three-dimensional): Tread
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: No

Walpole

Percent of map unit: 5 percent
Landform: Terraces
Hydric soil rating: Yes

420B—Canton fine sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2w81b
Elevation: 0 to 1,180 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: All areas are prime farmland

Map Unit Composition

Canton and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Canton

Setting

Landform: Hills, moraines, ridges
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Crest, nose slope, side slope
Down-slope shape: Convex, linear
Across-slope shape: Convex
Parent material: Coarse-loamy over sandy melt-out till derived from gneiss, granite, and/or schist

Typical profile

Ap - 0 to 7 inches: fine sandy loam
Bw1 - 7 to 15 inches: fine sandy loam
Bw2 - 15 to 26 inches: gravelly fine sandy loam
2C - 26 to 65 inches: gravelly loamy sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: 19 to 39 inches to strongly contrasting textural stratification
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.14 to 14.17 in/hr)
Depth to water table: More than 80 inches

Custom Soil Resource Report

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Very low (about 2.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2s

Hydrologic Soil Group: B

Ecological site: F144AY034CT - Well Drained Till Uplands

Hydric soil rating: No

Minor Components

Scituate

Percent of map unit: 10 percent

Landform: Hills, drumlins, ground moraines

Landform position (two-dimensional): Summit, backslope, footslope

Landform position (three-dimensional): Crest, side slope

Down-slope shape: Convex, linear

Across-slope shape: Convex

Hydric soil rating: No

Montauk

Percent of map unit: 5 percent

Landform: Moraines, ground moraines, hills, drumlins

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Crest, side slope

Down-slope shape: Convex, linear

Across-slope shape: Convex

Hydric soil rating: No

Charlton

Percent of map unit: 4 percent

Landform: Ridges, ground moraines, hills

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Crest, side slope

Down-slope shape: Convex, linear

Across-slope shape: Convex

Hydric soil rating: No

Swansea

Percent of map unit: 1 percent

Landform: Marshes, depressions, bogs, swamps, kettles

Down-slope shape: Concave

Across-slope shape: Concave

Hydric soil rating: Yes

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Custom Soil Resource Report

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Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

TAG Central LLC

Owner Name

444 E Central Street

Street Address

Franklin

City

MA

State

284-066-000-000

Map/Lot #

02038

Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade
2. Soil Survey Web Soil Survey 254B Merrimac fine sandy loam 3-8% slopes
Source Soil Map Unit Soil Series
Kames, outwash plains, outwash terraces,
moraines, eskers Somewhat excessively drained.
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Landform Soil Limitations
Soil Parent material
3. Surficial Geological Report 2018 / Stone, Stone, & DiGiacomo-Cohen Coarse Deposits
Year Published/Source Map Unit
consist of gravel deposits, sand and gravel deposits, and sand deposits
Description of Geologic Map Unit:
4. Flood Rate Insurance Map Within a regulatory floodway? ☒ Yes ☐ No
5. Within a velocity zone? ☐ Yes ☒ No
6. Within a Mapped Wetland Area? ☒ Yes ☐ No If yes, MassGIS Wetland Data Layer: Shrub Swamp
Wetland Type
7. Current Water Resource Conditions (USGS): 5/17/2025 Range: ☒ Above Normal ☐ Normal ☐ Below Normal
Month/Day/ Year
8. Other references reviewed: Not in Zone A or IWPA zones.
(Zone II, IWPA, Zone A, EEA Data Portal, etc.)
Is within Zone II area.



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-101 2025/05/27 9:30am Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use Vacant lot Landscaped area and N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) small trees Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)
Vegetation

Description of Location: Lawn area near entrance of site.

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terrace TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line 30+ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 32" Depth to Weeping in Hole 45" Depth to Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-12	A	FSL	10YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
12-18	Bw	Fine Med. Sand	10YR 5/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
18-48	C	Medium Sand	10YR 5/4	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
					Cnc : Dpl:	-	-	-	-	-	-
					Cnc : Dpl:						
					Cnc : Dpl:						



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-102 2025/05/20 9:55am Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use: Parking Lot Trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: Parking Lot at driveway near front

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terraces TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 16" Depth to Weeping in Hole 46" Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-12	B	FSL	10YR 3/1	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
12-46	C1	Medium Loamy Sand	10YR 4/1	16	Cnc :7.5YR 5/8 Dpl: GLEY1 6/10Y	5%	-	-	Massive	Friable	Trace Silt
				-	Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

D. Determination of High Groundwater Elevation

1. Method Used (Choose one):

☒ Depth to soil redoximorphic features

Obs. Hole # STP-1

_____ inches

Obs. Hole # TP-102

16 inches

☒ Depth to observed standing water in observation hole

32 inches

_____ inches

☐ Depth to adjusted seasonal high groundwater (S_h)
(USGS methodology)

_____ inches

_____ inches

Index Well Number

Reading Date

$$S_h = S_c - [S_r \times (OW_c - OW_{max}) / OW_r]$$

Obs. Hole/Well# _____ S_c _____ S_r _____ OW_c _____ OW_{max} _____ OW_r _____ S_h _____

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

b. If yes, at what depth was it observed (exclude O, A, and E Horizons)?

Upper boundary: 12
inches

Lower boundary: 46
inches

c. If no, at what depth was impervious material observed?

Upper boundary: _____
inches

Lower boundary: _____
inches



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator

DAVID M. ROBINSON, EIT

Typed or Printed Name of Soil Evaluator / License #

N/A

Name of Approving Authority Witness

05/27/2025

Date

June 30, 2026

Expiration Date of License

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:

See Test Pits Plan, Sheet TPP-1



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

TAG Central LLC

Owner Name

444 E Central Street

Street Address

Franklin

City

MA

State

284-066-000-000

Map/Lot #

02038

Zip Code

B. Site Information

- (Check one) ☒ New Construction ☐ Upgrade
- Soil Survey Web Soil Survey 254B Merrimac fine sandy loam 3-8% slopes
Source Soil Map Unit Soil Series
Kames, outwash plains, outwash terraces,
moraines, eskers Somewhat excessively drained.
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Landform Soil Limitations
Soil Parent material
- Surficial Geological Report 2018 / Stone, Stone, & DiGiacomo-Cohen Coarse Deposits
Year Published/Source Map Unit
consist of gravel deposits, sand and gravel deposits, and sand deposits
Description of Geologic Map Unit:
- Flood Rate Insurance Map Within a regulatory floodway? ☒ Yes ☐ No
- Within a velocity zone? ☐ Yes ☒ No
- Within a Mapped Wetland Area? ☒ Yes ☐ No If yes, MassGIS Wetland Data Layer: Shrub Swamp
Wetland Type
- Current Water Resource Conditions (USGS): 5/17/2025 Range: ☒ Above Normal ☐ Normal ☐ Below Normal
Month/Day/ Year
- Other references reviewed: Not in Zone A or IWPA zones.
(Zone II, IWPA, Zone A, EEA Data Portal, etc.)
Is within Zone II area.



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-103 2025/05/27 10:00am Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use Vacant lot Landscaped area and N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) small trees Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)
Vegetation

Description of Location: Treeline opposite existing shop building

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terrace TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 40" Depth to Weeping in Hole 40" Depth to Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-26	Bw	FSL	10YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
26-40	C	Fine Sand	10YR 5/2	40	Cnc :7.5YR 5/8 Dpl: GLEY1 6/10Y	5%	-	-	Massive	Friable	-
				-	Cnc : Dpl:	-	-	-	Massive	Friable	-
					Cnc : Dpl:	-	-	-	-	-	-
					Cnc : Dpl:						
					Cnc : Dpl:						



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-104 2025/05/21 10:00am Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use: Parking Lot Trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: Near path to rear southeast portion of site

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terraces TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 48" Depth to Weeping in Hole 72" Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-20	A	FSL	10YR 3/1	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
20-28	Bw	Gravelly Med Sand	10YR 4/1	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
28-72	C	Fine Sand	10YR5/2	40	Cnc :7.5YR 5/8 Dpl: GLEY1 6/10Y	10%	-	-	Massive	Friable	Trace Silt
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

D. Determination of High Groundwater Elevation

1. Method Used (Choose one):

☒ Depth to soil redoximorphic features

Obs. Hole # TP-103

40 inches

Obs. Hole # TP-104

40 inches

☐ Depth to observed standing water in observation hole

_____ inches

_____ inches

☐ Depth to adjusted seasonal high groundwater (S_h)
(USGS methodology)

_____ inches

_____ inches

Index Well Number

Reading Date

$$S_h = S_c - [S_r \times (OW_c - OW_{max}) / OW_r]$$

Obs. Hole/Well# _____ S_c _____ S_r _____ OW_c _____ OW_{max} _____ OW_r _____ S_h _____

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

b. If yes, at what depth was it observed (exclude O, A, and E Horizons)?

Upper boundary: 20
inches

Lower boundary: 72
inches

c. If no, at what depth was impervious material observed?

Upper boundary: _____
inches

Lower boundary: _____
inches



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator

DAVID M. ROBINSON, EIT

Typed or Printed Name of Soil Evaluator / License #

N/A

Name of Approving Authority Witness

05/27/2025

Date

June 30, 2026

Expiration Date of License

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:

See Test Pits Plan, Sheet TPP-1



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

TAG Central LLC

Owner Name

444 E Central Street

Street Address

Franklin

City

MA

State

284-066-000-000

Map/Lot #

02038

Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade
2. Soil Survey Web Soil Survey 254B Merrimac fine sandy loam 3-8% slopes
Source Soil Map Unit Soil Series
Kames, outwash plains, outwash terraces, Somewhat excessively drained.
moraines, eskers Soil Limitations
Landform
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Soil Parent material
3. Surficial Geological Report 2018 / Stone, Stone, & DiGiacomo-Cohen Coarse Deposits
Year Published/Source Map Unit
consist of gravel deposits, sand and gravel deposits, and sand deposits
Description of Geologic Map Unit:
4. Flood Rate Insurance Map Within a regulatory floodway? ☒ Yes ☐ No
5. Within a velocity zone? ☐ Yes ☒ No
6. Within a Mapped Wetland Area? ☒ Yes ☐ No If yes, MassGIS Wetland Data Layer: Shrub Swamp
Wetland Type
7. Current Water Resource Conditions (USGS): 5/17/2025 Range: ☒ Above Normal ☐ Normal ☐ Below Normal
Month/Day/ Year
8. Other references reviewed: Not in Zone A or IWPA zones.
(Zone II, IWPA, Zone A, EEA Data Portal, etc.)
Is within Zone II area.



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-105 2025/05/21 11:00am Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use Vacant lot Landscaped area and N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) small trees Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)
Vegetation

Description of Location: Treeline opposite existing shop building

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terrace TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 53" Depth to Weeping in Hole - Depth to Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-53	FILL	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
53-80	C	Fine Sand	10R 7/1	53	Cnc :7.5YR 5/8 Dpl: GLEY1 6/10Y	5%	-	-	Massive	Friable	Trace Silt
				-	Cnc : Dpl:	-	-	-			-
					Cnc : Dpl:	-	-	-	-	-	-
					Cnc : Dpl:						
					Cnc : Dpl:						



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-106 2025/05/21 11:45am Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use: Parking Lot Trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: Near stockpile to rear of site

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terraces TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 70" Depth to Weeping in Hole N/A Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-42	FILL	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
42-79	C	Gravelly Med Sand	10YR 4/1	52	Cnc :7.5YR 5/8 Dpl: GLEY1 6/10Y	5%	-	-	Massive	Friable	Trace Silt
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

D. Determination of High Groundwater Elevation

1. Method Used (Choose one):

☒ Depth to soil redoximorphic features

Obs. Hole # TP-105

53 inches

Obs. Hole # TP-106

52 inches

☐ Depth to observed standing water in observation hole

_____ inches

_____ inches

☐ Depth to adjusted seasonal high groundwater (S_h)
(USGS methodology)

_____ inches

_____ inches

Index Well Number

Reading Date

$$S_h = S_c - [S_r \times (OW_c - OW_{max}) / OW_r]$$

Obs. Hole/Well# _____ S_c _____ S_r _____ OW_c _____ OW_{max} _____ OW_r _____ S_h _____

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

b. If yes, at what depth was it observed (exclude O, A, and E Horizons)?

Upper boundary: 9
inches

Lower boundary: 58
inches

c. If no, at what depth was impervious material observed?

Upper boundary: _____
inches

Lower boundary: _____
inches



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.



Signature of Soil Evaluator

DAVID M. ROBINSON, EIT

Typed or Printed Name of Soil Evaluator / License #

N/A

Name of Approving Authority Witness

05/27/2025

Date

June 30, 2026

Expiration Date of License

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:

See Test Pits Plan, Sheet TPP-1



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

TAG Central LLC

Owner Name

444 E Central Street

Street Address

Franklin

City

MA

State

284-066-000-000

Map/Lot #

02038

Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade
2. Soil Survey Web Soil Survey 254B Merrimac fine sandy loam 3-8% slopes
Source Soil Map Unit Soil Series
Kames, outwash plains, outwash terraces,
moraines, eskers Somewhat excessively drained.
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Landform Soil Limitations
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Soil Parent material
3. Surficial Geological Report 2018 / Stone, Stone, & DiGiacomo-Cohen Coarse Deposits
Year Published/Source Map Unit
consist of gravel deposits, sand and gravel deposits, and sand deposits
Description of Geologic Map Unit:
4. Flood Rate Insurance Map Within a regulatory floodway? ☒ Yes ☐ No
5. Within a velocity zone? ☐ Yes ☒ No
6. Within a Mapped Wetland Area? ☒ Yes ☐ No If yes, MassGIS Wetland Data Layer: Shrub Swamp
Wetland Type
7. Current Water Resource Conditions (USGS): 5/17/2025 Range: ☒ Above Normal ☐ Normal ☐ Below Normal
Month/Day/ Year
8. Other references reviewed: Not in Zone A or IWPA zones.
(Zone II, IWPA, Zone A, EEA Data Portal, etc.)
Is within Zone II area.



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-107 2025/05/21 1:30pm Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use Vacant lot Grassed area and small N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) trees Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)
Vegetation

Description of Location: Treeline opposite existing shop building

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terrace TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 42" Depth to Weeping in Hole $\pm$ Depth to Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-9	A	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
9-58	C	Fine Sandy Loam	5R 4/6	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
				-	Cnc : Dpl:	-	-	-			-
					Cnc : Dpl:	-	-	-	-	-	-
					Cnc : Dpl:						
					Cnc : Dpl:						



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-108 2025/05/20 10:45am Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use: Parking Lot Trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: Near stockpile to rear of site

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terraces TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 64" Depth to Weeping in Hole 64" Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-12	A	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
12-24	Bw	Med Sand	10YR 4/1	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
24-66	C	Med Sand	5R 7/1	-	Cnc :- Dpl: -	-	-	-	Granular	Loose	
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

D. Determination of High Groundwater Elevation

1. Method Used (Choose one):

☐ Depth to soil redoximorphic features

Obs. Hole #TP-107

inches

Obs. Hole #TP-108

inches

☒ Depth to observed standing water in observation hole

42 inches

64 inches

☐ Depth to adjusted seasonal high groundwater (S_h)
(USGS methodology)

_____ inches

_____ inches

Index Well Number

Reading Date

$$S_h = S_c - [S_r \times (OW_c - OW_{max}) / OW_r]$$

Obs. Hole/Well# _____ S_c _____ S_r _____ OW_c _____ OW_{max} _____ OW_r _____ S_h _____

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

b. If yes, at what depth was it observed (exclude O, A, and E Horizons)?

Upper boundary: 9
inches

Lower boundary: 66
inches

c. If no, at what depth was impervious material observed?

Upper boundary: _____
inches

Lower boundary: _____
inches



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator

DAVID M. ROBINSON, EIT

Typed or Printed Name of Soil Evaluator / License #

N/A

Name of Approving Authority Witness

05/27/2025

Date

June 30, 2026

Expiration Date of License

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:

See Test Pit Plan TPP-1



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

TAG Central LLC

Owner Name

444 E Central Street

Street Address

Franklin

City

MA

State

284-066-000-000

Map/Lot #

02038

Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade
2. Soil Survey Web Soil Survey 254B Merrimac fine sandy loam 3-8% slopes
Source Soil Map Unit Soil Series
Kames, outwash plains, outwash terraces,
moraines, eskers Somewhat excessively drained.
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Landform Soil Limitations
Soil Parent material
3. Surficial Geological Report 2018 / Stone, Stone, & DiGiacomo-Cohen Coarse Deposits
Year Published/Source Map Unit
consist of gravel deposits, sand and gravel deposits, and sand deposits
Description of Geologic Map Unit:
4. Flood Rate Insurance Map Within a regulatory floodway? ☒ Yes ☐ No
5. Within a velocity zone? ☐ Yes ☒ No
6. Within a Mapped Wetland Area? ☒ Yes ☐ No If yes, MassGIS Wetland Data Layer: Shrub Swamp
Wetland Type
7. Current Water Resource Conditions (USGS): 5/17/2025 Range: ☒ Above Normal ☐ Normal ☐ Below Normal
Month/Day/ Year
8. Other references reviewed: Not in Zone A or IWPA zones.
(Zone II, IWPA, Zone A, EEA Data Portal, etc.)
Is within Zone II area.



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-110 2025/05/20 11:30am Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use Vacant lot Grassed area and small trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: Treeline westerly path to rear of site

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terrace TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 56" Depth to Weeping in Hole 62" Depth to Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-12	A	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
12-24	Bw	Sandy Loam	5R 4/6	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
24-56	C1	Coarse Sand	2.5YR 6/3	-	Cnc :- Dpl: -	-	-	-	S. Grain	Loose	Gravelly
56-62	C2	Fine Sand	2.5 YR 6/1	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	Gravelly. Grey, gleyed.
					Cnc : Dpl:						
					Cnc : Dpl:						



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-111 2025/05/20 11:45am Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use: Parking Lot Trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: Near stockpile to rear of site

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terraces TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body ± feet Drainage Way ± feet Wetlands ± feet
Property Line ± feet Drinking Water Well ± feet Other ± feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 54" Depth to Weeping in Hole 46" Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-9	A	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
9-28	Bw	FLS	10YR 4/1	-	Cnc :- Dpl: -	-	-	-	Granular	Friable	
28-56	C	Coarse Sand	5R 7/1	36"	Cnc :- 7.5YR5/8 Dpl: - GLEY1 6/10Y	5%	2%	-	S. Grain	Loose	Gravelly
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

D. Determination of High Groundwater Elevation

1. Method Used (Choose one):

☒ Depth to soil redoximorphic features

Obs. Hole # TP-110

_____ inches

Obs. Hole # TP-111

36 inches

☒ Depth to observed standing water in observation hole

56 inches

_____ inches

☐ Depth to adjusted seasonal high groundwater (S_h)
(USGS methodology)

_____ inches

_____ inches

Index Well Number

Reading Date

$$S_h = S_c - [S_r \times (OW_c - OW_{max}) / OW_r]$$

Obs. Hole/Well# _____ S_c _____ S_r _____ OW_c _____ OW_{max} _____ OW_r _____ S_h _____

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

b. If yes, at what depth was it observed (exclude O, A, and E Horizons)?

Upper boundary: 9
inches

Lower boundary: 56
inches

c. If no, at what depth was impervious material observed?

Upper boundary: _____
inches

Lower boundary: _____
inches



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator

DAVID M. ROBINSON, EIT

Typed or Printed Name of Soil Evaluator / License #

N/A

Name of Approving Authority Witness

05/27/2025

Date

June 30, 2026

Expiration Date of License

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:

See Test Pits Plan, Sheet TPP-1



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

TAG Central LLC

Owner Name

444 E Central Street

Street Address

Franklin

City

MA

State

284-066-000-000

Map/Lot #

02038

Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade
2. Soil Survey Web Soil Survey 254B Merrimac fine sandy loam 3-8% slopes
Source Soil Map Unit Soil Series
Kames, outwash plains, outwash terraces,
moraines, eskers Somewhat excessively drained.
Landform Soil Limitations
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Soil Parent material
3. Surficial Geological Report 2018 / Stone, Stone, & DiGiacomo-Cohen Coarse Deposits
Year Published/Source Map Unit
consist of gravel deposits, sand and gravel deposits, and sand deposits
Description of Geologic Map Unit:
4. Flood Rate Insurance Map Within a regulatory floodway? ☒ Yes ☐ No
5. Within a velocity zone? ☐ Yes ☒ No
6. Within a Mapped Wetland Area? ☒ Yes ☐ No If yes, MassGIS Wetland Data Layer: Shrub Swamp
Wetland Type
7. Current Water Resource Conditions (USGS): 5/17/2025 Range: ☒ Above Normal ☐ Normal ☐ Below Normal
Month/Day/ Year
8. Other references reviewed: Not in Zone A or IWPA zones.
(Zone II, IWPA, Zone A, EEA Data Portal, etc.)
Is within Zone II area.



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-112 2025/05/20 12:30pm Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use Vacant lot Grassed area and small trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: Treeline westerly path to rear of site

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terrace TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 42" Depth to Weeping in Hole 72" Depth to Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-12	A	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
12-30	Bw	Fine Sand	5R 4/6	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
30-46	BC	Coarse Sand	2.5YR 6/3	36	Cnc :- 7.5YR5/8 Dpl: - GLEY1 6/10Y	10%	-	-	S. Grain	Loose	-
46-74	C1	Fine Sand	2.5 YR 6/1	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
					Cnc : Dpl:						
					Cnc : Dpl:						



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-113 2025/05/20 1:30pm Cloudy, overcast 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use: Woodland Trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: Near stockpile to rear of site

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terraces TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 74" Depth to Weeping in Hole N/A Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-12	A	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
12-36	Bw	Fine Sand	10YR 4/1	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	Trace Silt
36-78	C	Fine Sand	5R 7/1	36"	Cnc :- 7.5YR5/8 Dpl: - GLEY1 6/10Y	5%	-	-	Massive	Friable	Trace Silt
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

D. Determination of High Groundwater Elevation

1. Method Used (Choose one):

☒ Depth to soil redoximorphic features

Obs. Hole # TP-112

36 inches

Obs. Hole # TP-113

36 inches

☐ Depth to observed standing water in observation hole

_____ inches

_____ inches

☐ Depth to adjusted seasonal high groundwater (S_h)
(USGS methodology)

_____ inches

_____ inches

Index Well Number

Reading Date

$$S_h = S_c - [S_r \times (OW_c - OW_{max}) / OW_r]$$

Obs. Hole/Well# _____ S_c _____ S_r _____ OW_c _____ OW_{max} _____ OW_r _____ S_h _____

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

b. If yes, at what depth was it observed (exclude O, A, and E Horizons)?

Upper boundary: 12
inches

Lower boundary: 78
inches

c. If no, at what depth was impervious material observed?

Upper boundary: _____
inches

Lower boundary: _____
inches



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.



Signature of Soil Evaluator

DAVID M. ROBINSON, EIT

Typed or Printed Name of Soil Evaluator / License #

N/A

Name of Approving Authority Witness

05/27/2025

Date

June 30, 2026

Expiration Date of License

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:

See Test Pit Plan, Sheet TPP-1



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

TAG Central LLC

Owner Name

444 E Central Street

Street Address

Franklin

City

MA

State

284-066-000-000

Map/Lot #

02038

Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade
2. Soil Survey Web Soil Survey 254B Merrimac fine sandy loam 3-8% slopes
Source Soil Map Unit Soil Series
Kames, outwash plains, outwash terraces,
moraines, eskers Somewhat excessively drained.
Landform Soil Limitations
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Soil Parent material
3. Surficial Geological Report 2018 / Stone, Stone, & DiGiacomo-Cohen Coarse Deposits
Year Published/Source Map Unit
consist of gravel deposits, sand and gravel deposits, and sand deposits
Description of Geologic Map Unit:
4. Flood Rate Insurance Map Within a regulatory floodway? ☒ Yes ☐ No
5. Within a velocity zone? ☐ Yes ☒ No
6. Within a Mapped Wetland Area? ☒ Yes ☐ No If yes, MassGIS Wetland Data Layer: Shrub Swamp
Wetland Type
7. Current Water Resource Conditions (USGS): 5/17/2025 Range: ☒ Above Normal ☐ Normal ☐ Below Normal
Month/Day/ Year
8. Other references reviewed: Not in Zone A or IWPA zones.
(Zone II, IWPA, Zone A, EEA Data Portal, etc.)
Is within Zone II area.



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-114 2025/05/20 12:30pm Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use Vacant lot Grassed area and small trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: SW Corner to rear of site

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terrace TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 64" Depth to Weeping in Hole - Depth to Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-10	A	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
10-36	Bw	Fine Sand	5R 4/6	32	Cnc :- 7.5YR5/8 Dpl: - GLEY1 6/10Y	5%	-	-	Massive	Friable	-
36-66	BC	Fine Sand	2.5YR 6/3	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
				-	Cnc :- Dpl: -	-	-	-	-	-	-
					Cnc : Dpl:						
					Cnc : Dpl:						



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-115 2025/05/20 1:00pm Cloudy, overcast 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use: Woodland Trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: Near stockpile to rear of site

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terraces TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 36" Depth to Weeping in Hole 76" Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-10	A	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
10-36	Bw	Fine Sand	10YR 4/1	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
36-78	C	Fine Sand	2.5 YR 7/1	36"	Cnc :- 7.5YR5/8 Dpl: - GLEY1 6/10Y	5%	-	-	Massive	Friable	Trace Silt
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

D. Determination of High Groundwater Elevation

1. Method Used (Choose one):

☒ Depth to soil redoximorphic features

Obs. Hole # TP-114

32 inches

Obs. Hole # TP-115

36 inches

☐ Depth to observed standing water in observation hole

_____ inches

_____ inches

☐ Depth to adjusted seasonal high groundwater (S_h)
(USGS methodology)

_____ inches

_____ inches

Index Well Number

Reading Date

$$S_h = S_c - [S_r \times (OW_c - OW_{max}) / OW_r]$$

Obs. Hole/Well# _____ S_c _____ S_r _____ OW_c _____ OW_{max} _____ OW_r _____ S_h _____

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

b. If yes, at what depth was it observed (exclude O, A, and E Horizons)?

Upper boundary: 10
inches

Lower boundary: 78
inches

c. If no, at what depth was impervious material observed?

Upper boundary: _____
inches

Lower boundary: _____
inches



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.



Signature of Soil Evaluator

DAVID M. ROBINSON, EIT

Typed or Printed Name of Soil Evaluator / License #

N/A

Name of Approving Authority Witness

05/27/2025

Date

June 30, 2026

Expiration Date of License

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:

See Test Pits Plan Sheet TPP-1



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

TAG Central LLC

Owner Name

444 E Central Street

Street Address

Franklin

City

MA

State

284-066-000-000

Map/Lot #

02038

Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade
2. Soil Survey Web Soil Survey 254B Merrimac fine sandy loam 3-8% slopes
Source Soil Map Unit Soil Series
Kames, outwash plains, outwash terraces, Somewhat excessively drained.
moraines, eskers Soil Limitations
Landform
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Soil Parent material
3. Surficial Geological Report 2018 / Stone, Stone, & DiGiacomo-Cohen Coarse Deposits
Year Published/Source Map Unit
consist of gravel deposits, sand and gravel deposits, and sand deposits
Description of Geologic Map Unit:
4. Flood Rate Insurance Map Within a regulatory floodway? ☒ Yes ☐ No
5. Within a velocity zone? ☐ Yes ☒ No
6. Within a Mapped Wetland Area? ☒ Yes ☐ No If yes, MassGIS Wetland Data Layer: Shrub Swamp
Wetland Type
7. Current Water Resource Conditions (USGS): 5/17/2025 Range: ☒ Above Normal ☐ Normal ☐ Below Normal
Month/Day/ Year
8. Other references reviewed: Not in Zone A or IWPA zones.
(Zone II, IWPA, Zone A, EEA Data Portal, etc.)
Is within Zone II area.



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-116 2025/05/20 11:30am Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use Vacant lot Grassed area N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: Grassed area near green house

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terrace TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 38" Depth to Weeping in Hole - Depth to Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-12	A	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
12-24	Bw	Fine Sand	5R 4/6	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
24-66	C	Coarse Sand	2.5YR 6/3	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	-
				-	Cnc :- Dpl: -	-	-	-			-
					Cnc : Dpl:						
					Cnc : Dpl:						

Additional Notes:



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review (*minimum of two holes required at every proposed primary and reserve disposal area*)

Deep Observation Hole Number: TP-117 2025/05/20 1:30pm Cloudy, overcast 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use: Woodland Trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: Near stockpile to rear of site

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terraces TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body ± feet Drainage Way ± feet Wetlands ± feet
Property Line ± feet Drinking Water Well ± feet Other ± feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: Depth to Weeping in Hole Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-24	A	FSL	2.5YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
24-36	Bw	Fine Sandy Loam	10YR 4/1	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
36-68	C	Fine Loamy Sand	5R 7/1	48"	Cnc :- 7.5YR5/8 Dpl: - GLEY1 6/10Y	5%	-	-	Massive	Friable	Trace Silt
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						

Additional Notes:



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

D. Determination of High Groundwater Elevation

1. Method Used (Choose one):

☒ Depth to soil redoximorphic features

Obs. Hole # TP-116

_____ inches

Obs. Hole # TP-117

48 inches

☒ Depth to observed standing water in observation hole

38 inches

_____ inches

☐ Depth to adjusted seasonal high groundwater (S_h)
(USGS methodology)

_____ inches

_____ inches

Index Well Number _____

Reading Date _____

$$S_h = S_c - [S_r \times (OW_c - OW_{max}) / OW_r]$$

Obs. Hole/Well# _____ S_c _____ S_r _____ OW_c _____ OW_{max} _____ OW_r _____ S_h _____

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

b. If yes, at what depth was it observed (exclude O, A, and E Horizons)?

Upper boundary: 12
inches

Lower boundary: 68
inches

c. If no, at what depth was impervious material observed?

Upper boundary: _____
inches

Lower boundary: _____
inches



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator

DAVID M. ROBINSON, EIT

Typed or Printed Name of Soil Evaluator / License #

N/A

Name of Approving Authority Witness

05/27/2025

Date

June 30, 2026

Expiration Date of License

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:

See Test Pits Plan Sheet TPP-1



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

TAG Central LLC

Owner Name

444 E Central Street

Street Address

Franklin

City

MA

State

284-066-000-000

Map/Lot #

02038

Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade
2. Soil Survey Web Soil Survey 254B Merrimac fine sandy loam 3-8% slopes
Source Soil Map Unit Soil Series
Kames, outwash plains, outwash terraces, Somewhat excessively drained.
moraines, eskers Soil Limitations
Landform
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Soil Parent material
3. Surficial Geological Report 2018 / Stone, Stone, & DiGiacomo-Cohen Coarse Deposits
Year Published/Source Map Unit
consist of gravel deposits, sand and gravel deposits, and sand deposits
Description of Geologic Map Unit:
4. Flood Rate Insurance Map Within a regulatory floodway? ☒ Yes ☐ No
5. Within a velocity zone? ☐ Yes ☒ No
6. Within a Mapped Wetland Area? ☒ Yes ☐ No If yes, MassGIS Wetland Data Layer: Shrub Swamp
Wetland Type
7. Current Water Resource Conditions (USGS): 12/17/2023 Range: ☒ Above Normal ☐ Normal ☐ Below Normal
Month/Day/ Year
8. Other references reviewed: Not in Zone A or IWPA zones.
(Zone II, IWPA, Zone A, EEA Data Portal, etc.)
Is within Zone II area.



C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

1. Land Use	Vacant lot (e.g., woodland, agricultural field, vacant lot, etc.)	Landscaped area and small trees Vegetation	some stones nearby Surface Stones (e.g., cobbles, stones, boulders, etc.)	0-3 Slope (%)

3. Distances from:

Open Water Body	± feet	Drainage Way	± feet	Wetlands	± feet
Property Line	+ feet	Drinking Water Well	+ feet	Other	+ feet

5. Groundwater Observed: ☒ Yes ☐ No If yes: 27" Depth to Weeping in Hole 84" Depth to Standing Water in Hole

[illegible]



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: STP-2 12/6/2023 10:55 Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use: Vacant lot Trees N/A 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: NW of STP-3, South of STP-1.

2. Soil Parent Material: Loamy glaciofluvial deposits Outwash Terraces TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 40" Depth to Weeping in Hole 108" Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-6	A	FSL	10YR 3/1	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
6-37	Ab	Medium Loamy Sand	10YR 4/1	-	Cnc :- Dpl: -	-	15-35	-	S. Grain	Loose	
37-114	C1	Fine Loamy Sand	10YR 7/1	40"	Cnc :7.5YR 5/8 Dpl: GLEY1 6/10Y	2%	15-35	15-35	S. Grain	Loose	
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

Additional Notes:

D. Determination of High Groundwater Elevation

1. Method Used (Choose one):

☒ Depth to soil redoximorphic features

Obs. Hole # STP-1

22 inches

Obs. Hole # STP-2

40 inches

☐ Depth to observed standing water in observation hole

_____ inches

_____ inches

☐ Depth to adjusted seasonal high groundwater (S_h)
(USGS methodology)

_____ inches

_____ inches

Index Well Number

Reading Date

$$S_h = S_c - [S_r \times (OW_c - OW_{max}) / OW_r]$$

Obs. Hole/Well# _____ S_c _____ S_r _____ OW_c _____ OW_{max} _____ OW_r _____ S_h _____

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

b. If yes, at what depth was it observed (exclude O, A, and E Horizons)?

Upper boundary: 6
inches

Lower boundary: 114
inches

c. If no, at what depth was impervious material observed?

Upper boundary: _____
inches

Lower boundary: _____
inches



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator

Carlton Quinn, PE

Typed or Printed Name of Soil Evaluator / License #

12/18/2023

Date

June 30, 2024

Expiration Date of License

Name of Approving Authority Witness

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

TAG Central LLC

Owner Name

444 E Central Street

Street Address

Franklin

City

MA

State

284-066-000-000

Map/Lot #

02038

Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade
2. Soil Survey Web Soil Survey 254B Merrimac fine sandy loam 3-8% slopes
Source Soil Map Unit Soil Series
Kames, outwash plains, outwash terraces, Somewhat excessively drained.
moraines, eskers Soil Limitations
Landform
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Soil Parent material
3. Surficial Geological Report 2018 / Stone, Stone, & DiGiacomo-Cohen Coarse Deposits
Year Published/Source Map Unit
consist of gravel deposits, sand and gravel deposits, and sand deposits
Description of Geologic Map Unit:
4. Flood Rate Insurance Map Within a regulatory floodway? ☒ Yes ☐ No
5. Within a velocity zone? ☐ Yes ☒ No
6. Within a Mapped Wetland Area? ☒ Yes ☐ No If yes, MassGIS Wetland Data Layer: Shrub Swamp
Wetland Type
7. Current Water Resource Conditions (USGS): 12/17/2023 Range: ☒ Above Normal ☐ Normal ☐ Below Normal
Month/Day/ Year
8. Other references reviewed: Not in Zone A or IWPA zones.
(Zone II, IWPA, Zone A, EEA Data Portal, etc.)
Is within Zone II area.



**Commonwealth of Massachusetts
City/Town of Franklin**

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: STP-3 12/6/2023 2:33 Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use Vacant Lot Trees Crushed stone nearby 0-3
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)
Description of Location: SE of STP-2, NW of STP-4

2. Soil Parent Material: Loamy glaciofluvial deposits outwash terrace TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body ± feet Drainage Way ± feet Wetlands ± feet
Property Line ± feet Drinking Water Well ± feet Other ± feet

4. Unsuitable Materials Present: ☒ Yes ☐ No If Yes: ☒ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 101" Depth to Weeping in Hole - Depth to Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-7	A	Fine Sandy Loam	10YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
7-24	Ab	Coarse Sandy Loam	10YR 5/2	-	Cnc :- Dpl: -	-	15-35	-	Massive	Friable	Fill Layer
24-45	Ab2	Sandy Loam	10YR 2/1	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	Fill Layer
45-112	C	Fine Sand	10YR 7/1	45"	Cnc :7.5YR 5/8 Dpl: GLEY1 6/10Y	2%	15-35	15-35	S. Grain	Loose	Boulder at 112"
					Cnc : Dpl:						
					Cnc : Dpl:						

Additional Notes:



C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

-71.377240
Longitude

- Description of Location: SE of STP-3

3. Distances from:
- | | | | | | |
|-----------------|--------|---------------------|--------|----------|--------|
| Open Water Body | ± feet | Drainage Way | ± feet | Wetlands | ± feet |
| Property Line | + feet | Drinking Water Well | + feet | Other | + feet |

4. Unsuitable Materials Present: ☒ Yes ☐ No If Yes: ☒ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 72" Depth to Weeping in Hole 107" Depth Standing Water in Hole

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-4	A	FSL	10YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
4-29	Ab	SL	10YR3/3	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	Fill Layer
29-37	C1	Fine Sand	10YR 5/3	-	Cnc :- Dpl: -	-	-	-	S. Grain	Loose	
37-38	Ab2	FSL	10YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
38-46	C2	Sand	10YR 5/2	-	Cnc :- Dpl: -	-	15-35	15-35	S. Grain	Loose	
46-120	2C	FLS	10YR 7/1	72"	Cnc :7.5YR 5/8 Dpl: GLEY1 6/10Y	2%	15-35	15-35	Massive	Friable	

STP 3-4



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

D. Determination of High Groundwater Elevation

1. Method Used (Choose one):

☒ Depth to soil redoximorphic features

Obs. Hole # STP-3

45 inches

Obs. Hole # STP-4

72 inches

☐ Depth to observed standing water in observation hole

_____ inches

_____ inches

☐ Depth to adjusted seasonal high groundwater (S_h)
(USGS methodology)

_____ inches

_____ inches

Index Well Number _____

Reading Date _____

$$S_h = S_c - [S_r \times (OW_c - OW_{max}) / OW_r]$$

Obs. Hole/Well# _____ S_c _____ S_r _____ OW_c _____ OW_{max} _____ OW_r _____ S_h _____

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

b. If yes, at what depth was it observed (exclude O, A, and E Horizons)?

Upper boundary: 45
inches

Lower boundary: 112
inches

c. If no, at what depth was impervious material observed?

Upper boundary: _____
inches

Lower boundary: _____
inches



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator

Carlton Quinn, PE

Typed or Printed Name of Soil Evaluator / License #

12/18/2023

Date

June 30, 2024

Expiration Date of License

Name of Approving Authority Witness

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

A. Facility Information

TAG Central LLC

Owner Name

444 E Central Street

Street Address

Franklin

City

MA

State

284-066-000-000

Map/Lot #

02038

Zip Code

B. Site Information

1. (Check one) ☒ New Construction ☐ Upgrade
2. Soil Survey Web Soil Survey 254B Merrimac fine sandy loam 3-8% slopes
Source Soil Map Unit Soil Series
Kames, outwash plains, outwash terraces, Somewhat excessively drained.
moraines, eskers Soil Limitations
Landform
loamy glaciofluvial deposits derived from granite, schist and gneiss.
Soil Parent material
3. Surficial Geological Report 2018 / Stone, Stone, & DiGiacomo-Cohen Coarse Deposits
Year Published/Source Map Unit
consist of gravel deposits, sand and gravel deposits, and sand deposits
Description of Geologic Map Unit:
4. Flood Rate Insurance Map Within a regulatory floodway? ☒ Yes ☐ No
5. Within a velocity zone? ☐ Yes ☒ No
6. Within a Mapped Wetland Area? ☒ Yes ☐ No If yes, MassGIS Wetland Data Layer: Shrub Swamp
Wetland Type
7. Current Water Resource Conditions (USGS): 12/17/2023 Range: ☒ Above Normal ☐ Normal ☐ Below Normal
Month/Day/ Year
8. Other references reviewed: Not in Zone A or IWPA zones.
(Zone II, IWPA, Zone A, EEA Data Portal, etc.)
Is within Zone II area.



Commonwealth of Massachusetts
City/Town of Franklin

Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

C. On-Site Review *(minimum of two holes required at every proposed primary and reserve disposal area)*

Deep Observation Hole Number: STP-5 12/6/2023 1:20 Cloudy 42.079070 -71.377240
Hole # Date Time Weather Latitude Longitude

1. Land Use Vacant Lot Trees Some stones, light boulders 3-8
(e.g., woodland, agricultural field, vacant lot, etc.) Vegetation Surface Stones (e.g., cobbles, stones, boulders, etc.) Slope (%)

Description of Location: SW of STP-1.

2. Soil Parent Material: Loamy glaciofluvial deposits outwash terrace TS
Landform Position on Landscape (SU, SH, BS, FS, TS, Plain)

3. Distances from: Open Water Body $\pm$ feet Drainage Way $\pm$ feet Wetlands $\pm$ feet
Property Line $\pm$ feet Drinking Water Well $\pm$ feet Other $\pm$ feet

4. Unsuitable Materials Present: ☐ Yes ☒ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☒ Yes ☐ No If yes: 48" Depth to Weeping in Hole 107" Depth to Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-12	A	FSL	10YR 3/2	-	Cnc : - Dpl: -	-	-	-	Massive	Friable	
12-107	C	FLS	10YR 7/1	48"	Cnc : 7.5YR 5/8 Dpl: GLEY1 6/10Y	2%	-	-	S. Grain	Loose	
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						
					Cnc : Dpl:						

Additional Notes:



C. On-Site Review (minimum of two holes required at every proposed primary and reserve disposal area)

-71.377240
Longitude

- Description of Location: West side of stream, near dry vegetation patch.

- | | | | | | | |
|--------------------|-----------------|--------|---------------------|--------|----------|--------|
| 3. Distances from: | Open Water Body | ± feet | Drainage Way | ± feet | Wetlands | ± feet |
| | Property Line | + feet | Drinking Water Well | + feet | Other | + feet |

4. Unsuitable Materials Present: ☐ Yes ☐ No If Yes: ☐ Disturbed Soil/Fill Material ☐ Weathered/Fractured Rock ☐ Bedrock

5. Groundwater Observed: ☐ Yes ☒ No If yes: - Depth to Weeping in Hole - Depth Standing Water in Hole

Soil Log

Depth (in)	Soil Horizon /Layer	Soil Texture (USDA)	Soil Matrix: Color-Moist (Munsell)	Redoximorphic Features			Coarse Fragments % by Volume		Soil Structure	Soil Consistence (Moist)	Other
				Depth	Color	Percent	Gravel	Cobbles & Stones			
0-8	A	SL	10YR 3/2	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
8-24	Bw	SL	5YR 5/6	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
24-31	C	Coarse Sand	10YR 5/3		Cnc :- Dpl: -	-	-	-	S. Grain	Loose	
31-40	Bw2	SL	5YR 5/8	-	Cnc :- Dpl: -	-	-	-	Massive	Friable	
40-114	2C	LS	GLE Y1 6/10Y	-	Cnc :- Dpl: -	-	-	-	S. Grain	Loose	
					Cnc : Dpl:						

Additional Notes:



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

D. Determination of High Groundwater Elevation

1. Method Used (Choose one):

☒ Depth to soil redoximorphic features

Obs. Hole # STP-5

48 inches

Obs. Hole # STP-6

N/A inches

☐ Depth to observed standing water in observation hole

_____ inches

_____ inches

☐ Depth to adjusted seasonal high groundwater (S_h)
(USGS methodology)

_____ inches

_____ inches

Index Well Number _____

Reading Date _____

$$S_h = S_c - [S_r \times (OW_c - OW_{max}) / OW_r]$$

Obs. Hole/Well# _____ S_c _____ S_r _____ OW_c _____ OW_{max} _____ OW_r _____ S_h _____

E. Depth of Pervious Material

1. Depth of Naturally Occurring Pervious Material

a. Does at least four feet of naturally occurring pervious material exist in all areas observed throughout the area proposed for the soil absorption system?

☒ Yes ☐ No

b. If yes, at what depth was it observed (exclude O, A, and E Horizons)?

Upper boundary: 12
inches

Lower boundary: 114
inches

c. If no, at what depth was impervious material observed?

Upper boundary: _____
inches

Lower boundary: _____
inches



Form 11 - Soil Suitability Assessment for On-Site Sewage Disposal

F. Certification

I certify that I am currently approved by the Department of Environmental Protection pursuant to 310 CMR 15.017 to conduct soil evaluations and that the above analysis has been performed by me consistent with the required training, expertise and experience described in 310 CMR 15.017. I further certify that the results of my soil evaluation, as indicated in the attached Soil Evaluation Form, are accurate and in accordance with 310 CMR 15.100 through 15.107.

Signature of Soil Evaluator

Carlton Quinn, PE

Typed or Printed Name of Soil Evaluator / License #

12/18/2023

Date

June 30, 2024

Expiration Date of License

Name of Approving Authority Witness

Approving Authority

Note: In accordance with 310 CMR 15.018(2) this form must be submitted to the approving authority within 60 days of the date of field testing, and to the designer and the property owner with [Percolation Test Form 12](#).

Field Diagrams: Use this area for field diagrams:

KEVIN M. MARTIN, P.E.
KMM GEOTECHNICAL CONSULTANTS, LLC
7 Marshall Road
Hampstead, NH 03841
603-489-5556 (p)/ 603-489-5558 (f)/781-718-4084(m)
kevinmartinpe@aol.com

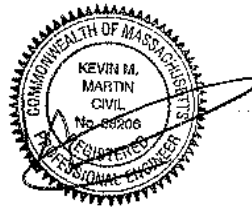
MEMORANDUM

TO: A.J. Alevizos
TAG Central LLC
275 Regatta Drive
Jupiter, FL 33477

FROM: Kevin M. Martin, P.E.
Geotechnical Engineer

DATE: November 12, 2024

RE: **GEOTECHNICAL SUMMARY REPORT
PROPOSED RESIDENTIAL BUILDINGS
444 EAST CENTRAL STREET
FRANKLIN, MASSACHUSETTS**



This memorandum serves as a Geotechnical Summary Report for the referenced project. The contents of this memorandum are subject to the attached ***Limitations***.

SITE & PROJECT DESCRIPTION

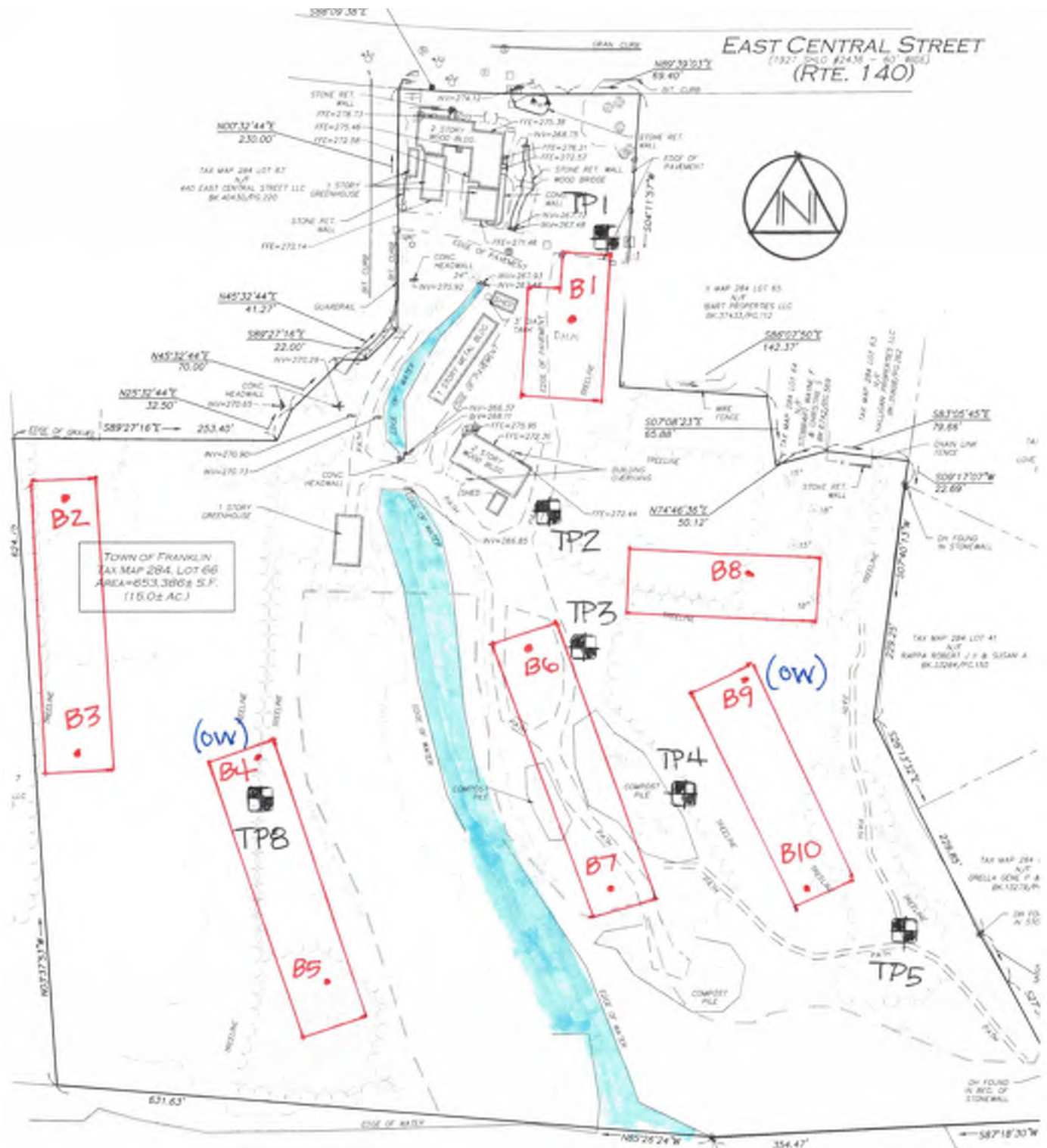
Present site development includes a landscape/ garden center with various buildings and garages. Present construction and use will be razed to accommodate the project. KMM has no knowledge of past construction, use and/or development of the property except what is visible or shown on the *Site Plan*. Based on the *Site Plan*, grades across the site vary from elevation ≈ 280 -268 ft possessing gradual downward contour to the rear (south). The site is intersected by Uncas Brook.

The project includes a new residential apartment complex. The complex will include five (5) apartment buildings and a clubhouse. The apartment buildings will include 4-story, wood-framed, multi-unit residential structures. It is intended to support the buildings using a conventional spread footing foundation (no basement). Expansive Fill (≈ 5 -6 ft) will be required to achieve final grade.

The purpose of this study is to review the subgrade conditions and provide a geotechnical evaluation related to foundation design and construction as required by the *Massachusetts State Building Code*. This report does not include an environmental assessment relative to oil, gasoline, solid waste and/or other hazardous materials. The environmental conditions of the property should be addressed by others as necessary. This study also does not include review of site design or construction issues

such as infiltration systems, dry wells, retaining walls, excavation support, underground utilities, temporary shoring, water-proofing, septic systems, protection of surrounding buildings/utilities, pools, crane pads or other site and/or temporary design unless specifically addressed herein.





SITE PLAN - TEST BORE LOCATIONS



INTERSECTING UNCAS BROOK

SUBSURFACE EXPLORATION PROGRAM

Test Borings

The exploration program for the project included ten (10) test borings around the building pads where accessible. The test borings (B1 to B10) were advanced to depths of ≈ 8 -22 utilizing 3 inch pneumatically driven casing. All the test bores to the east (B1, B6, B7, B8, B9 & B10) met refusal at depths of ≈ 8 -18 ft. Soil samples were typically retrieved at no greater than 5 ft intervals with a 2 inch diameter split-spoon sampler. Standard Penetration Tests (SPTs) were performed at the sampling intervals in general accordance with ASTM-D1586 (*Standard Method for Penetration Test and Split-Barrel Sampling of Soils*). Rock coring was not completed for this study. Offset test holes were advanced in some cases to further review depth to refusal. Field descriptions and penetration resistance of the soils encountered, observed depth to groundwater, depth to refusal and other pertinent data are contained on the attached *Test Boring Logs*. The attached *Sketch* shows the test bore locations. Ground elevations were extrapolated from the *Site Plan*.

Observation Wells

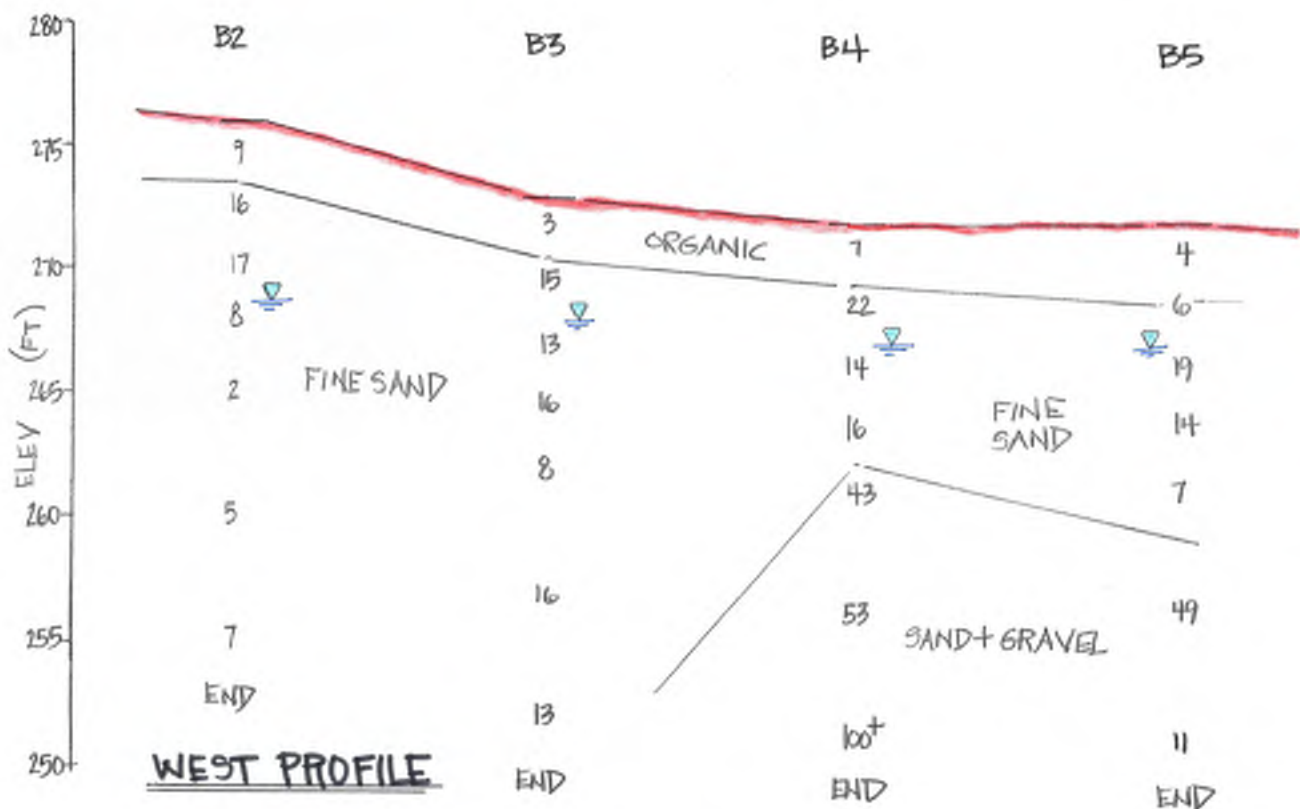
Observation Wells were installed to ≈ 13 ft depth at B4 & B9. The wells include 2 inch PVC well screen and riser with requisite end plugs, filter sand, bentonite seal and expansion plugs. The wells may be used to gauge seasonal fluctuations.

Test Pits by Others

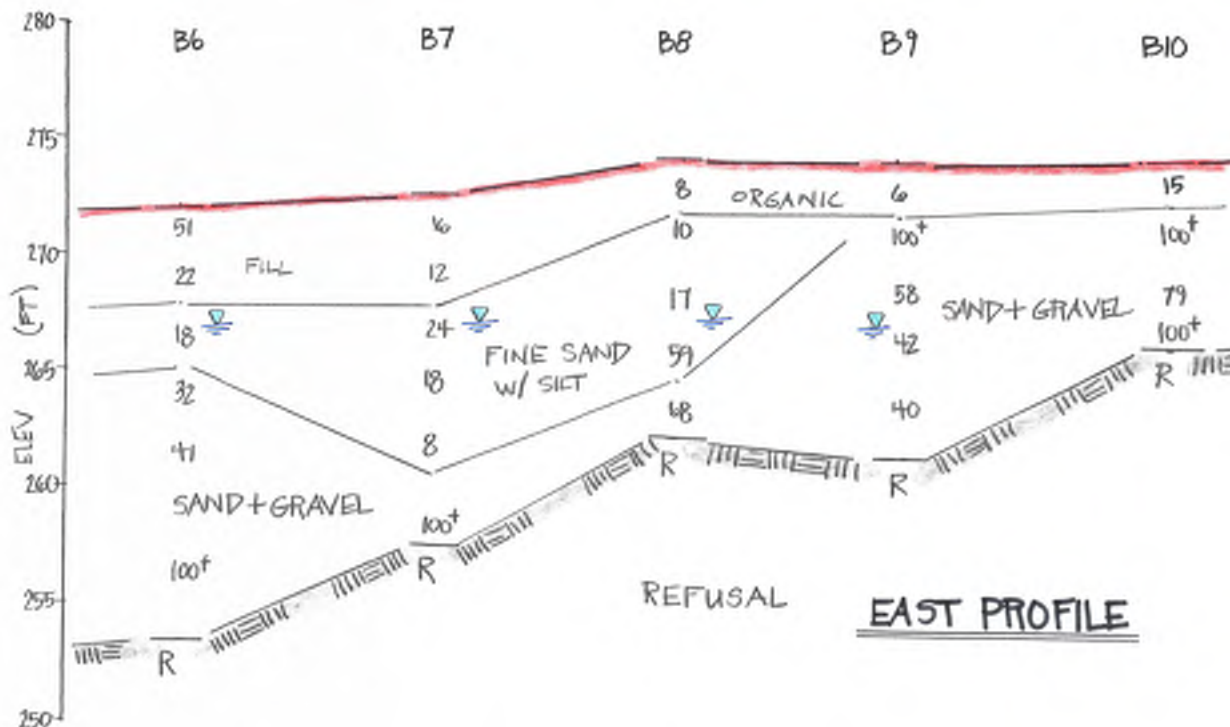
The *Site Plan* includes six (6) *Test Pit Logs* excavated in December 2023 for site design purposes to depths of ≈ 9 -10 ft. The Logs were reviewed as considered relevant to this study.

SUBSURFACE CONDITIONS

The subsurface conditions below (1) Fill and/or Organic laden soils include (2) Fluvial soils, (3) Granular Outwash then (4) apparent Bedrock Refusal. The subgrade conditions are markedly different along the East & West of Uncas Brook. Two (2) *Subsurface Profiles* (East & West) depicting the soil and groundwater conditions are attached for review.



SUBSURFACE PROFILE - WEST



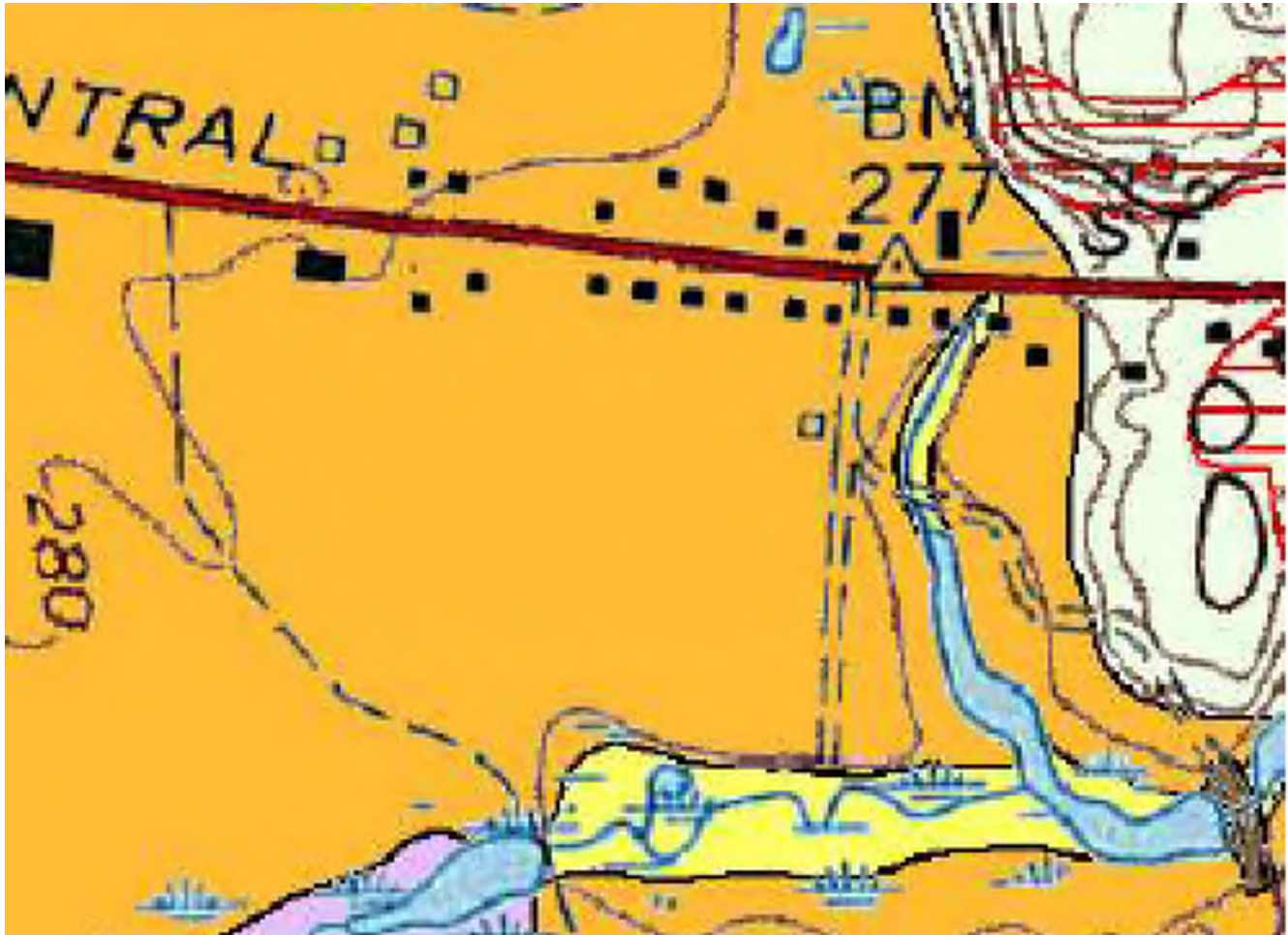
SUBSURFACE PROFILE - EAST

Fill and/or Organic laden soils were encountered to depths of $\approx 2-4$ ft across the site. Topsoil and Loamy Subsoil are present in most areas to depths of $\approx 1-2$ ft. Fill is also present in areas to depths of $\approx 2-5$ ft. Fill should be expected given stockpiles, foundations, intersecting utilities, past grading, etc. Fill was also identified on some of the *Test Pit Logs*. The Fill was more noticeable along the stockpile areas to the south (B6-B7-TP3-TP4).

Finer-grained Fluvial soils were identified at shallower depths being more predominate to the west. A Fine Sand, trace to little silt was encountered to depths of ≈ 10 ft to greater than 22 ft to the west. A Fine Sand & Silt was encountered to shallow depths of $\approx 7-10$ ft to the east. The Fine Sand varies from loose to medium dense to the west whereas the Fine Sand & Silt is generally more compact. The Fine Sand & Silt is rendered moisture sensitive, poor-draining and frost susceptible.

Stratified Outwash was identified at most locations especially to the east and south. The Granular Outwash generally includes a Sand & Gravel, cobbles. These soils are dense, granular, well-draining, stable and compact.

Test Bore Refusal, presumably Bedrock, was encountered at depths of $\approx 8-18$ ft in all the bores to the east. The other test bores (to the west) were terminated at 22 ft without refusal. The depth to refusal becomes progressively shallower to the south. Bedrock was not encountered in the test pits to $\approx 9-10$ ft depth. Bedrock in the area is characteristically hard and of sound quality.



USGS SURFICIAL GEOLOGIC MAP - FRANKLIN, MASS - 2018



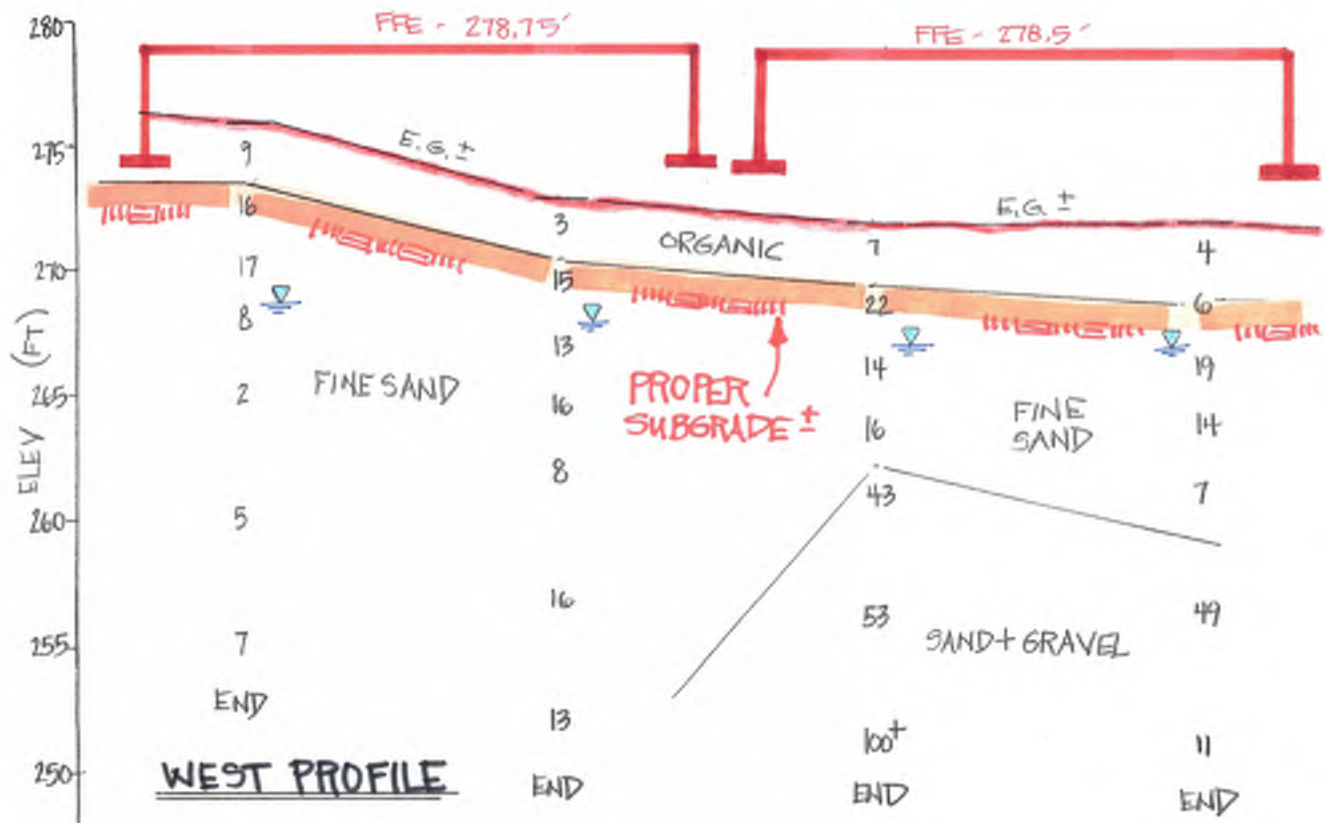
Coarse deposits consist of *gravel deposits*, *sand and gravel deposits*, and *sand deposits*, not differentiated in this report. *Gravel deposits* are composed of at least 50 percent gravel-size clasts; cobbles and boulders predominate; minor amounts of sand occur within gravel beds, and sand comprises a few separate layers. Gravel layers generally are poorly sorted, and bedding commonly is distorted and faulted due to postdepositional collapse related to melting of ice. *Sand and gravel deposits* occur as mixtures of gravel and sand within individual layers and as layers of sand alternating with layers of gravel. Sand and gravel layers generally range between 25 and 50 percent gravel particles and between 50 and 75 percent sand particles. Layers are well sorted to poorly sorted; bedding may be distorted and faulted due to postdepositional collapse. *Sand deposits* are composed mainly of very coarse to fine sand, commonly in well-sorted layers. Coarser layers may contain up to 25 percent gravel particles, generally granules and pebbles; finer layers may contain some very fine sand, silt, and clay

The *USGS Surficial Geologic Map* identifies post glacial stratified Outwash in the low site area to include Coarse deposits with shallow bedrock to the east. These conditions are consistent with this study.

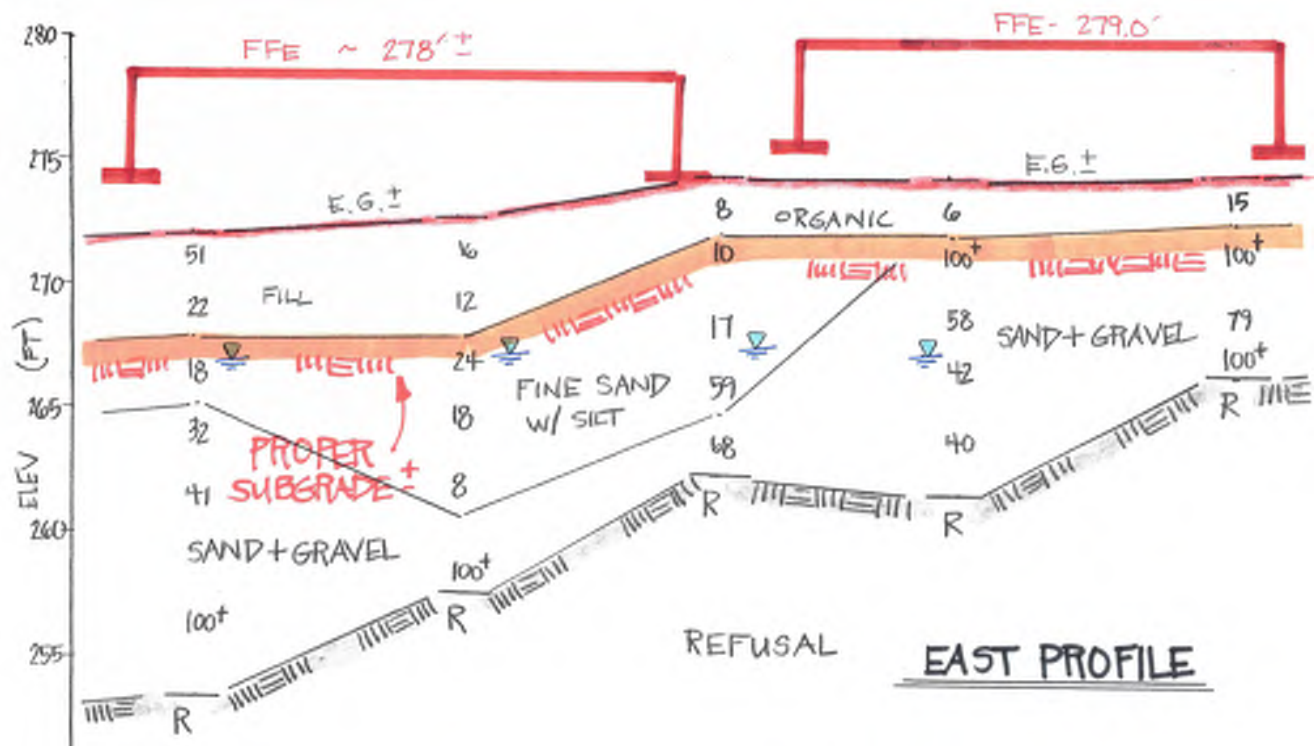
Groundwater was encountered in the test holes at depths of ≈ 5 -8 ft below grade (typically 5-6 ft). Wet and saturated soils were encountered at these depths. These correspond to elevations ≈ 167 -169 ft. It should be noted that fluctuations in the level of the groundwater may occur due to variations in rainfall, temperature, flooding, utilities and other factors differing from the time of the measurements. This study was completed at a time of seasonally low groundwater.

FOUNDATION SUBGRADE RECOMMENDATIONS

The subgrade conditions are favorable for supporting the proposed building on a conventional spread footing foundation with a concrete floor slab. The undocumented Fill and Organic soils, however, are **not** considered suitable for foundation support given questionable strength and compressibility. As such, these soils as well as intersecting utilities, abandoned foundations and other questionable materials shall be fully removed from the building pads including the *Footing Zone of Influence (FZOI)* to expose the Glacial subgrade. The *FZOI* is defined as that area extending laterally one foot from the edge of footing then outward and downward at a 1H:1V splay (up to ≈ 4 ft laterally beyond the edge of footing). The attached *Profiles* illustrates "Proper Subgrade" in relation to the proposed construction. Structural Fill (Table 1) should be used below the foundation as necessary.



CONCEPTUAL SPREAD FOOTING FOUNDATION



CONCEPTUAL SPREAD FOOTING FOUNDATION

The parent subgrade soils (Fluvial/Glacial) should be exposed in the foundation areas prior to casting the footings or placing structural fill. It is recommended that the parent subgrade soils be proof-rolled with vibratory densification and exhibit stable and compact conditions. The purpose of the proof-rolling is to densify the site soils and identify potential loose or unstable areas which should be removed as necessary. Proof-rolling should involve at least 4-5 passes with a vibratory compactor (minimum 850 pound static weight) operating at peak energy. During the proof rolling process, the subgrade should be observed by an Engineer to identify areas exhibiting weaving or instability. It will be necessary to remove weakened or unstable soils and replace with a Structural Fill or crushed stone. Proof-rolling should not be used when the subgrade is wet (groundwater, storm water, perched water, etc) as this may result in soil pumping and instability. The contractor should exercise extra precaution to minimize subgrade disturbance in these wet areas. Specifically, the groundwater should be continuously maintained at least one foot below construction grade until the backfilling is complete. A base of $\frac{3}{4}$ -inch minus crushed stone (or larger graded stone) should be placed atop the earthen subgrade if wet conditions and groundwater seepage are present. The stone should be *immediately* placed atop the undisturbed subgrade then tamped with a plate compactor exhibiting stable conditions. The purpose of the stone base is to protect the wet subgrade, facilitate necessary dewatering and provide a dry/stable base upon which to progress foundation construction. Groundwater is typically more problematic if construction occurs during the wetter winter or spring season. The drier summer months are more favorable for groundwater control. Proper groundwater control and storm water management are also necessary to maintain site stability.

The subgrade should ultimately be stable, dewatered, compact and protected from frost throughout construction. Bearing subgrades that become weakened or disturbed due to wet conditions will be rendered unsuitable for structural support. The Contractor shall ultimately be responsible for the means and methods of temporary groundwater control, subgrade protection and site stability during construction. An Engineer from KMM should be scheduled to review the foundation subgrade conditions and preparation during construction.

FOUNDATION DESIGN RECOMMENDATIONS

The footings are expected to gain bearing support atop the parent soils and/or compacted structural fill. Footings may be designed using an allowable bearing capacity of 4 ksf (FS=3). The allowable bearing capacity may be increased a third ($\frac{1}{3}$) when considering transient loads such as wind or seismic. The bearing capacity is contingent upon the perimeter strip footings and isolated column footings being no less than 2 ft and 3 ft in width respectively. For footings less than 3 ft in lateral dimension, the allowable bearing capacity should be reduced to one-third and multiplied by the least lateral footing dimension in feet. Foundation settlement should be less than 1 inch with differential settlement less than $\frac{1}{2}$ inch. The settlement should be elastic and occur during construction. Exterior footings shall be provided with at least 4 ft of frost protection.

The subsurface conditions were reviewed with respect to seismic criteria set forth in the *Massachusetts State Building Code*. Based on the relative density of the soils and the depth to groundwater, the site does not appear susceptible to liquefaction in the event of an earthquake. Based on interpretation of the *Building Code*, the *Site Classification* is “D” (Stable Soil).

It is recommended that a minimum 6-inch base of *Gravel Base Fill* (Table 1) be placed below the concrete floor slabs for moisture, resilient strength and frost control. The gravel base shall be no less than 12 inches for exterior concrete slabs exposed to frost. A subgrade modulus of 150 pci may be used for design of the floor slab. A vapor retarder should be used below the concrete floor slab dependent upon the floor treatment. A vapor barrier should be specified by others per ACI Standards. A 10-mil StegoWrap™ or equal are often used as a vapor retarder.

Structural fill necessary within and below the foundation should also conform to the attached *Specifications* (Table 1). Expansive Fill will be required for site grading.

CONSTRUCTION CONCERNS

The contractor should be required to maintain a stable-dewatered subgrade for the building foundation and other concerned areas during construction. Subgrade disturbance may be influenced by excavation methods, moisture, precipitation, groundwater control and construction activities. The Granular Outwash soils are not considered vulnerable to disturbance when exposed to wet conditions and construction activities given their good drainage. Steady groundwater seepage, however, will likely disturb these soils if not properly managed during construction. The finer-grained Fluvial soils are considered moisture sensitive given poor drainage. The contractor should take precautions to reduce subgrade disturbance. Such precautions may include diverting storm run-off away from construction areas, reducing traffic in sensitive areas, minimizing the extent of exposed subgrade if inclement weather is forecast, backfilling footings as soon as practicable and maintaining an

effective dewatering program. Soils exhibiting weaving or instability should be over-excavated to a competent bearing subgrade then replaced with a free draining structural fill or crushed stone. The moisture concerns are typically more problematic if construction takes place during the winter to spring season or other periods of inclement weather. A protective base of $\frac{3}{4}$ -inch minus crushed stone may be placed at least ≈ 6 inches below and laterally beyond the footing limits for protection during construction. The stone base is to protect the site soils, facilitate necessary dewatering and provide a dry/stable base upon which to progress foundation construction. The protective base should be considered elective and dependent upon the site conditions. The stone base should be considered necessary if wet conditions are present at footing grade. The protective stone base shall be tamped with a plate compactor and exhibit stable conditions.

The groundwater table will need to be temporarily controlled during construction to complete work in dry conditions and protect the competency of the subgrade. The groundwater table should be continuously maintained at least one foot below construction grade until backfilling is complete. The groundwater is expected to be controlled with conventional sumps and pumps. The temporary sumps should be filtered with stone and fabric and extend at least ≈ 24 inches below construction grade. A ≈ 6 inch lift of $\frac{3}{4}$ -inch minus crushed stone should be placed atop the wet subgrade to protect its competency and facilitate dewatering. Adequate dewatering and storm water management are necessary for maintaining the competency of the site soils. The discharge of the collected water should be reviewed by others.

The subgrade should ultimately be stable, dewatered, compact and protected from frost throughout construction. Bearing subgrades that become weakened or disturbed due to wet conditions will be rendered unsuitable for structural support. The Contractor shall ultimately be responsible for the means and methods of temporary groundwater control, subgrade protection and site stability during construction. An Engineer from KMM should be scheduled to review the foundation subgrade conditions and preparation during construction.

CONSTRUCTION MONITORING

It is recommended that a qualified engineer or representative be retained to review earthwork activities such as the preparation of the foundation bearing subgrade and the placement/compaction of Structural Fill. It is recommended that KMM be retained to provide construction monitoring services. This is to observe compliance with the design concepts presented herein.

We trust the contents of this memorandum report are responsive to your needs at this time. Should you have any questions or require additional assistance, please do not hesitate to contact our office.

LIMITATIONS

Explorations

1. The analyses, recommendations and designs submitted in this report are based in part upon the data obtained from preliminary subsurface explorations. The nature and extent of variations between these explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.
2. The generalized soil profile described in the text is intended to convey trends in subsurface conditions. The boundaries between strata are approximate and idealized and have been developed by interpretation of widely spaced explorations and samples; actual soil transitions are probably more gradual. For specific information, refer to the individual test pit and/or boring logs.
3. Water level readings have been made in the test pits and/or test borings under conditions stated on the logs. These data have been reviewed and interpretations have been made in the text of this report. However, it must be noted that fluctuations in the level of the groundwater may occur due to variations in rainfall, temperature, and other factors differing from the time the measurements were made.

Review

4. It is recommended that this firm be given the opportunity to review final design drawings and specifications to evaluate the appropriate implementation of the recommendations provided herein.
5. In the event that any changes in the nature, design, or location of the proposed areas are planned, the conclusions and recommendations contained in this report shall not be considered valid unless the changes are reviewed and conclusions of the report modified or verified in writing by KMM Geotechnical Consultants, LLC.

Construction

6. It is recommended that this firm be retained to provide geotechnical engineering services during the earthwork phases of the work. This is to observe compliance with the design concepts, specifications, and recommendations and to allow design changes in the event that subsurface conditions differ from those anticipated prior to the start of construction.

Use of Report

7. This report has been prepared for the exclusive use of TAG Central, LLC in accordance with generally accepted soil and foundation engineering practices. No other warranty, expressed or implied, is made.
8. This report has been prepared for this project by KMM Geotechnical Consultants, LLC. This report was completed for preliminary design purposes and may be limited in its scope to complete an accurate bid. Contractors wishing a copy of the report may secure it with the understanding that its scope is limited to preliminary geotechnical design considerations only.

TABLE 1

444 East Central Street
Franklin, Massachusetts

Recommended Soil Gradation & Compaction Specifications

Gravel Base Fill

(Select CRUSHED GRAVEL borrow)

SIEVE SIZE	PERCENT PASSING BY WEIGHT
3 inch	100
3/4 inch	50-90
No. 4	30-70
No. 200	2-8

NOTES: For minimum 6 inch base below Concrete Slab-on-Grade (heated)
For minimum 12-inch base for exterior concrete slabs exposed to frost
Shall have less than 12% fines (No. 200 sieve) based on the Sand fraction

Structural Fill

(Gravelly SAND)

SIEVE SIZE	PERCENT PASSING BY WEIGHT
5 inch	100
3/4 inch	50-100
No. 4	20-80
No. 200	0-10

NOTES: For use below building foundations for structural bearing support
A ¾-inch crushed stone may be used for Structural Fill in wet conditions
Reclaimed Borrow (crushed concrete) shall conform to *MA-DPW Specification*
Reclaimed Borrow shall limit (less than 3%) brick or asphalt component
Shall have less than 20% fines (No. 200 sieve) based on the Sand fraction

Structural Fill placed beneath the foundation should include the *Footing Zone of Influence* which is defined as that area extending laterally one foot from the edge of the footing then outward and downward at a 1H:1V splay. Structural Fill should be placed in loose lifts not exceeding 12 inches for heavy vibratory rollers and 8 inches for vibratory plate compactors. All the fill on the project should be compacted to at least 95 percent of maximum dry density as determined by the Modified Proctor Test (ASTM-D1557). The fill shall be compacted within ± 2 of the optimum moisture content. The adequacy of the compaction efforts should be verified by field density testing which is also a requirement of the *Massachusetts State Building Code*.

TEST BORING LOG



SOIL X Corp.

148 Pioneer Drive
Leominster, MA 01453

**444 East Central Street
Franklin, MA**

B-1

24-10026

Ground Elevation: 272 ft+/-

Date Started: 10/29/24

Date Finished: 10/29/24

Driller: MT

Soil Engineer/Geologist:

GROUNDWATER OBSERVATIONS

DATE	DEPTH	CASING AT	STABILIZATION
10/29/24	6 ft		

Depth Ft.		Sample				Strata Break	Visual Identification of Soil and / or Rock Sample
		No.	Rec	Depth	Blows/6"		
1		1	12"	0'0"-2'0"	4-8-19-20	10'	Brown, fine to coarse Sand & Gravel, trace silt, dry
		2	13"	2'0"-4'0"	11-11-18-14		Brown, mottled, f-m Sand & Gravel, little silt, dry
5		3	9"	5'0"-7'0"	17-29-21-19		Brown, fine to medium Sand & Gravel, little silt, wet
		4	14"	7'0"-9'0"	13-10-11-25		Brown, f-m Sand, some gravel, little silt, wet
10							Refusal @ 10' BGS GWT @ 6' upon completion
15							
20							
25							
30							

Notes: GeoProbe 6712

Cohesionless: 0 - 4 V. Loose, 4 - 10 Loose,
10 - 30 M Dense, 30 - 50 Dense, 50+ V Dense.
Cohesive: 0 - 2 V Soft, 2 - 4 Soft, 4 - 8 M Stiff
8 - 15 Stiff, 15 - 30 V. Stiff, 30 + Hard.

Trace 0 to 10%
Little 10 to 20%
Some 20 to 35%
And 35% to 50%

ID SIZE (IN)
HAMMER WGT (LB)
HAMMER FALL (IN)

CASING

SAMPLE

CORE TYPE

SS
140 lb.
30"

TEST BORING LOG



**444 East Central Street
Franklin, MA**

B-2

24-10026

Ground Elevation: 276 ft+/-

Date Started: 10/29/24

Date Finished: 10/29/24

Driller: MT

Soil Engineer/Geologist:

GROUNDWATER OBSERVATIONS

DATE	DEPTH	CASING AT	STABILIZATION
10/29/24	8 ft		

Depth Ft.		Sample				Strata Break	Visual Identification of Soil and / or Rock Sample
		No.	Rec	Depth	Blows/6"		
1		1	14"	0'0"-2'0"	1-4-5-5	2'	Rust, Fine Sand, some silt, trace gravel, roots (SUBSOIL)
		2	16"	2'0"-4'0"	5-7-9-11		Tan, fine to medium Sand, little gravel, trace silt, dry
5		3	12"	5'0"-7'0"	10-9-8-7		Tan, fine to medium Sand, dry (SAND)
		4	12"	7'0"-9'0"	5-4-4-5		Brown, Fine Sand, wet
10		5	14"	10'0"-12'0"	1-1-1-2		Tan, Fine Sand, wet
							(FLUVIAL)
15		6	18"	15'0"-17'0"	3-3-2-2		Tan, Fine Sand, wet
20		7	16"	20'0"-22'0"	4-4-3-7		Tan, Fine Sand, little silt, wet
25							EOB @ 22' BGS GWT @ 8'5" upon completion
30							

Notes: GeoProbe 6712

Cohesionless: 0 - 4 V. Loose, 4 - 10 Loose, 10 -30 M Dense, 30 -50 Dense, 50+ V Dense. Cohesive: 0 -2 V Soft, 2 -4 Soft, 4 -8 M Stiff 8 -15 Stiff, 15 -30 V. Stiff, 30 + Hard.	Trace 0 to 10% Little 10 to 20% Some 20 to 35% And 35% to 50%	ID SIZE (IN) HAMMER WGT (LB) HAMMER FALL (IN)	CASING SAMPLE CORE TYPE	SS 140 lb. 30"
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TEST BORING LOG



**444 East Central Street
Franklin, MA**

B-3

24-10026

Ground Elevation: 273 ft+/-
Date Started: 10/29/24
Date Finished: 10/29/24
Driller: MT

GROUNDWATER OBSERVATIONS

DATE	DEPTH	CASING AT	STABILIZATION
10/29/24	5 ft		

Soil Engineer/Geologist:

Depth Ft.		Sample				Strata Break	Visual Identification of Soil and / or Rock Sample
		No.	Rec	Depth	Blows/6"		
1		1	15"	0'0"-2'0"	1-1-2-3	2'	Dark Brown, Organic Silt (TOPSOIL)
		2	15"	2'0"-4'0"	4-6-9-6		Tan-Rust, Fine Sand, trace silt, dry
5		3	16"	5'0"-7'0"	8-7-6-7		Grey, Fine Sand, little silt, wet
		4	17"	7'0"-9'0"	8-8-8-8		Grey, Fine Sand, wet
10		5	19"	10'0"-12'0"	4-5-3-4		Grey, Fine Sand, little/trace silt
							(FLUVIAL)
15		6	23"	15'0"-17'0"	8-9-7-8		Fine Sand
20		7	15"	20'0"-22'0"	6-5-8-6		Fine Sand, some silt
25							EOB @ 22' BGS GWT @ 5' upon completion
30							

Notes: GeoProbe 6712

Cohesionless: 0 - 4 V. Loose, 4 - 10 Loose, 10 -30 M Dense, 30 -50 Dense, 50+ V Dense. Cohesive: 0 -2 V Soft, 2 -4 Soft, 4 -8 M Stiff 8 -15 Stiff, 15 -30 V. Stiff, 30 + Hard.	Trace 0 to 10% Little 10 to 20% Some 20 to 35% And 35% to 50%	ID SIZE (IN) HAMMER WGT (LB) HAMMER FALL (IN)	CASING SAMPLE CORE TYPE	SS 140 lb. 30"
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TEST BORING LOG



**444 East Central Street
Franklin, MA**

B-4

24-10026

Ground Elevation: 272 ft+/-
Date Started: 10/29/24
Date Finished: 10/29/24
Driller: MT

Soil Engineer/Geologist:

GROUNDWATER OBSERVATIONS

DATE	DEPTH	CASING AT	STABILIZATION
10/29/24	5 ft		

Depth Ft.		Sample				Strata Break	Visual Identification of Soil and / or Rock Sample
		No.	Rec	Depth	Blows/6"		
1		1	18"	0'0"-2'0"	2-4-3-5	2'	Topsoil/ Subsoil (ORGANIC)
		2	15"	2'0"-4'0"	8-10-12-9		Orange-Tan, f-m Sand, trace gravel, trace silt, dry
5		3	19"	5'0"-7'0"	9-6-8-9		Grey, Fine Sand, little silt, wet (FLUVIAL)
		4	16"	7'0"-9'0"	8-7-9-10		Grey, Fine Sand, little silt, wet
10		5	18"	10'0"-12'0"	22-22-21-19	10'	Brown, fine to coarse Sand & Gravel, trace silt, wet
15		6	13"	15'0"-17'0"	27-27-26-21		Brown, fine to coarse Sand & Gravel, trace silt, wet
							(GLACIAL)
20		7	5"	20'0"-22'0"	34-50/2"		Same
25							EOB @ 21 BGS GWT @ 5 ft upon completion
30							

Notes: GeoProbe 6712

Cohesionless: 0 - 4 V. Loose, 4 - 10 Loose,
10 -30 M Dense, 30 -50 Dense, 50+ V Dense.
Cohesive: 0 -2 V Soft, 2 -4 Soft, 4 -8 M Stiff
8 -15 Stiff, 15 -30 V. Stiff, 30 + Hard.

Trace 0 to 10%
Little 10 to 20%
Some 20 to 35%
And 35% to 50%

ID SIZE (IN)
HAMMER WGT (LB)
HAMMER FALL (IN)

CASING

SAMPLE

CORE TYPE

SS
140 lb.
30"

TEST BORING LOG



**444 East Central Street
Franklin, MA**

B-5

24-10026

Ground Elevation: 272 ft+/-
Date Started: 10/29/24
Date Finished: 10/29/24
Driller: MT

Soil Engineer/Geologist:

GROUNDWATER OBSERVATIONS

DATE	DEPTH	CASING AT	STABILIZATION
10/29/24	5 ft		

Depth Ft.		Sample				Strata Break	Visual Identification of Soil and / or Rock Sample
		No.	Rec	Depth	Blows/6"		
1		1	10"	0'0"-2'0"	2-2-2-4	3'	Dark Brown, loamy, silty Sand, trace gravel, dry (ORGANIC)
		2	18"	2'0"-4'0"	3-3-3-6		Brown, Fine Sand, little silt
5		3	16"	5'0"-7'0"	9-10-9-8		Tan, Fine Sand, trace silt, wet
		4	20"	7'0"-9'0"	7-7-7-7	14'	Fine Sand, little silt, wet (FLUVIAL)
10		5	21"	10'0"-12'0"	3-4-3-3		Tan, Fine Sand, some silt, wet
15		6	10"	15'0"-17'0"	15-16-33-29		Brown, fine to coarse Sand & Gravel, trace silt, wet
							(GLACIAL)
20		7	9"	20'0"-22'0"	5-4-7-32		Brown, fine to coarse Sand & Gravel, wet
25							EOB @ 22 BGS GW @ 5' BGS upon completion.
30							

Notes: GeoProbe 6712

Cohesionless: 0 - 4 V. Loose, 4 - 10 Loose, 10 - 30 M Dense, 30 - 50 Dense, 50+ V Dense.	Trace 0 to 10% Little 10 to 20% Some 20 to 35% And 35% to 50%	ID SIZE (IN) HAMMER WGT (LB) HAMMER FALL (IN)	CASING	SAMPLE SS 140 lb. 30"	CORE TYPE
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TEST BORING LOG



SOIL X, Corp.

148 Pioneer Drive
Leominster, MA 01453

**444 East Central Street
Franklin, MA**

B-6

24-10026

Ground Elevation: 272 ft+/-

Date Started: 10/29/24

Date Finished: 10/29/24

Driller: MT

Soil Engineer/Geologist:

GROUNDWATER OBSERVATIONS

DATE	DEPTH	CASING AT	STABILIZATION TIME
10/29/24	5 ft		

Depth Ft.		Sample				Strata Break	Visual Identification of Soil and / or Rock Sample
		No.	Rec	Depth	Blows/6"		
1		1	13"	0'0"-2'0"	7-21-30-24		Brown-Grey, fine to coarse Sand & Gravel, trace silt, dry
		2	18"	2'0"-4'0"	12-10-12-9	4'	Grey, f-m Sand, some silt, trace gravel (SUSPECT FILL)
5		3	15"	5'0"-7'0"	11-9-9-10	7'	Tan, Silt & Fine Sand, wet (FLUVIAL)
		4	18"	7'0"-9'0"	14-14-18-16		Brown, fine to coarse Sand & Gravel, wet
10		5	13"	10'0"-12'0"	23-18-23-22		Brown, fine to coarse Sand & Gravel, trace silt, wet
							(GLACIAL)
15		6	15"	15'0"-17'0"	65-43-50/5"		Same, cobbles
						18'5"	
20							Refusal @ 18'5" BGS
							GWT @ 5' upon completion
25							
30							

Notes: GeoProbe 6712

Cohesionless: 0 - 4 V. Loose, 4 - 10 Loose, 10 - 30 M Dense, 30 - 50 Dense, 50+ V Dense.	Trace 0 to 10% Little 10 to 20% And 35% to 50%	CASING ID SIZE (IN) HAMMER FALL (IN)	SAMPLE SS 30"	CORE TYPE
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TEST BORING LOG



**444 East Central Street
Franklin, MA**

B-7

24-10026

Ground Elevation: 272 ft+/-
Date Started: 10/29/24
Date Finished: 10/29/24
Driller: MT

Soil Engineer/Geologist:

GROUNDWATER OBSERVATIONS

DATE	DEPTH	CASING AT	STABILIZATION
10/29/24	5 ft		

Depth Ft.		Sample				Strata Break	Visual Identification of Soil and / or Rock Sample
		No.	Rec	Depth	Blows/6"		
1		1	9"	0'0"-2'0"	4-5-11-7	4'	Dark Brown, loamy, silty Sand, trace gravel, organic (ORGANIC)
		2	12"	2'0"-4'0"	6-6-6-5		Dark Brown, loamy, silty Sand, little gravel, trace organic, dry
5		3	18"	5'0"-7'0"	10-12-12-11		Brown, f-c Sand, little gravel, little silt, wet
		4	19"	7'0"-9'0"	10-9-9-8		Grey, Fine Sand, some silt, wet (FLUVIAL)
10		5	22"	10'0"-12'0"	2-4-4-8	13'	Brown, Silt, wet
15		6	2"	15'0"-17'0"	50/3"	15'	fine to coarse Sand & Gravel, wet (GLACIAL)
20							Refusal @ 15' BGS GWT @ 5 ft upon completion
25							
30							

Notes: GeoProbe 6712

Cohesionless: 0 - 4 V. Loose, 4 - 10 Loose, 10 -30 M Dense, 30 -50 Dense, 50+ V Dense.	Trace 0 to 10%		CASING	SAMPLE	CORE TYPE
Cohesive: 0 -2 V Soft, 2 -4 Soft, 4 -8 M Stiff 8 -15 Stiff, 15 -30 V. Stiff, 30 + Hard.	Little 10 to 20%	ID SIZE (IN)		SS	
	Some 20 to 35%	HAMMER WGT (LB)		140 lb.	
	And 35% to 50%	HAMMER FALL (IN)		30"	

TEST BORING LOG



**444 East Central Street
Franklin, MA**

B-8

24-10026

Ground Elevation: 274 ft+/-
Date Started: 10/29/24
Date Finished: 10/29/24
Driller: MT

Soil Engineer/Geologist:

GROUNDWATER OBSERVATIONS

DATE	DEPTH	CASING AT	STABILIZATION
10/29/24	7 ft		

Depth Ft.	Sample					Strata Break	Visual Identification of Soil and / or Rock Sample
	No.	Rec	Depth	Blows/6"			
1	1	22"	0'0"-2'0"	4-3-5-3		6"	TOPSOIL
	2	16"	2'0"-4'0"	3-4-6-7		2'	Tan, Silt & Fine Sand (SUBSOIL)
5	3	17"	5'0"-7'0"	12-9-8-8			Tan, fine to medium Sand, trace silt, dry
	4	20"	7'0"-9'0"	11-13-46-31			Tan, Silt w/ Fine Sand, dry (FLUVIAL)
10						9'	Tan, Fine Sand, little silt, wet
	5	15"	10'0"-12'0"	26-24-44-38		12'	fine to coarse Sand & Gravel, trace silt, wet (GLACIAL)
15							Refusal @ 12' BGS
20							GWT @ 7' upon completion
25							
30							

Notes: GeoProbe 6712

Cohesionless: 0 - 4 V. Loose, 4 - 10 Loose, 10 -30 M Dense, 30 -50 Dense, 50+ V Dense.	Trace 0 to 10%		CASING	SAMPLE	CORE TYPE
Cohesive: 0 -2 V Soft, 2 -4 Soft, 4 -8 M Stiff	Little 10 to 20%	ID SIZE (IN)		SS	
8 -15 Stiff, 15 -30 V. Stiff, 30 + Hard.	Some 20 to 35%	HAMMER WGT (LB)		140 lb.	
	And 35% to 50%	HAMMER FALL (IN)		30"	

TEST BORING LOG



SOIL X Corp.

148 Pioneer Drive
Leominster, MA 01453

**444 East Central Street
Franklin, MA**

B-9 (OW)

24-10026

Ground Elevation: 274 ft+/-

Date Started: 10/29/24

Date Finished: 10/29/24

Driller: MT

Soil Engineer/Geologist:

GROUNDWATER OBSERVATIONS

DATE	DEPTH	CASING AT	STABILIZATION
10/29/24	7 ft		

Depth Ft.		Sample				Strata Break	Visual Identification of Soil and / or Rock Sample
		No.	Rec	Depth	Blows/6"		
1		1	9"	0'0"-2'0"	1-3-3-4	4"	TOPSOIL
		2	8"	2'0"-4'0"	7-50/5"	2'	Brown, Fine Sand, some silt, trace gravel, dry (SUBSOIL)
							Tan, fine to medium Sand & Gravel, trace silt, cobbles, dry
5		3	23"	5'0"-7'0"	43-37-21-19		Brown-Grey, fine to coarse Sand & Gravel, little silt, dry
		4	23"	7'0"-9'0"	17-21-21-21		Brown, fine to coarse Sand & Gravel, trace silt, wet
10		5	13"	10'0"-12'0"	16-24-16-12		(GLACIAL) Grey, fine to coarse Sand & Gravel, Fractured Rock, wet
15						13'	Refusal @ 13' BGS GWT @ 7' upon completion
20							Observation Well installed to 12 ft
25							
30							

Notes: GeoProbe 6712

Cohesionless: 0 - 4 V. Loose, 4 - 10 Loose,
10 -30 M Dense, 30 -50 Dense, 50+ V Dense.
Cohesive: 0 -2 V Soft, 2 -4 Soft, 4 -8 M Stiff
8 -15 Stiff, 15 -30 V. Stiff, 30 + Hard.

Trace 0 to 10%
Little 10 to 20%
Some 20 to 35%
And 35% to 50%

ID SIZE (IN)
HAMMER WGT (LB)
HAMMER FALL (IN)

CASING

SAMPLE

CORE TYPE

SS
140 lb.
30"

TEST BORING LOG



**444 East Central Street
Franklin, MA**

B-10

24-10026

Ground Elevation: 274 ft+/-

Date Started: 10/29/24

Date Finished: 10/29/24

Driller: MT

Soil Engineer/Geologist:

GROUNDWATER OBSERVATIONS

DATE	DEPTH	CASING AT	STABILIZATION
10/29/24	n/a		

Depth Ft.	Sample				Strata Break	Visual Identification of Soil and / or Rock Sample
	No.	Rec	Depth	Blows/6"		
1	1	16"	0'0"-2'0"	2-6-9-9	2'5"	Tan, f-m Sand, some silt, trace gravel, dry (SUBSOIL)
	2	9"	2'0"-4'0"	25-50/5"		Tan, f-m Sand & Gravel, little silt, cobbles, dry
5	3	18"	5'0"-7'0"	38-41-38-43	8'	(GLACIAL) Brown, fine to coarse Sand & Gravel, trace silt, dry
	4	5"	7'0"-9'0"	50/5"		Tan, fine to coarse Sand, some gravel, trace silt, dry
10						Refusal @ 8' BGS Offset Refusals @ 7' & 7½'
15						No GWT upon completion
20						
25						
30						

Notes: GeoProbe 6712

Cohesionless: 0 - 4 V. Loose, 4 - 10 Loose,
10 -30 M Dense, 30 -50 Dense, 50+ V Dense.
Cohesive: 0 -2 V Soft, 2 -4 Soft, 4 -8 M Stiff
8 -15 Stiff, 15 -30 V. Stiff, 30 + Hard.

Trace 0 to 10%
Little 10 to 20%
Some 20 to 35%
And 35% to 50%

ID SIZE (IN)
HAMMER WGT (LB)
HAMMER FALL (IN)

CASING

SAMPLE

CORE TYPE

SS

140 lb.

30"



Title	Pipe Sizing Table
Project	40B Multifamily
Location	444 East Central Street, Franklin MA
Date	6/12/2025
Revised	7/21/2025

Minimum Slope:	0.005
Minimum Pipe Size:	12"
Rainfall Intensity (in/hr):	6.60 (25 year storm)
Manning's n:	0.013 HDPE (SMOOTH BORE)
Minimum Pipe Cover:	1.3'

Line						Req'd. Capac.	Pipe Size	Slope	Flow at Inv. Slope		Drop	Invert Elevation		Rim Elev.		Pipe
From Upper	To Lower	Length (feet)	Area (acres)	wgt. C	CA	Q _d (cfs)	D (in)	s (%)	Q _{full} (cfs)	V _{full} (fps)	(feet)	Upper (ft)	Lower (ft)	Upper (ft)	Cover (ft)	Material
CB1WQU	DMH1	84	0.246	0.83	0.204	1.35	12	0.60%	2.75	3.50	0.50	272.20	271.70	274.64	1.32	HDPE
CB2	DMH27WQU	48	0.298	0.39	0.118	0.78	12	0.56%	2.67	3.39	0.27	271.61	271.34	274.10	1.37	HDPE
CB3	DMH27WQU	77	0.212	0.65	0.138	0.91	12	0.51%	2.56	3.25	0.40	271.74	271.34	274.30	1.44	HDPE
DMH27WQU	DMH2A	9				1.69	15	0.50%	4.59	3.73	0.04	272.34	272.30	275.00	1.28	HDPE
CB4C	DMH30	61	0.246	0.82	0.201	1.33	12	0.78%	3.15	4.00	0.47	273.25	272.78	275.70	1.32	HDPE
CB4D	DMH30	38	0.234	0.79	0.185	1.22	12	0.99%	3.56	4.52	0.38	273.16	272.78	275.70	1.41	HDPE
DMH30	DMH29	34				2.55	15	1.00%	6.48	5.27	0.34	272.68	272.34	275.35	1.30	HDPE
DMH29	DMH26WQU	11				2.55	15	1.00%	6.48	5.27	0.12	272.24	272.13	275.30	1.69	HDPE
CB4A	DMH26WQU	7	0.091	0.90	0.082	0.54	12	1.01%	3.58	4.55	0.07	272.19	272.13	275.00	1.68	HDPE
CB4B	DMH26WQU	27	0.235	0.75	0.177	1.17	12	1.00%	3.57	4.54	0.27	272.40	272.13	274.95	1.43	HDPE
DMH26WQU	DMH4	43				4.26	18	1.00%	10.53	5.94	0.43	272.13	271.70	275.10	1.35	HDPE
CB5C	DMH31	41	0.060	0.85	0.051	0.34	12	1.00%	3.56	4.53	0.41	273.90	273.49	276.65	1.63	HDPE
DMH31	DMH28WQU	20				0.34	12	1.00%	3.57	4.53	0.20	273.39	273.19	276.80	2.29	HDPE
CB5A	DMH28WQU	29	0.240	0.84	0.202	1.34	12	1.00%	3.57	4.53	0.29	273.47	273.19	276.40	1.80	HDPE
CB5B	DMH28WQU	45	0.094	0.87	0.082	0.54	12	1.00%	3.57	4.54	0.45	273.64	273.19	276.60	1.84	HDPE
DMH28WQU	DMH6	58				2.22	15	1.00%	6.47	5.26	0.58	273.09	272.50	276.75	2.29	HDPE
CB6WQU	DMH7	26	0.290	0.78	0.225	1.48	12	0.99%	3.56	4.52	0.26	272.76	272.50	276.00	2.12	HDPE
CB7A	DMH8WQU	10	0.163	0.79	0.129	0.85	12	1.00%	3.57	4.54	0.10	272.92	272.82	276.50	2.45	HDPE
CB7B	DMH8WQU	40	0.260	0.76	0.199	1.31	12	1.01%	3.58	4.55	0.40	273.22	272.82	276.00	1.65	HDPE
DMH8WQU	DMH9	22				2.16	15	0.99%	6.43	5.23	0.22	272.72	272.50	276.65	2.55	HDPE
CB8	DMH33WQU	27	0.252	0.80	0.201	1.33	12	1.00%	3.57	4.54	0.27	273.47	273.20	276.00	1.41	HDPE
CB9B	DMH33WQU	64	0.338	0.72	0.244	1.61	12	0.60%	2.77	3.52	0.38	273.58	273.20	276.00	1.30	HDPE
DMH33WQU	DMH11	20				2.94	15	1.00%	6.48	5.27	0.20	273.20	273.00	276.30	1.73	HDPE
CB9D	DMH32	49	0.035	0.67	0.024	0.16	12	0.89%	3.37	4.29	0.44	273.95	273.51	276.40	1.32	HDPE
CB9A	DMH32	14	0.270	0.80	0.215	1.42	12	0.50%	2.52	3.21	0.07	273.58	273.51	276.00	1.30	HDPE
DMH32	DMH12WQU	48				1.57	12	0.53%	2.59	3.29	0.25	273.41	273.16	276.30	1.76	HDPE
CB9C	DMH12WQU	7	0.085	0.90	0.077	0.51	12	1.13%	3.79	4.82	0.08	273.24	273.16	276.80	2.44	HDPE
DMH12WQU	DMH13	28				2.08	15	0.58%	4.92	4.00	0.16	273.16	273.00	276.85	2.32	HDPE
CB10A	DMH15WQU	87	0.066	0.80	0.053	0.35	12	1.00%	3.58	4.54	0.87	271.41	270.54	276.00	3.46	HDPE
CB10B	DMH15WQU	15	0.239	0.83	0.198	1.30	12	1.01%	3.59	4.57	0.16	270.70	270.54	276.50	4.68	HDPE
DMH15WQU	DMH16	14				1.65	12	0.99%	3.55	4.51	0.14	270.44	270.30	276.75	5.19	HDPE
CB11WQU	DMH17	15	0.264	0.82	0.216	1.43	12	1.01%	3.59	4.56	0.15	270.55	270.40	277.00	5.32	HDPE
CB17	DMH24	143	0.827	0.50	0.416	2.74	15	0.61%	5.06	4.11	0.88	274.97	274.10	277.65	1.30	HDPE
CB16	DMH24	19	0.473	0.55	0.258	1.70	12	1.00%	3.57	4.53	0.19	274.29	274.10	277.65	2.24	HDPE
DMH24	DMH23	176				4.45	18	1.00%	10.53	5.95	1.76	274.00	272.23	278.00	2.38	HDPE
CB15	DMH23	19	0.463	0.60	0.276	1.82	12	0.99%	3.55	4.51	0.19	272.42	272.23	277.00	3.45	HDPE
DMH23	DMH22	121				6.26	18	1.00%	10.52	5.94	1.21	272.13	270.92	277.50	3.75	HDPE
CB13	DMH22	87	0.095	0.84	0.199	1.31	12	1.01%	3.59	4.56	0.31	271.79	270.92	275.90	1.63	HDPE
CB14	DMH22	33	0.142	0.83	0.199	1.31	12	1.00%	3.57	4.54	1.12	271.25	270.92	276.75	1.33	HDPE
DMH22	DMH21WQU	22				8.89	24	1.00%	22.72	7.22	0.22	270.82	270.60	277.35	4.41	HDPE
CB12	DMH21WQU	14	0.164	0.74	0.199	1.31	12	1.00%	3.57	4.54	1.12	270.74	270.60	277.00	1.33	HDPE
DMH21WQU	DMH20	20				10.20	24	1.00%	22.70	7.21	0.20	270.50	270.30	277.30	4.68	HDPE

			Computation Sheet	
Title	MA DEP Standard Calculations		By	MTB
Project	40B Multifamily		Chk'd	CMQ
Location	444 East Central Street, Franklin MA		Apprv'd	CMQ
Date	02-11-2025			
Revised	7-21-2025			

Stormwater Recharge/Water Quality Volume Table

$R_v = F * \text{Impervious Area}$

R_v = Required Recharge Volume, expressed in ft^3 , cubic yards or acre-feet

F = Target Depth Factor associated with each Hydraulic Soil Group

Impervious Area = pavement & rooftop area on site

A_{wq} = Required Water Quality Treatment Volume, expressed in ft^3

D_{wq} = Water Quality Depth

A_{IMP} = Impervious Area (excluding non-metal roofs)

Watershed	Area (Sq. Ft.)	Landscaped	Impervious Area (Square Feet)	Recharge Required			Water Quality Volume Required	
			HSG A (F=.6)	F Avg. (Inches)	Impervious Area (Feet)	R_v (ft^3)	D_{wq} (Inch)	A_{wq}
CB1	10,725	1,397	9,328	0.6	9,328	466	1.0	777
CB2	12,976	11,923	1,053	0.6	1,053	53	1.0	88
CB3	9,239	4,963	4,276	0.6	4,276	214	1.0	356
CB4A	3,976	321	3,655	0.6	3,655	183	1.0	305
CB4B	10,218	2,717	7,501	0.6	7,501	375	1.0	625
CB4C	10,741	1,624	9,117	0.6	9,117	456	1.0	760
CB4D	10,203	2,043	8,160	0.6	8,160	408	1.0	680
CB5A	10,459	721	9,738	0.6	9,738	487	1.0	812
CB5B	3,865	192	3,673	0.6	3,673	184	1.0	306
CB5C	2,619	225	2,394	0.6	2,394	120	1.0	200
CB6	12,631	2,854	9,777	0.6	9,777	489	1.0	815
CB7A	7,101	1,427	5,674	0.6	5,674	284	1.0	473
CB7B	11,340	2,817	8,523	0.6	8,523	426	1.0	710
CB8	10,991	2,060	8,931	0.6	8,931	447	1.0	744
CB9A	11,743	2,202	9,541	0.6	9,541	477	1.0	795
CB9B	14,736	4,805	9,931	0.6	9,931	497	1.0	828
CB9C	3,708	0	3,708	0.6	3,708	185	1.0	309
CB9D	1,543	642	901	0.6	901	45	1.0	75
CB10A	2,889	539	2,350	0.6	2,350	118	1.0	196
CB10B	10,414	1,387	9,027	0.6	9,027	451	1.0	752
CB11	11,505	1,699	9,806	0.6	9,806	490	1.0	817
CB12	7,149	2,035	5,114	0.6	5,114	256	1.0	426
CB13	4,141	475	3,666	0.6	3,666	183	1.0	306
CB14	6,167	802	5,365	0.6	5,365	268	1.0	447
CB15	20,154	11,154	9,000	0.6	9,000	450	1.0	750
CB16	20,584	13,260	7,324	0.6	7,324	366	1.0	610
CB17	36,035	25,990	10,045	0.6	10,045	502	1.0	837
R1	5,046	0	5,046	0.6	5,046	252	1.0	421
R2	18,290	0	18,290	0.6	18,290	915	1.0	1,524
R3	18,290	0	18,290	0.6	18,290	915	1.0	1,524
R4	18,290	0	18,290	0.6	18,290	915	1.0	1,524
R5	20,565	0	20,565	0.6	20,565	1,028	1.0	1,714
Patio	2,833	0	2,833	0.6	2,833	142	1.0	236
Total	361,166	100,274	260,892	0.6	260,892	13,045	1.0	21,741

			Computation Sheet	
Title	MA DEP Standard Calculations		By	MTB
Project	40B Multifamily		Chk'd	CMQ
Location	444 East Central Street, Franklin MA		Apprv'd	CMQ
Date	02-11-2025			

Stormwater Recharge Summary

$R_v = F * \text{Impervious Area}$

R_v = Required Recharge Volume, expressed in ft^3 , cubic yards or acre-feet

F = Target Depth Factor associated with each Hydraulic Soil Group

Impervious Area = pavement & rooftop area on site

	Required (cf)	Provided (cf)	
AR _v =	2,282	9,171	Underground Infiltration System #1 (CB1,CB4A,CB4B,CB4C,CB4D,R-1,Patio)
AR _v =	2,282	9,171	Total

	Required (cf)	Provided (cf)	
AR _v =	266	508	Underground Infiltration System #2 (CB2,CB3)
AR _v =	266	508	Total

	Required (cf)	Provided (cf)	
AR _v =	3,818	7,065	Underground Infiltration System #3 (CB5A,CB5B,CB5C,CB6,CB7B,CB7A,R2,R3)
AR _v =	3,818	7,065	Total

	Required (cf)	Provided (cf)	
AR _v =	1,651	2,873	Underground Infiltration System #4 (CB8,CB9A,CB9B,CB9C,DB9D)
AR _v =	1,651	2,873	Total

	Required (cf)	Provided (cf)	
AR _v =	5,028	21,847	Underground Infiltration System #5 (CB10A,CB10B,CB11,CB12,CB13,CB14,CB15,CB16,CB17,R4,R5)
AR _v =	5,028	21,847	Total

Water Quality Volume

A_{wq} = Required Water Quality Treatment Volume, expressed in ft^3

D_{wq} = Water Quality Depth

A_{imp} = Impervious Area (excluding non-metal roofs)

	Required (cf)	Provided (cf)	
A_{wq} =	3,803	9,171	Underground Infiltration System #1 (CB1,CB4A,CB4B,CB4C,CB4D,R-1,Patio)
A_{wq} =	3,803	9,171	Total

	Required (cf)	Provided (cf)	
A_{wq} =	444	508	Underground Infiltration System #2 (CB2,CB3)
A_{wq} =	444	508	Total

	Required (cf)	Provided (cf)	
A_{wq} =	6,363	7,065	Underground Infiltration System #3 (CB5A,CB5B,CB5C,CB6,CB7B,CB7A,R2,R3)
A_{wq} =	6,363	7,065	Total

	Required (cf)	Provided (cf)	
A_{wq} =	2,751	2,873	Underground Infiltration System #4 (CB8,CB9A,CB9B,CB9C,DB9D)
A_{wq} =	2,751	2,873	Total

	Required (cf)	Provided (cf)	
A_{wq} =	8,379	21,847	Underground Infiltration System #5 (CB10A,CB10B,CB11,CB12,CB13,CB14,CB15,CB16,CB17,R4,R5)
A_{wq} =	8,379	21,847	Total

			Computation Sheet	
Title	MA DEP Standard Calculations		By	MTB
Project	40B Multifamily		Chk'd	CMQ
Location	444 East Central Street, Franklin MA		Apprv'd	CMQ
Date	02-11-2025			

Draindown Within 72 Hours

$\text{Time}_{\text{drawdown}} = (Rv) (1/\text{Design Infiltration Rate in inches per hour}) (\text{Conversion for inches to feet}) (1/\text{bottom area in feet})$

UIS1	
Infiltration Rate (in/Hr)=	8.27
Bottom Area (ft ²)=	4,465
Infiltration Volume (ft ³)=	9,171
Time_{drawdown} (Hours)=	2.98

$\text{Time}_{\text{drawdown}} = (Rv) (1/\text{Design Infiltration Rate in inches per hour}) (\text{Conversion for inches to feet}) (1/\text{bottom area in feet})$

UIS2	
Infiltration Rate (in/Hr)=	8.27
Bottom Area (ft ²)=	452
Infiltration Volume (ft ³)=	508
Time_{drawdown} (Hours)=	1.63

$\text{Time}_{\text{drawdown}} = (Rv) (1/\text{Design Infiltration Rate in inches per hour}) (\text{Conversion for inches to feet}) (1/\text{bottom area in feet})$

UIS3	
Infiltration Rate (in/Hr)=	8.27
Bottom Area (ft ²)=	7,031
Infiltration Volume (ft ³)=	7,065
Time_{drawdown} (Hours)=	1.46

$\text{Time}_{\text{drawdown}} = (Rv) (1/\text{Design Infiltration Rate in inches per hour}) (\text{Conversion for inches to feet}) (1/\text{bottom area in feet})$

UIS4	
Infiltration Rate (in/Hr)=	8.27
Bottom Area (ft ²)=	2,495
Infiltration Volume (ft ³)=	2,873
Time_{drawdown} (Hours)=	1.67

$\text{Time}_{\text{drawdown}} = (Rv) (1/\text{Design Infiltration Rate in inches per hour}) (\text{Conversion for inches to feet}) (1/\text{bottom area in feet})$

UIS5	
Infiltration Rate (in/Hr)=	8.27
Bottom Area (ft ²)=	8,721
Infiltration Volume (ft ³)=	21,847
Time_{drawdown} (Hours)=	3.63

TSS Removal Worksheet - FES1

BMP*	TSS Removal Rate*	Starting TSS Load**	Amount Removed (C*D)	Remaining Load (D-E)
Infiltration Trench	80%	100%	80%	20%
Deep Sump and Hooded Catch Basin	25%	20%	5%	15%

* BMP and TSS Removal Rate Values from the MassDEP Stormwater Handbook Vol. 1. Removal rates for proprietary devices are from approved studies and/or manufacturer data

** Equals remaining load from previous BMP (E)

**Treatment Train
TSS Removal =**

85%

TSS Removal Worksheet - FES2,FES5,FES6,FES7,FES8

BMP*	TSS Removal Rate*	Starting TSS Load**	Amount Removed (C*D)	Remaining Load (D-E)
Subsurface Infiltration Structure	80%	100%	80%	20%

* BMP and TSS Removal Rate Values from the MassDEP Stormwater Handbook Vol. 1. Removal rates for proprietary devices are from approved studies and/or manufacturer data

** Equals remaining load from previous BMP (E)

**Treatment Train
TSS Removal =**

80%

TSS Removal Worksheet - FES9

BMP*	TSS Removal Rate*	Starting TSS Load**	Amount Removed (C*D)	Remaining Load (D-E)
Infiltration Trench	80%	100%	80%	20%
Deep Sump and Hooded Catch Basin	25%	20%	5%	15%

* BMP and TSS Removal Rate Values from the MassDEP Stormwater Handbook Vol. 1. Removal rates for proprietary devices are from approved studies and/or manufacturer data

** Equals remaining load from previous BMP (E)

**Treatment Train
TSS Removal =**

85%

Franklin 40B Project

Computation Sheet

Title **Inlet Grate Capacity**
 Project Franklin 40B Project
 Date July 21, 2925
 A&M Project Number: 3317-01

By CMQ
 Chk'd CMQ
 Apprv'd CMQ

SUMP CATCH BASINS (at a low point, not gutter flow)

Rainfall Intensity (in/hr): 6.60 (25 year storm)
 Single Grate Open Area (s.f.): 1.1 (Neenah R-2501)
 Double Grate Open Area (s.f.): 2.2
 2-Double Grate Open Area (s.f.): 4.4
 Orifice Coefficient: 0.6
 gravitational constant (fps²): 32.2 (unless along a curb)
 Perimeter of single CB (ft): 8 (unless along a curb)
 Perimeter of double CB (ft): 12

Structure I.D.	Contributing Area (acres)	Weighted CN value	C*A	Q _{25 Year Storm}	Orifice Flow Ponding depth (ft)	Perimeter of grate for weir flow	Weir Flow Ponding Depth	Total depth of ponding (ft)	CB Rim Elevation	Peak Elevation
CB1WQU	0.246	0.83	0.204	1.3	0.06	6.0	0.17	0.17	274.64	274.81
CB2	0.298	0.39	0.118	0.8	0.02	8.0	0.10	0.10	274.10	274.20
CB3	0.212	0.65	0.138	0.9	0.03	6.0	0.13	0.13	274.30	274.43
CB4C	0.246	0.82	0.201	1.3	0.06	4.0	0.22	0.22	275.70	275.92
CB4D	0.234	0.79	0.185	1.2	0.05	6.0	0.16	0.16	275.70	275.86
CB4A	0.091	0.90	0.082	0.5	0.01	4.0	0.12	0.12	275.00	275.12
CB4B	0.235	0.75	0.177	1.2	0.00	6.0	0.15	0.15	274.95	275.10
CB5C	0.060	0.85	0.051	0.3	0.00	4.0	0.09	0.09	276.65	276.74
CB5A	0.240	0.84	0.202	1.3	0.02	8.0	0.14	0.14	276.40	276.54
CB5B	0.094	0.87	0.082	0.5	0.00	6.0	0.09	0.09	176.60	176.69
CB7A	0.163	0.79	0.129	0.8	0.01	4.0	0.16	0.16	276.50	276.66
CB7B	0.260	0.76	0.199	1.3	0.02	6.0	0.16	0.16	276.00	276.16
CB8	0.252	0.80	0.201	1.3	0.02	6.0	0.16	0.16	276.00	276.16
CB9B	0.338	0.72	0.244	1.6	0.02	6.0	0.19	0.19	276.00	276.19
CB9D	0.035	0.67	0.024	0.2	0.00	6.0	0.04	0.04	276.40	276.44
CB9A	0.270	0.80	0.215	1.4	0.02	8.0	0.14	0.14	276.00	276.14
CB9C	0.085	0.90	0.077	0.5	0.00	6.0	0.09	0.09	276.80	276.89
CB10A	0.066	0.80	0.053	0.3	0.00	4.0	0.09	0.09	276.00	276.09
CB10B	0.239	0.83	0.198	1.3	0.02	6.0	0.16	0.16	276.50	276.66
CB11WQU	0.264	0.82	0.216	1.4	0.02	8.0	0.14	0.14	277.00	277.14
CB17	0.827	0.50	0.416	2.7	0.07	6.0	0.27	0.27	277.65	277.92
CB16	0.473	0.55	0.258	1.7	0.03	6.0	0.19	0.19	277.65	277.84
CB15	0.463	0.60	0.276	1.8	0.03	8.0	0.17	0.17	277.00	277.17
CB13	0.095	0.84	0.080	0.5	0.00	4.0	0.12	0.12	275.90	276.02
CB14	0.142	0.83	0.117	0.8	0.02	4.0	0.15	0.15	276.75	276.90
CB12	0.164	0.74	0.122	0.8	0.01	4.0	0.15	0.15	277.00	277.15

Orifice Equation: $Q = C^*A*(2*g*h)^{1.5}$

Solve for h yields: $(Q/(C^*A))^2/(2*g)$

Weir Flow Equation: $Q=3.3*P(h)^{1.5}$

Solve for h yields: $(Q/(3.3*P))^{0.667}$

Project: 444 East Central Street
Location: Franklin, MA
Prepared For: Allen & Major Associates



Purpose: To calculate the water quality flow rate (WQF) over a given site area. In this situation the WQF is derived from the first 1" of runoff from the contributing impervious surface.

Reference: Massachusetts Dept. of Environmental Protection Wetlands Program / United States Department of Agriculture Natural Resources Conservation Service TR-55 Manual

Procedure: Determine unit peak discharge using Figure 3 or 4. Figure 4 is in tabular form so is preferred. Using the t_c , read the unit peak discharge (q_u) from Figure 3 or Table in Figure 4. q_u is expressed in the following units: cfs/mi²/watershed inches (csm/in).

Compute Q Rate using the following equation:

$$Q = (q_u) (A) (WQV)$$

where:

Q = flow rate associated with first 1" of runoff

q_u = the unit peak discharge, in csm/in.

A = impervious surface drainage area (in square miles)

WQV = water quality volume in watershed inches (1" in this case)

Structure Name	Impv. (acres)	A (miles ²)	t_c (min)	t_c (hr)	WQV (in)	q_u (csm/in.)	Q (cfs)
CB 1	0.21	0.0003346	6.0	0.100	1.00	774.00	0.26
DMH 26	0.66	0.0010314	6.0	0.100	1.00	774.00	0.80
DMH 28	0.35	0.0005540	6.0	0.100	1.00	774.00	0.43
CB 6	0.22	0.0003507	6.0	0.100	1.00	774.00	0.27
DMH 8	0.33	0.0005092	6.0	0.100	1.00	774.00	0.39
CB 8	0.21	0.0003204	6.0	0.100	1.00	774.00	0.25
DMH 12	0.55	0.0008638	6.0	0.100	1.00	774.00	0.67
DMH 15	0.26	0.0004081	6.0	0.100	1.00	774.00	0.32
CB 11	0.23	0.0003517	6.0	0.100	1.00	774.00	0.27
DMH 21	0.90	0.0014057	6.0	0.100	1.00	774.00	1.09

The WQf sizing calculation selects the minimum size CDS/Cascade/StormCeptor model capable of operating at the computed WQf peak flowrate prior to bypassing. It assumes free discharge of the WQf through the unit and ignores the routing effect of any upstream storm drain piping. As with all hydrodynamic separators, there will be some impact to the Hydraulic Gradient of the corresponding drainage system, and evaluation of this impact should be considered in the design.



Brief Stormceptor Sizing Report - CB 1

Project Information & Location			
Project Name	444 East Central Street	Project Number	863260
City	Franklin	State/ Province	Massachusetts
Country	United States of America	Date	7/22/2025
Designer Information		EOR Information (optional)	
Name	David Adams	Name	
Company	Contech Engineered Solutions	Company	Allen & Major
Phone #	207-885-6191	Phone #	
Email	dadams@conteches.com	Email	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	CB 1
Target TSS Removal (%)	80
TSS Removal (%) Provided	93
Recommended Stormceptor Model	STC 450i

The recommended Stormceptor Model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
Stormceptor Model	% TSS Removal Provided
STC 450i	93
STC 900	96
STC 1200	97
STC 1800	97
STC 2400	98
STC 3600	98
STC 4800	99
STC 6000	99
STC 7200	99
STC 11000	99
STC 13000	99
STC 16000	100



Sizing Details			
Drainage Area		Water Quality Objective	
Total Area (acres)	0.21	TSS Removal (%)	80.0
Imperviousness %	100.0	Runoff Volume Capture (%)	
Rainfall		Oil Spill Capture Volume (Gal)	
Station Name	BLUE HILL	Peak Conveyed Flow Rate (CFS)	
State/Province	Massachusetts	Water Quality Flow Rate (CFS)	
Station ID #	0736	Up Stream Storage	
Years of Records	58	Storage (ac-ft)	Discharge (cfs)
Latitude	42°12'44"N	0.000	0.000
Longitude	71°6'53"W	Up Stream Flow Diversion	
		Max. Flow to Stormceptor (cfs)	

Particle Size Distribution (PSD) The selected PSD defines TSS removal		
OK-110		
Particle Diameter (microns)	Distribution %	Specific Gravity
1.0	0.0	2.65
53.0	3.0	2.65
75.0	15.0	2.65
88.0	25.0	2.65
106.0	41.0	2.65
125.0	15.0	2.65
150.0	1.0	2.65
212.0	0.0	2.65

Notes
- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules. - Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed. - For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

For Stormceptor Specifications and Drawings Please Visit:
<https://www.conteches.com/technical-guides/search?filter=1WBC005EYX>

**CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION
BASED ON THE RATIONAL RAINFALL METHOD**

**444 EAST CENTRAL STREET
FRANKLIN, MA**

Area **0.66 ac**
Weighted C **0.9**
 t_c **6 min**
CDS Model **2015-4**

Unit Site Designation **DMH 26**
Rainfall Station # **68**

CDS Treatment Capacity **1.4 cfs**

<u>Rainfall Intensity¹</u> <u>(in/hr)</u>	<u>Percent Rainfall Volume¹</u>	<u>Cumulative Rainfall Volume</u>	<u>Total Flowrate (cfs)</u>	<u>Treated Flowrate (cfs)</u>	<u>Incremental Removal (%)</u>
0.02	9.3%	9.3%	0.01	0.01	9.3
0.04	9.5%	18.8%	0.02	0.02	9.5
0.06	8.7%	27.5%	0.04	0.04	8.7
0.08	10.1%	37.6%	0.05	0.05	10.1
0.10	7.2%	44.8%	0.06	0.06	7.1
0.12	6.0%	50.8%	0.07	0.07	5.9
0.14	6.3%	57.1%	0.08	0.08	6.2
0.16	5.6%	62.7%	0.10	0.10	5.5
0.18	4.7%	67.4%	0.11	0.11	4.6
0.20	3.6%	71.0%	0.12	0.12	3.5
0.25	8.2%	79.1%	0.15	0.15	7.9
0.50	14.9%	94.0%	0.30	0.30	13.6
0.75	3.2%	97.3%	0.45	0.45	2.8
1.00	1.2%	98.5%	0.59	0.59	1.0
1.50	0.7%	99.2%	0.89	0.89	0.5
2.00	0.8%	100.0%	1.19	1.19	0.5
					96.7
Removal Efficiency Adjustment ² =					6.5%
Predicted % Annual Rainfall Treated =					93.5%
Predicted Net Annual Load Removal Efficiency =					90.2%

1 - Based on 10 years of rainfall data from NCDC station 736, Blue Hill, Norfolk County, MA

2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.

**CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION
BASED ON THE RATIONAL RAINFALL METHOD**

**444 EAST CENTRAL STREET
FRANKLIN, MA**

Area **0.35 ac**
Weighted C **0.9**
 t_c **6 min**
CDS Model **2015-4**

Unit Site Designation **DMH 28**
Rainfall Station # **68**

CDS Treatment Capacity **1.4 cfs**

<u>Rainfall Intensity¹ (in/hr)</u>	<u>Percent Rainfall Volume¹</u>	<u>Cumulative Rainfall Volume</u>	<u>Total Flowrate (cfs)</u>	<u>Treated Flowrate (cfs)</u>	<u>Incremental Removal (%)</u>
0.02	9.3%	9.3%	0.01	0.01	9.3
0.04	9.5%	18.8%	0.01	0.01	9.5
0.06	8.7%	27.5%	0.02	0.02	8.7
0.08	10.1%	37.6%	0.03	0.03	10.1
0.10	7.2%	44.8%	0.03	0.03	7.2
0.12	6.0%	50.8%	0.04	0.04	6.0
0.14	6.3%	57.1%	0.04	0.04	6.3
0.16	5.6%	62.7%	0.05	0.05	5.6
0.18	4.7%	67.4%	0.06	0.06	4.7
0.20	3.6%	71.0%	0.06	0.06	3.6
0.25	8.2%	79.1%	0.08	0.08	8.1
0.50	14.9%	94.0%	0.16	0.16	14.3
0.75	3.2%	97.3%	0.24	0.24	3.0
1.00	1.2%	98.5%	0.32	0.32	1.1
1.50	0.7%	99.2%	0.47	0.47	0.6
2.00	0.8%	100.0%	0.63	0.63	0.6
					98.6
Removal Efficiency Adjustment ² =					6.5%
Predicted % Annual Rainfall Treated =					93.5%
Predicted Net Annual Load Removal Efficiency =					92.2%

1 - Based on 10 years of rainfall data from NCDC station 736, Blue Hill, Norfolk County, MA

2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.



Brief Stormceptor Sizing Report - CB 6

Project Information & Location			
Project Name	444 East Central Street	Project Number	863260
City	Franklin	State/ Province	Massachusetts
Country	United States of America	Date	7/22/2025
Designer Information		EOR Information (optional)	
Name	David Adams	Name	
Company	Contech Engineered Solutions	Company	Allen & Major
Phone #	207-885-6191	Phone #	
Email	dadams@conteches.com	Email	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	CB 6
Target TSS Removal (%)	80
TSS Removal (%) Provided	93
Recommended Stormceptor Model	STC 450i

The recommended Stormceptor Model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
Stormceptor Model	% TSS Removal Provided
STC 450i	93
STC 900	96
STC 1200	97
STC 1800	97
STC 2400	98
STC 3600	98
STC 4800	99
STC 6000	99
STC 7200	99
STC 11000	99
STC 13000	99
STC 16000	100



Sizing Details			
Drainage Area		Water Quality Objective	
Total Area (acres)	0.22	TSS Removal (%)	80.0
Imperviousness %	100.0	Runoff Volume Capture (%)	
Rainfall		Oil Spill Capture Volume (Gal)	
Station Name	BLUE HILL	Peak Conveyed Flow Rate (CFS)	
State/Province	Massachusetts	Water Quality Flow Rate (CFS)	
Station ID #	0736	Up Stream Storage	
Years of Records	58	Storage (ac-ft)	Discharge (cfs)
Latitude	42°12'44"N	0.000	0.000
Longitude	71°6'53"W	Up Stream Flow Diversion	
		Max. Flow to Stormceptor (cfs)	

Particle Size Distribution (PSD) The selected PSD defines TSS removal		
OK-110		
Particle Diameter (microns)	Distribution %	Specific Gravity
1.0	0.0	2.65
53.0	3.0	2.65
75.0	15.0	2.65
88.0	25.0	2.65
106.0	41.0	2.65
125.0	15.0	2.65
150.0	1.0	2.65
212.0	0.0	2.65

Notes
- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules. - Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed. - For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

For Stormceptor Specifications and Drawings Please Visit:
<https://www.conteches.com/technical-guides/search?filter=1WBC005EYX>

**444 EAST CENTRAL STREET
FRANKLIN, MA**

CDS Treatment Capacity **1.0 cfs**

[illegible]

1 - Based on 10 years of rainfall data from NCDC station 736, Blue Hill, Norfolk County, MA

2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.



Brief Stormceptor Sizing Report - CB 8

Project Information & Location			
Project Name	444 East Central Street	Project Number	863260
City	Franklin	State/ Province	Massachusetts
Country	United States of America	Date	7/22/2025
Designer Information		EOR Information (optional)	
Name	David Adams	Name	
Company	Contech Engineered Solutions	Company	Allen & Major
Phone #	207-885-6191	Phone #	
Email	dadams@conteches.com	Email	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	CB 8
Target TSS Removal (%)	80
TSS Removal (%) Provided	93
Recommended Stormceptor Model	STC 450i

The recommended Stormceptor Model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
Stormceptor Model	% TSS Removal Provided
STC 450i	93
STC 900	96
STC 1200	97
STC 1800	97
STC 2400	98
STC 3600	98
STC 4800	99
STC 6000	99
STC 7200	99
STC 11000	99
STC 13000	99
STC 16000	100



Sizing Details			
Drainage Area		Water Quality Objective	
Total Area (acres)	0.21	TSS Removal (%)	80.0
Imperviousness %	100.0	Runoff Volume Capture (%)	
Rainfall		Oil Spill Capture Volume (Gal)	
Station Name	BLUE HILL	Peak Conveyed Flow Rate (CFS)	
State/Province	Massachusetts	Water Quality Flow Rate (CFS)	
Station ID #	0736	Up Stream Storage	
Years of Records	58	Storage (ac-ft)	Discharge (cfs)
Latitude	42°12'44"N	0.000	0.000
Longitude	71°6'53"W	Up Stream Flow Diversion	
		Max. Flow to Stormceptor (cfs)	

Particle Size Distribution (PSD) The selected PSD defines TSS removal		
OK-110		
Particle Diameter (microns)	Distribution %	Specific Gravity
1.0	0.0	2.65
53.0	3.0	2.65
75.0	15.0	2.65
88.0	25.0	2.65
106.0	41.0	2.65
125.0	15.0	2.65
150.0	1.0	2.65
212.0	0.0	2.65

Notes
- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules. - Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed. - For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

For Stormceptor Specifications and Drawings Please Visit:
<https://www.conteches.com/technical-guides/search?filter=1WBC005EYX>

CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION BASED ON THE RATIONAL RAINFALL METHOD

**444 EAST CENTRAL STREET
FRANKLIN, MA**

Area	0.55 ac
Weighted C	0.9
t_c	6 min
CDS Model	1515-3

Unit Site Designation **DMH 12**
Rainfall Station # **68**

CDS Treatment Capacity **1.0 cfs**

[illegible]

1 - Based on 10 years of rainfall data from NCDC station 736, Blue Hill, Norfolk County, MA

2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.

CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION BASED ON THE RATIONAL RAINFALL METHOD

**444 EAST CENTRAL STREET
FRANKLIN, MA**

Area	0.26 ac
Weighted C	0.9
t_c	6 min
CDS Model	1515-3

Unit Site Designation
Rainfall Station #

CDS Treatment Capacity **1.0 cfs**

[illegible]

1 - Based on 10 years of rainfall data from NCDC station 736, Blue Hill, Norfolk County, MA

2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.



Brief Stormceptor Sizing Report - CB 11

Project Information & Location			
Project Name	444 East Central Street	Project Number	863260
City	Franklin	State/ Province	Massachusetts
Country	United States of America	Date	7/22/2025
Designer Information		EOR Information (optional)	
Name	David Adams	Name	
Company	Contech Engineered Solutions	Company	Allen & Major
Phone #	207-885-6191	Phone #	
Email	dadams@conteches.com	Email	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	CB 11
Target TSS Removal (%)	80
TSS Removal (%) Provided	93
Recommended Stormceptor Model	STC 450i

The recommended Stormceptor Model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
Stormceptor Model	% TSS Removal Provided
STC 450i	93
STC 900	96
STC 1200	96
STC 1800	97
STC 2400	98
STC 3600	98
STC 4800	99
STC 6000	99
STC 7200	99
STC 11000	99
STC 13000	99
STC 16000	99



Sizing Details			
Drainage Area		Water Quality Objective	
Total Area (acres)	0.23	TSS Removal (%)	80.0
Imperviousness %	100.0	Runoff Volume Capture (%)	
Rainfall		Oil Spill Capture Volume (Gal)	
Station Name	BLUE HILL	Peak Conveyed Flow Rate (CFS)	
State/Province	Massachusetts	Water Quality Flow Rate (CFS)	
Station ID #	0736	Up Stream Storage	
Years of Records	58	Storage (ac-ft)	Discharge (cfs)
Latitude	42°12'44"N	0.000	0.000
Longitude	71°6'53"W	Up Stream Flow Diversion	
		Max. Flow to Stormceptor (cfs)	

Particle Size Distribution (PSD) The selected PSD defines TSS removal		
OK-110		
Particle Diameter (microns)	Distribution %	Specific Gravity
1.0	0.0	2.65
53.0	3.0	2.65
75.0	15.0	2.65
88.0	25.0	2.65
106.0	41.0	2.65
125.0	15.0	2.65
150.0	1.0	2.65
212.0	0.0	2.65

Notes
- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules. - Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed. - For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

For Stormceptor Specifications and Drawings Please Visit:
<https://www.conteches.com/technical-guides/search?filter=1WBC005EYX>

**CDS ESTIMATED NET ANNUAL SOLIDS LOAD REDUCTION
BASED ON THE RATIONAL RAINFALL METHOD**

**444 EAST CENTRAL STREET
FRANKLIN, MA**

Area **0.90 ac**
Weighted C **0.9**
 t_c **6 min**
CDS Model **2015-4**

Unit Site Designation **DMH 21**
Rainfall Station # **68**

CDS Treatment Capacity **1.4 cfs**

<u>Rainfall Intensity¹ (in/hr)</u>	<u>Percent Rainfall Volume¹</u>	<u>Cumulative Rainfall Volume</u>	<u>Total Flowrate (cfs)</u>	<u>Treated Flowrate (cfs)</u>	<u>Incremental Removal (%)</u>
0.02	9.3%	9.3%	0.02	0.02	9.3
0.04	9.5%	18.8%	0.03	0.03	9.5
0.06	8.7%	27.5%	0.05	0.05	8.7
0.08	10.1%	37.6%	0.06	0.06	10.0
0.10	7.2%	44.8%	0.08	0.08	7.1
0.12	6.0%	50.8%	0.10	0.10	5.9
0.14	6.3%	57.1%	0.11	0.11	6.2
0.16	5.6%	62.7%	0.13	0.13	5.4
0.18	4.7%	67.4%	0.15	0.15	4.5
0.20	3.6%	71.0%	0.16	0.16	3.5
0.25	8.2%	79.1%	0.20	0.20	7.7
0.50	14.9%	94.0%	0.41	0.41	13.1
0.75	3.2%	97.3%	0.61	0.61	2.6
1.00	1.2%	98.5%	0.81	0.81	0.9
1.50	0.7%	99.2%	1.22	1.22	0.4
2.00	0.8%	100.0%	1.62	1.40	0.4
					95.1
Removal Efficiency Adjustment ² =					6.5%
Predicted % Annual Rainfall Treated =					93.4%
Predicted Net Annual Load Removal Efficiency =					88.6%

1 - Based on 10 years of rainfall data from NCDC station 736, Blue Hill, Norfolk County, MA

2 - Reduction due to use of 60-minute data for a site that has a time of concentration less than 30-minutes.

Illicit Discharge Compliance Statement

Responsibility:

The Owner is responsible for ultimate compliance with all provisions of the Massachusetts Stormwater Management Policy, the USEPA NPDES Construction General Permit and responsible for identifying and eliminating illicit discharges (as defined by the USEPA).

OWNER NAME: TAG Central LLC

ADDRESS: 275 Regatta Drive
Jupiter, FL 33477

TEL. NUMBER: (561) 685-5336

Engineer's Compliance Statement:

To the best of my knowledge, the attached plans, computations and specifications meet the requirements of Standard 10 of the Massachusetts Stormwater Handbook regarding illicit discharges to the stormwater management system and that no detectable illicit discharges exist on the site. All documents and attachments were prepared under my direction and qualified personnel properly gathered and evaluated the information submitted, to the best of my knowledge.

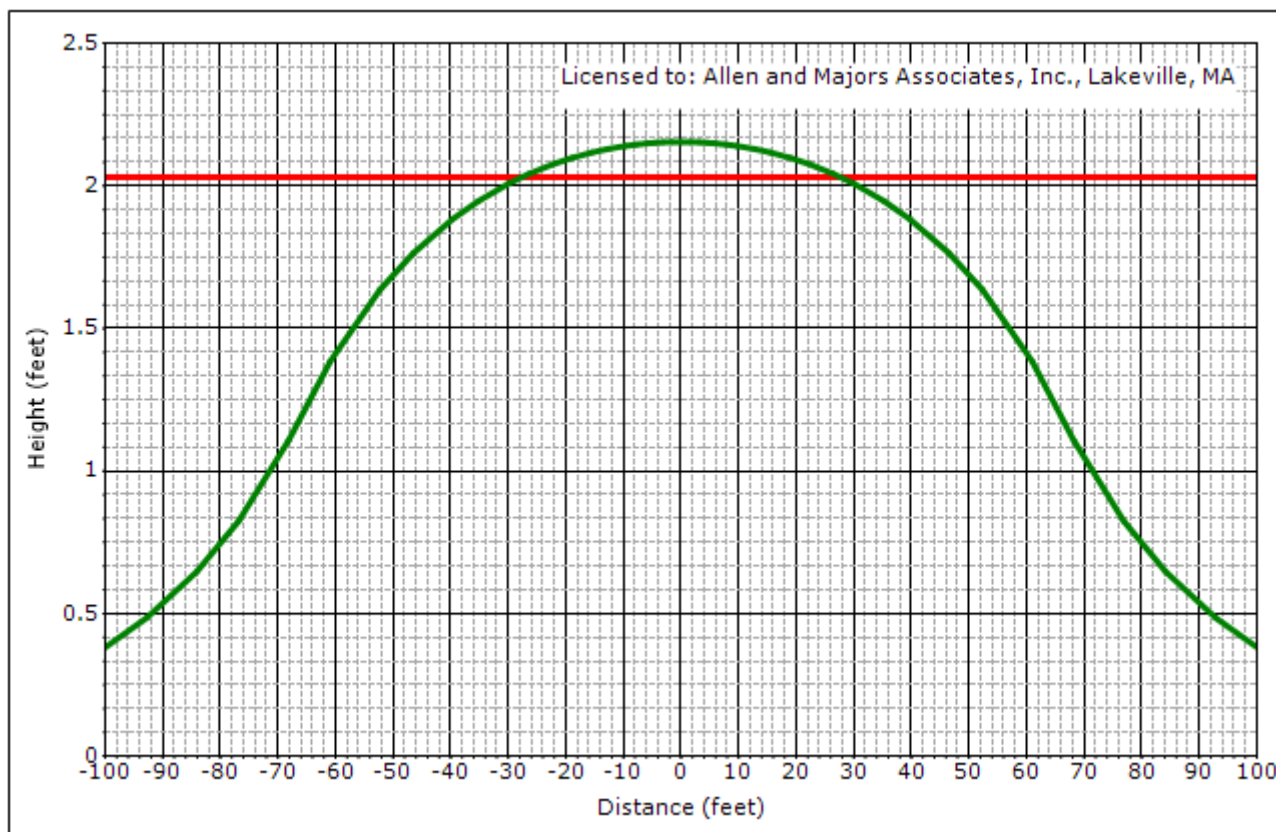
Included with this statement are site plans, drawn to scale, that identify the location of systems for conveying stormwater on the site and show that these systems do not allow the entry of any illicit discharges into the stormwater management system. The plans also show any systems for conveying wastewater and/or groundwater on the site and show that there are no connections between the stormwater and wastewater systems.

For a redevelopment project (if applicable), all actions taken to identify and remove illicit discharges, including without limitation, visual screening, dye or smoke testing, and the removal of any sources of illicit discharges to the stormwater management system are documented and included with this statement.



Alexander Alevizos
Principal

Groundwater Mounding Analysis (Hantush Method using Glover's Solution)



Company: Allen & Major
 Project: Associates, Inc
 444 E Central St
 Franklin MA (UIS-1)
 Analyst: Carlton Quinn
 Date: 07/23/2025

Recharge Basin Dimensions

Length (w): 131.92 ft
 Width (l): 33.85 ft
 Bottom Area: 4,465 ft²
 SHGW Separation: 2.03 ft

Recharge Rate Calculations

Duration (t): 1 d
 Volume (V): 9,171 ft³
 Rate (R): 2.05 ft/d
 Total Simulation Time: 4 d

Aquifer Characteristics

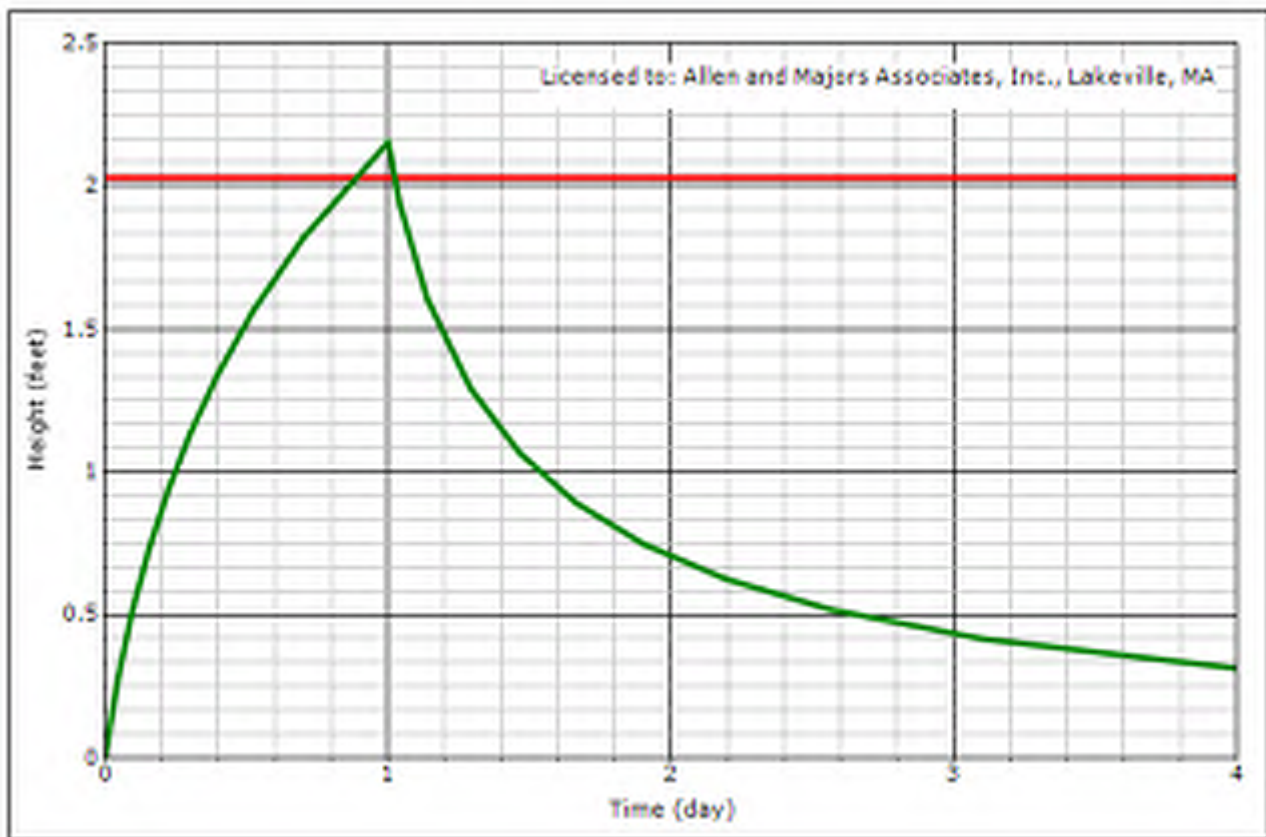
Hydraulic Conductivity (Kh): 82.7 ft/d
 Drainable Porosity(Sy): 0.3
 Saturated Thickness (h): 7.67 ft

Plot Geometry

X-Coordinate: 0 ft
 Y-Coordinate: 0 ft
 Left Side Plot Distance (Dl): 100 ft
 Right Side Plot Distance (Dr): 100 ft
 Plot Angle From Y-Axis (Φ): 0
 Constant Head Boundary: No

<u>Distance</u> <u>(ft)</u>	<u>Height</u> <u>(ft)</u>	<u>Distance</u> <u>(ft)</u>	<u>Height</u> <u>(ft)</u>
-100.00	0.3812	0.00	2.1544
-92.78	0.4853	1.60	2.1540
-84.10	0.6474	3.15	2.1529
-76.87	0.8238	4.50	2.1513
-68.19	1.1065	5.80	2.1492
-60.96	1.3842	7.79	2.1451
-52.29	1.6363	9.69	2.1400
-46.49	1.7644	12.69	2.1295
-39.79	1.8819	15.49	2.1172
-35.21	1.9468	18.96	2.0981
-30.10	2.0065	22.18	2.0766
-26.32	2.0432	26.32	2.0432
-22.18	2.0766	30.10	2.0065
-18.96	2.0981	35.21	1.9468
-15.49	2.1172	39.79	1.8819
-12.69	2.1295	46.49	1.7644
-9.69	2.1400	52.29	1.6363
-7.79	2.1451	60.96	1.3842
-5.80	2.1492	68.19	1.1065
-4.50	2.1513	76.87	0.8238
-3.15	2.1529	84.10	0.6474
-1.60	2.1540	92.78	0.4853
		100.00	0.3812

Groundwater Mounding Analysis (Hantush Method using Glover's Solution)



Company: Allen & Major
 Project: Associates, Inc
 444 E Central St
 Franklin MA (UIS-1)
 Analyst: Carlton Quinn
 Date: 07/23/2025

Recharge Basin Dimensions

Length (w): 131.92 ft
 Width (l): 33.85 ft
 Bottom Area: 4,465 ft²
 SHGW Separation: 2.03 ft

Recharge Rate Calculations

Duration (t): 1 d
 Volume (V): 9,171 ft³
 Rate (R): 2.05 ft/d
 Total Simulation Time: 4 d

Aquifer Characteristics

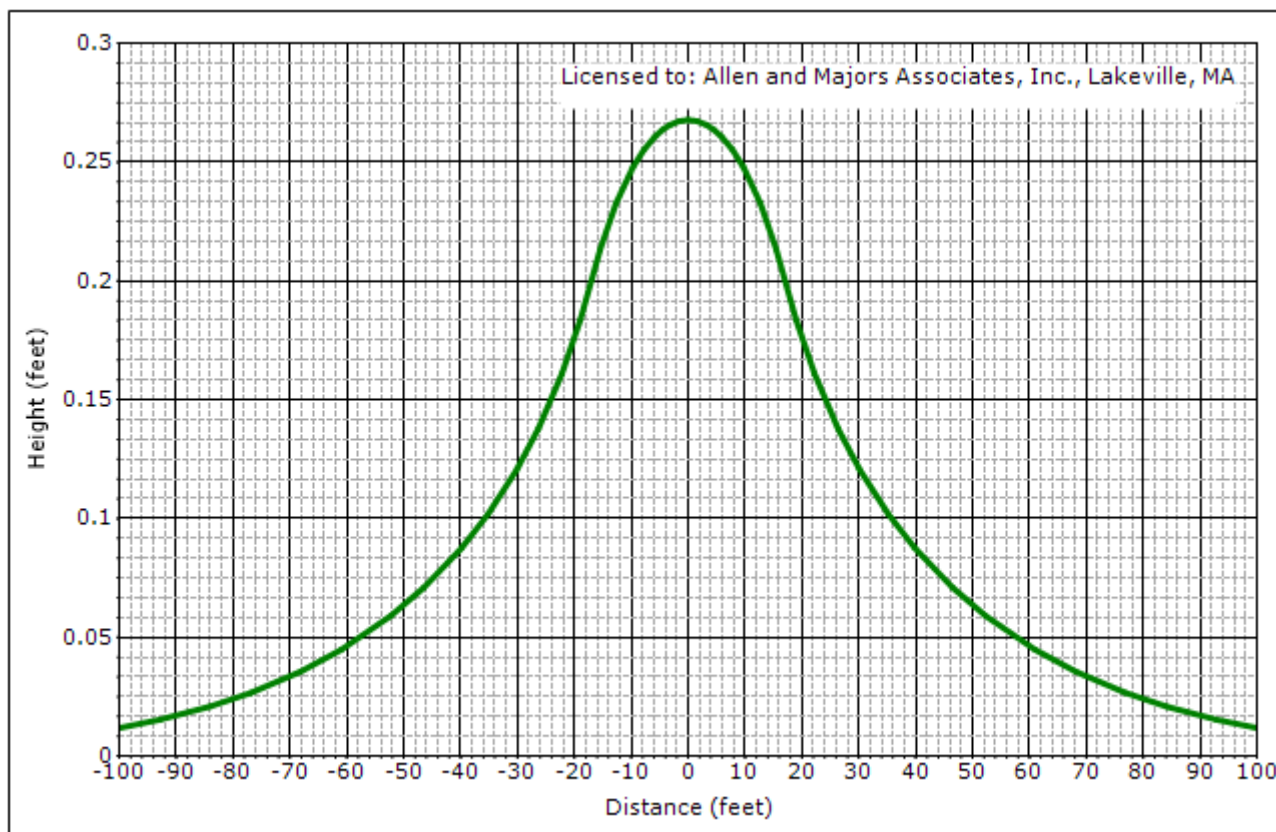
Hydraulic Conductivity (Kh): 82.7 ft/d
 Drainable Porosity(Sy): 0.3
 Saturated Thickness (h): 7.67 ft

Plot Geometry

X-Coordinate: 0 ft
 Y-Coordinate: 0 ft
 Left Side Plot Distance (Dl): 100 ft
 Right Side Plot Distance (Dr): 100 ft
 Plot Angle From Y-Axis (Φ): 0
 Constant Head Boundary: No

<u>Time (d)</u>	<u>Height (ft)</u>
0.00	0.0000
0.01	0.0899
0.05	0.2843
0.10	0.5201
0.15	0.7328
0.22	0.9357
0.30	1.1370
0.40	1.3434
0.52	1.5648
0.70	1.8187
1.00	2.1544
1.04	1.9414
1.14	1.6072
1.29	1.2933
1.46	1.0670
1.67	0.8914
1.90	0.7480
2.19	0.6262
2.57	0.5182
3.10	0.4174
4.00	0.3137

Groundwater Mounding Analysis (Hantush Method using Glover's Solution)



Company: Allen & Major
 Project: Associates, Inc
 444 E Central St
 Franklin MA (UIS-1)
 Analyst: Carlton Quinn
 Date: 07/23/2025

Recharge Basin Dimensions

Length (w): 33.68 ft
 Width (l): 14.83 ft
 Bottom Area: 499 ft²
 SHGW Separation: 2 ft

Recharge Rate Calculations

Duration (t): 1 d
 Volume (V): 508 ft³
 Rate (R): 1.08 ft/d
 Total Simulation Time: 4 d

Aquifer Characteristics

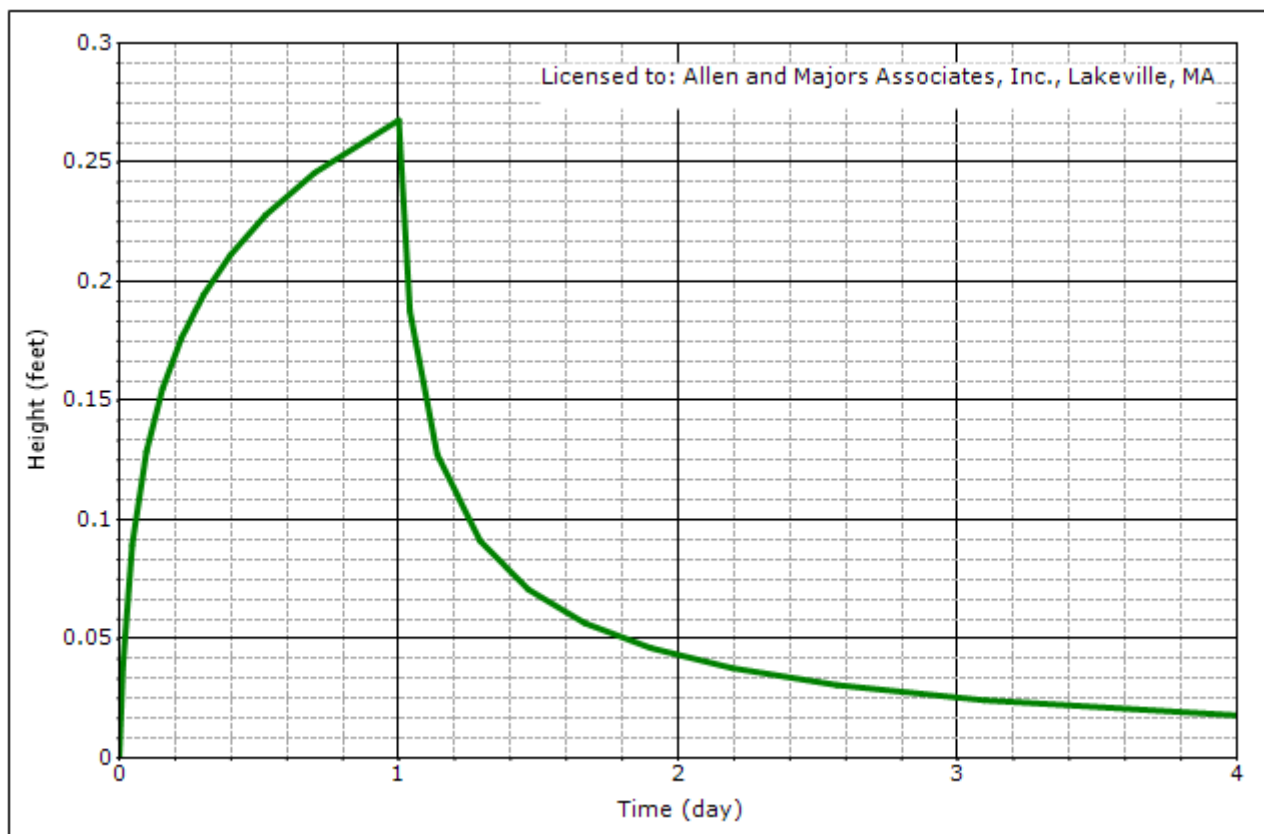
Hydraulic Conductivity (Kh): 82.7 ft/d
 Drainable Porosity(Sy): 0.3
 Saturated Thickness (h): 8.3 ft

Plot Geometry

X-Coordinate: 0 ft
 Y-Coordinate: 0 ft
 Left Side Plot Distance (Dl): 100 ft
 Right Side Plot Distance (Dr): 100 ft
 Plot Angle From Y-Axis (Φ): 0
 Constant Head Boundary: No

<u>Distance</u> <u>(ft)</u>	<u>Height</u> <u>(ft)</u>	<u>Distance</u> <u>(ft)</u>	<u>Height</u> <u>(ft)</u>
-100.00	0.0119	0.00	0.2677
-92.78	0.0154	1.60	0.2672
-84.10	0.0208	3.15	0.2657
-76.87	0.0266	4.50	0.2636
-68.19	0.0355	5.80	0.2609
-60.96	0.0448	7.79	0.2552
-52.29	0.0590	9.69	0.2480
-46.49	0.0709	12.69	0.2327
-39.79	0.0877	15.49	0.2132
-35.21	0.1017	18.96	0.1837
-30.10	0.1208	22.18	0.1611
-26.32	0.1379	26.32	0.1379
-22.18	0.1611	30.10	0.1208
-18.96	0.1837	35.21	0.1017
-15.49	0.2132	39.79	0.0877
-12.69	0.2327	46.49	0.0709
-9.69	0.2480	52.29	0.0590
-7.79	0.2552	60.96	0.0448
-5.80	0.2609	68.19	0.0355
-4.50	0.2636	76.87	0.0266
-3.15	0.2657	84.10	0.0208
-1.60	0.2672	92.78	0.0154
		100.00	0.0119

Groundwater Mounding Analysis (Hantush Method using Glover's Solution)



Company: Allen & Major
 Project: Associates, Inc
 444 E Central St
 Franklin MA (UIS-1)
 Analyst: Carlton Quinn
 Date: 07/23/2025

Recharge Basin Dimensions

Length (w): 33.68 ft
 Width (l): 14.83 ft
 Bottom Area: 499 ft²
 SHGW Separation: 2 ft

Recharge Rate Calculations

Duration (t): 1 d
 Volume (V): 508 ft³
 Rate (R): 1.08 ft/d
 Total Simulation Time: 4 d

Aquifer Characteristics

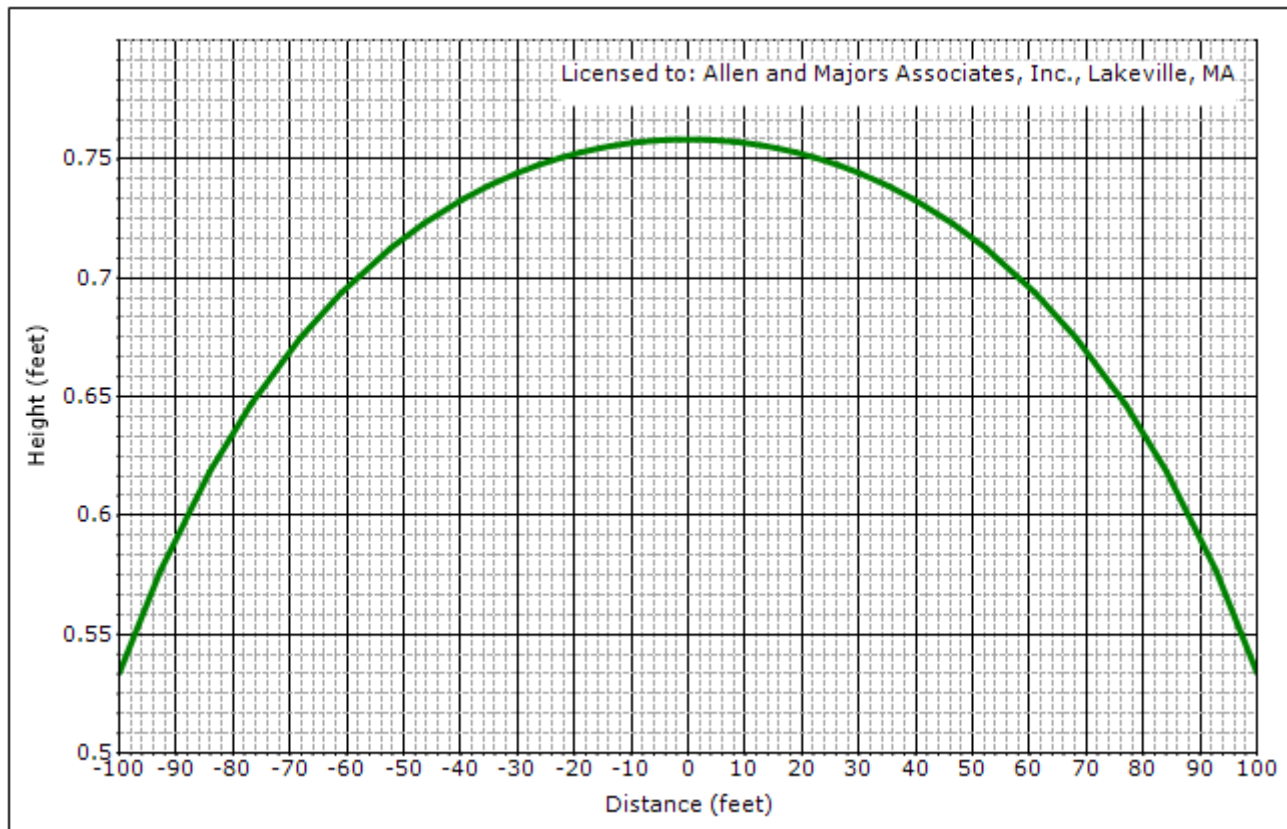
Hydraulic Conductivity (Kh): 82.7 ft/d
 Drainable Porosity(Sy): 0.3
 Saturated Thickness (h): 8.3 ft

Plot Geometry

X-Coordinate: 0 ft
 Y-Coordinate: 0 ft
 Left Side Plot Distance (Dl): 100 ft
 Right Side Plot Distance (Dr): 100 ft
 Plot Angle From Y-Axis (Φ): 0
 Constant Head Boundary: No

<u>Time (d)</u>	<u>Height (ft)</u>
0.00	0.0000
0.01	0.0394
0.05	0.0894
0.10	0.1286
0.15	0.1552
0.22	0.1762
0.30	0.1944
0.40	0.2112
0.52	0.2279
0.70	0.2457
1.00	0.2677
1.04	0.1877
1.14	0.1274
1.29	0.0912
1.46	0.0707
1.67	0.0567
1.90	0.0462
2.19	0.0378
2.57	0.0307
3.10	0.0243
4.00	0.0179

Groundwater Mounding Analysis (Hantush Method using Glover's Solution)



Company: Allen & Major
 Project: Associates, Inc
 444 E Central St
 Franklin MA (UIS-1)
 Analyst: Carlton Quinn
 Date: 07/23/2025

Recharge Basin Dimensions

Length (w): 231.5 ft
 Width (l): 30 ft
 Bottom Area: 6,945 ft²
 SHGW Separation: 2 ft

Recharge Rate Calculations

Duration (t): 1 d
 Volume (V): 7,065 ft³
 Rate (R): 1.02 ft/d
 Total Simulation Time: 4 d

Aquifer Characteristics

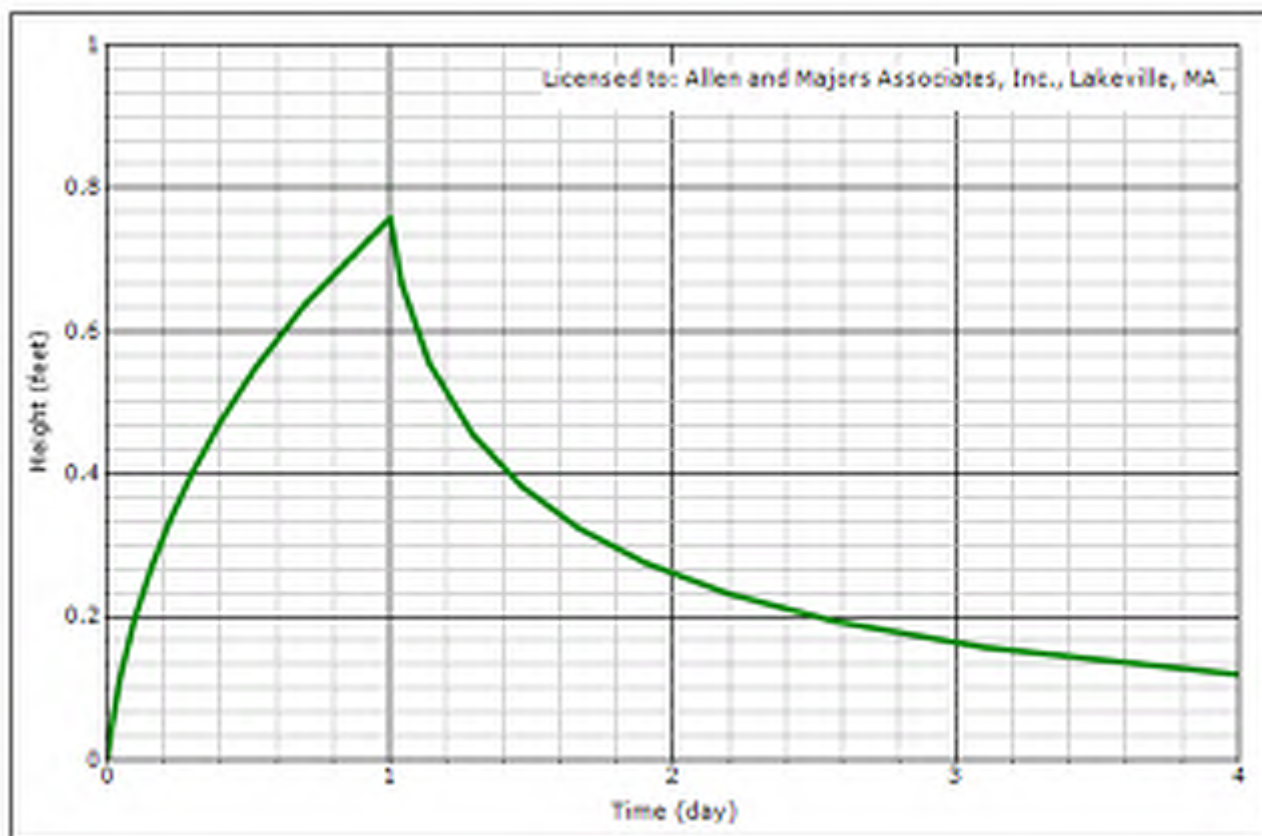
Hydraulic Conductivity (Kh): 82.7 ft/d
 Drainable Porosity(Sy): 0.3
 Saturated Thickness (h): 15.2 ft

Plot Geometry

X-Coordinate: 0 ft
 Y-Coordinate: 0 ft
 Left Side Plot Distance (Dl): 100 ft
 Right Side Plot Distance (Dr): 100 ft
 Plot Angle From Y-Axis (Φ): 0
 Constant Head Boundary: No

<u>Distance</u> <u>(ft)</u>	<u>Height</u> <u>(ft)</u>	<u>Distance</u> <u>(ft)</u>	<u>Height</u> <u>(ft)</u>
-100.00	0.5338	0.00	0.7583
-92.78	0.5768	1.60	0.7583
-84.10	0.6185	3.15	0.7582
-76.87	0.6468	4.50	0.7580
-68.19	0.6747	5.80	0.7578
-60.96	0.6938	7.79	0.7574
-52.29	0.7126	9.69	0.7569
-46.49	0.7229	12.69	0.7559
-39.79	0.7329	15.49	0.7546
-35.21	0.7387	18.96	0.7528
-30.10	0.7441	22.18	0.7507
-26.32	0.7475	26.32	0.7475
-22.18	0.7507	30.10	0.7441
-18.96	0.7528	35.21	0.7387
-15.49	0.7546	39.79	0.7329
-12.69	0.7559	46.49	0.7229
-9.69	0.7569	52.29	0.7126
-7.79	0.7574	60.96	0.6938
-5.80	0.7578	68.19	0.6747
-4.50	0.7580	76.87	0.6468
-3.15	0.7582	84.10	0.6185
-1.60	0.7583	92.78	0.5768
		100.00	0.5338

Groundwater Mounding Analysis (Hantush Method using Glover's Solution)



Company: Allen & Major
 Project: Associates, Inc
 444 E Central St
 Franklin MA (UIS-1)
 Analyst: Carlton Quinn
 Date: 07/23/2025

Recharge Basin Dimensions

Length (w): 231.5 ft
 Width (l): 30 ft
 Bottom Area: 6,945 ft²
 SHGW Separation: 2 ft

Recharge Rate Calculations

Duration (t): 1 d
 Volume (V): 7,065 ft³
 Rate (R): 1.02 ft/d
 Total Simulation Time: 4 d

Aquifer Characteristics

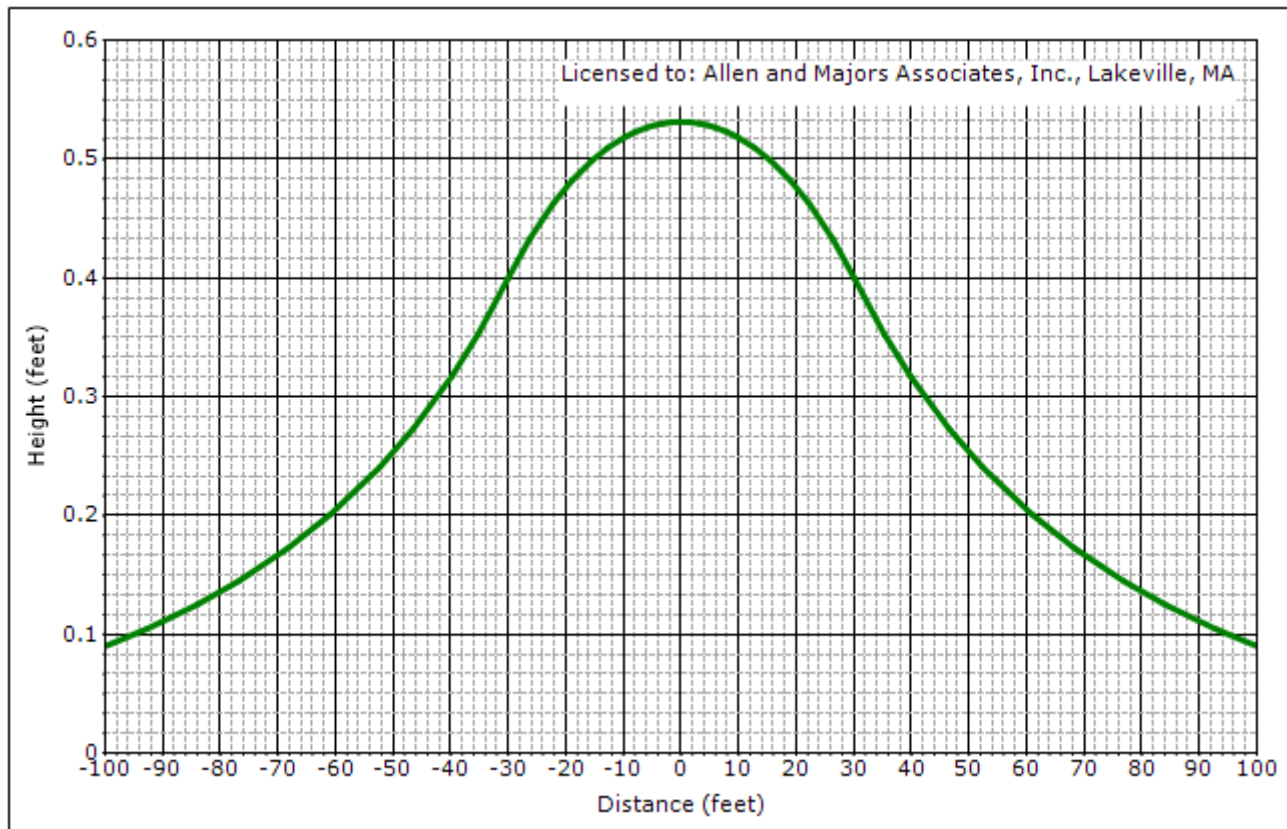
Hydraulic Conductivity (Kh): 82.7 ft/d
 Drainable Porosity(Sy): 0.3
 Saturated Thickness (h): 15.2 ft

Plot Geometry

X-Coordinate: 0 ft
 Y-Coordinate: 0 ft
 Left Side Plot Distance (Dl): 100 ft
 Right Side Plot Distance (Dr): 100 ft
 Plot Angle From Y-Axis (Φ): 0
 Constant Head Boundary: No

<u>Time (d)</u>	<u>Height (ft)</u>
0.00	0.0000
0.01	0.0424
0.05	0.1171
0.10	0.1981
0.15	0.2687
0.22	0.3356
0.30	0.4025
0.40	0.4723
0.52	0.5485
0.70	0.6378
1.00	0.7583
1.04	0.6670
1.14	0.5552
1.29	0.4559
1.46	0.3829
1.67	0.3245
1.90	0.2755
2.19	0.2329
2.57	0.1944
3.10	0.1578
4.00	0.1196

Groundwater Mounding Analysis (Hantush Method using Glover's Solution)



Company: Allen & Major
 Project: Associates, Inc
 444 E Central St
 Franklin MA (UIS-1)
 Analyst: Carlton Quinn
 Date: 07/23/2025

Recharge Basin Dimensions

Length (w): 60.7 ft
 Width (l): 39.5 ft
 Bottom Area: 2,398 ft²
 SHGW Separation: 2 ft

Recharge Rate Calculations

Duration (t): 1 d
 Volume (V): 2,873 ft³
 Rate (R): 1.2 ft/d
 Total Simulation Time: 4 d

Aquifer Characteristics

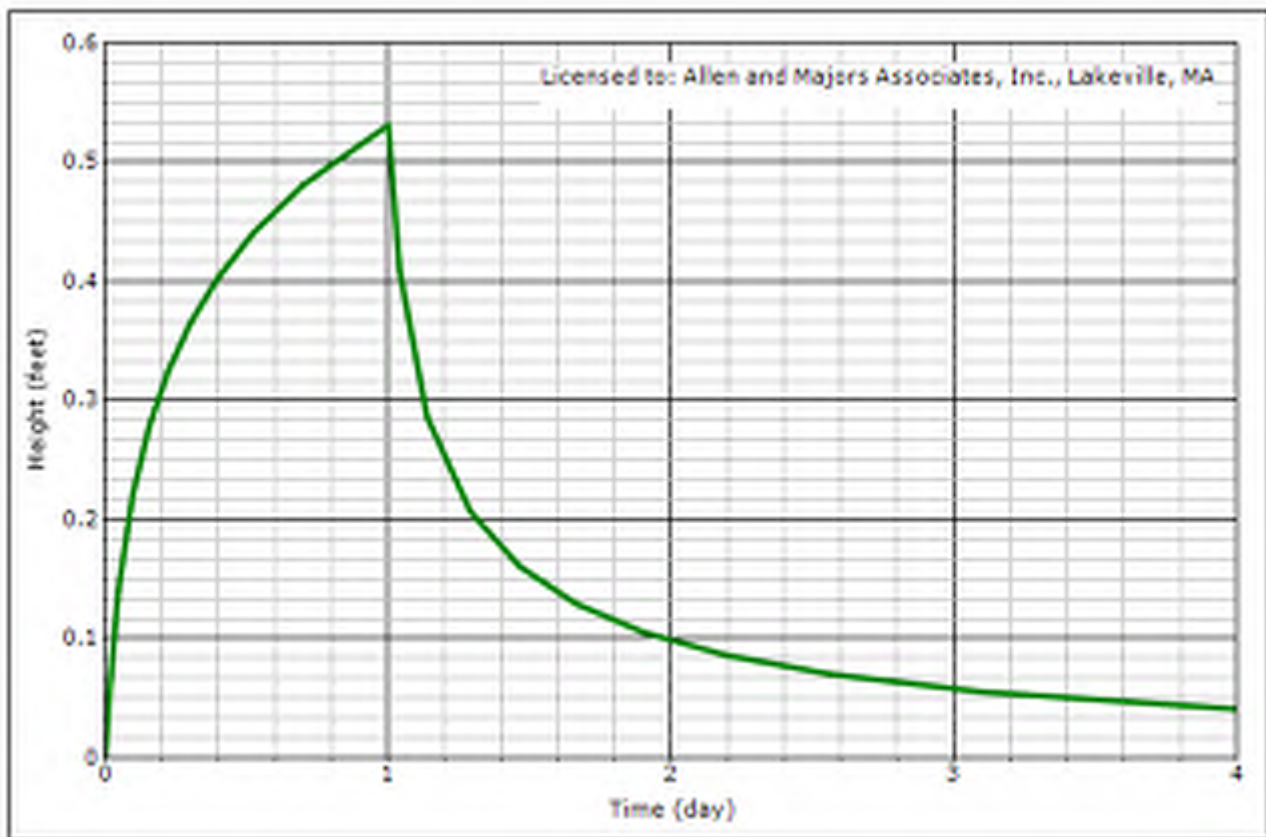
Hydraulic Conductivity (Kh): 82.7 ft/d
 Drainable Porosity(Sy): 0.3
 Saturated Thickness (h): 19.25 ft

Plot Geometry

X-Coordinate: 0 ft
 Y-Coordinate: 0 ft
 Left Side Plot Distance (Dl): 100 ft
 Right Side Plot Distance (Dr): 100 ft
 Plot Angle From Y-Axis (Φ): 0
 Constant Head Boundary: No

<u>Distance</u> <u>(ft)</u>	<u>Height</u> <u>(ft)</u>	<u>Distance</u> <u>(ft)</u>	<u>Height</u> <u>(ft)</u>
-100.00	0.0900	0.00	0.5313
-92.78	0.1045	1.60	0.5310
-84.10	0.1248	3.15	0.5300
-76.87	0.1447	4.50	0.5286
-68.19	0.1728	5.80	0.5269
-60.96	0.2007	7.79	0.5233
-52.29	0.2409	9.69	0.5188
-46.49	0.2732	12.69	0.5097
-39.79	0.3174	15.49	0.4989
-35.21	0.3531	18.96	0.4820
-30.10	0.3994	22.18	0.4629
-26.32	0.4328	26.32	0.4328
-22.18	0.4629	30.10	0.3994
-18.96	0.4820	35.21	0.3531
-15.49	0.4989	39.79	0.3174
-12.69	0.5097	46.49	0.2732
-9.69	0.5188	52.29	0.2409
-7.79	0.5233	60.96	0.2007
-5.80	0.5269	68.19	0.1728
-4.50	0.5286	76.87	0.1447
-3.15	0.5300	84.10	0.1248
-1.60	0.5310	92.78	0.1045
		100.00	0.0900

Groundwater Mounding Analysis (Hantush Method using Glover's Solution)



Company: Allen & Major
 Project: Associates, Inc
 444 E Central St
 Franklin MA (UIS-1)
 Analyst: Carlton Quinn
 Date: 07/23/2025

Time (d) **Height**
 (ft)

0.00	0.0000
0.01	0.0512
0.05	0.1405
0.10	0.2210
0.15	0.2784
0.22	0.3247
0.30	0.3653
0.40	0.4031
0.52	0.4407
0.70	0.4811
1.00	0.5313
1.04	0.4098
1.14	0.2861
1.29	0.2067
1.46	0.1608
1.67	0.1293
1.90	0.1055
2.19	0.0864
2.57	0.0702
3.10	0.0556
4.00	0.0411

Recharge Basin Dimensions

Length (w): 60.7 ft
 Width (l): 39.5 ft
 Bottom Area: 2,398 ft²
 SHGW Separation: 2 ft

Recharge Rate Calculations

Duration (t): 1 d
 Volume (V): 2,873 ft³
 Rate (R): 1.2 ft/d
 Total Simulation Time: 4 d

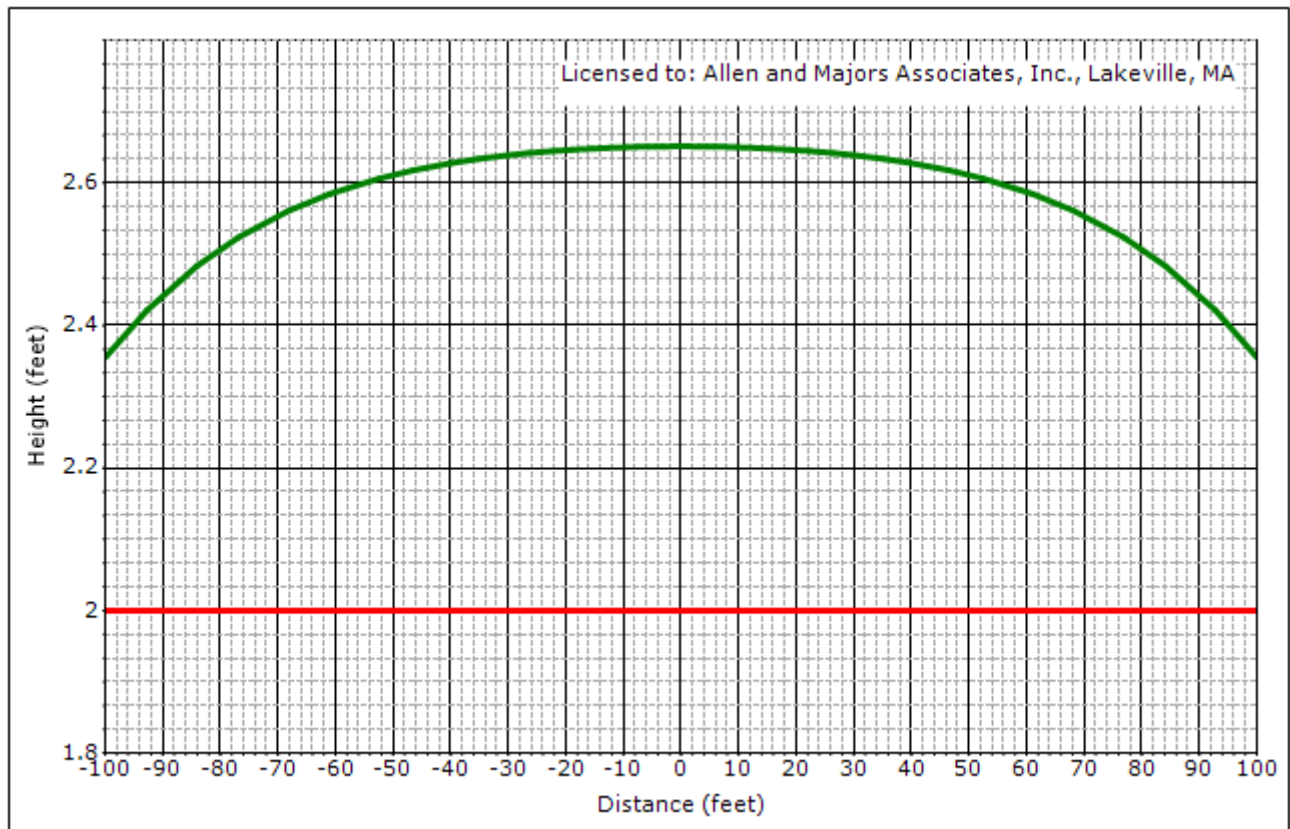
Aquifer Characteristics

Hydraulic Conductivity (Kh): 82.7 ft/d
 Drainable Porosity(Sy): 0.3
 Saturated Thickness (h): 19.25 ft

Plot Geometry

X-Coordinate: 0 ft
 Y-Coordinate: 0 ft
 Left Side Plot Distance (Dl): 100 ft
 Right Side Plot Distance (Dr): 100 ft
 Plot Angle From Y-Axis (Φ): 0
 Constant Head Boundary: No

Groundwater Mounding Analysis (Hantush Method using Glover's Solution)



Company: Allen & Major
Project: Associates, Inc
444 E Central St
Franklin MA (UIS-1)
Analyst: Carlton Quinn
Date: 07/23/2025

Recharge Basin Dimensions

Length (w): 288.43 ft
Width (l): 30 ft
Bottom Area: 8,653 ft²
SHGW Separation: 2 ft

Recharge Rate Calculations

Duration (t): 1 d
Volume (V): 21,847 ft³
Rate (R): 2.53 ft/d
Total Simulation Time: 4 d

Aquifer Characteristics

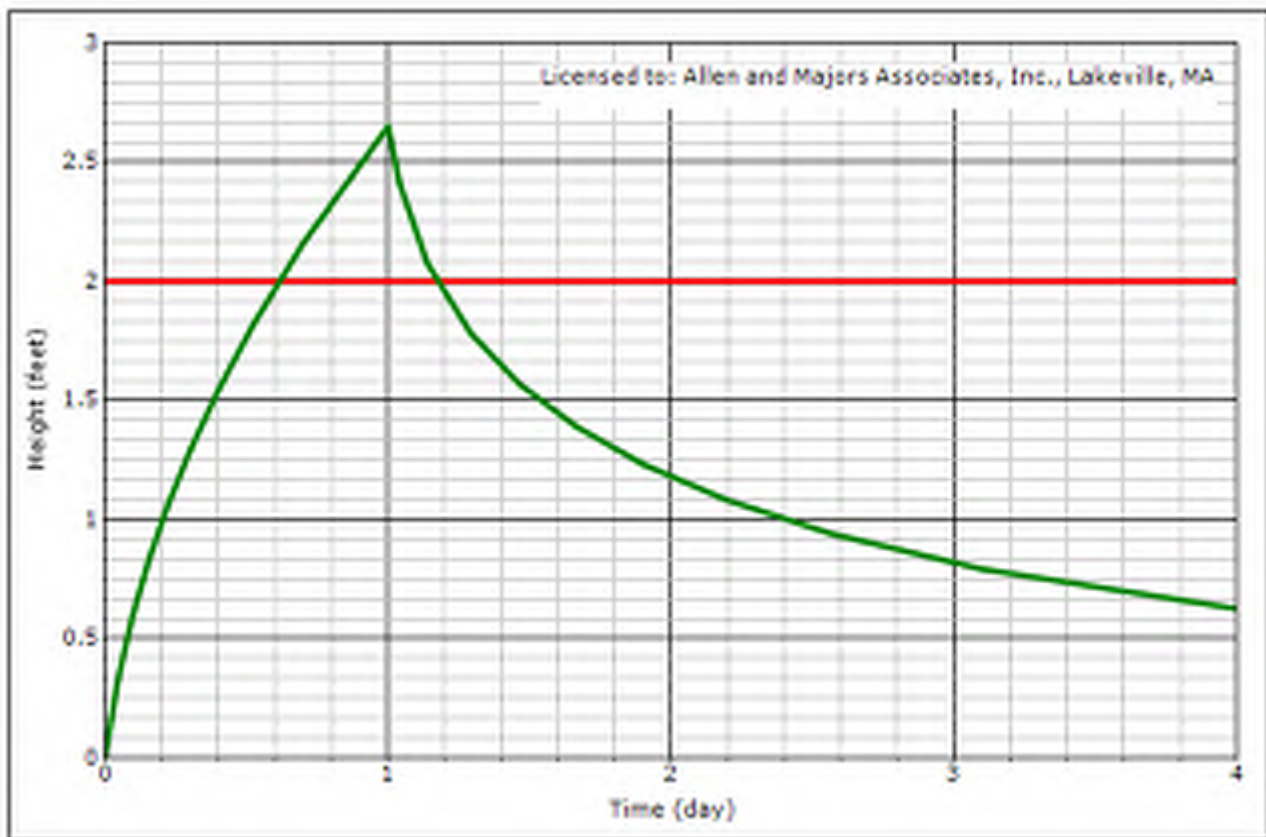
Hydraulic Conductivity (Kh): 82.7 ft/d
Drainable Porosity(Sy): 0.3
Saturated Thickness (h): 7.8 ft

Plot Geometry

X-Coordinate: 0 ft
Y-Coordinate: 0 ft
Left Side Plot Distance (Dl): 100 ft
Right Side Plot Distance (Dr): 100 ft
Plot Angle From Y-Axis (Φ): 0
Constant Head Boundary: No

<u>Distance</u> <u>(ft)</u>	<u>Height</u> <u>(ft)</u>	<u>Distance</u> <u>(ft)</u>	<u>Height</u> <u>(ft)</u>
-100.00	2.3550	0.00	2.6512
-92.78	2.4213	1.60	2.6511
-84.10	2.4834	3.15	2.6511
-76.87	2.5235	4.50	2.6509
-68.19	2.5608	5.80	2.6508
-60.96	2.5846	7.79	2.6504
-52.29	2.6064	9.69	2.6500
-46.49	2.6177	12.69	2.6491
-39.79	2.6280	15.49	2.6481
-35.21	2.6336	18.96	2.6465
-30.10	2.6388	22.18	2.6447
-26.32	2.6419	26.32	2.6419
-22.18	2.6447	30.10	2.6388
-18.96	2.6465	35.21	2.6336
-15.49	2.6481	39.79	2.6280
-12.69	2.6491	46.49	2.6177
-9.69	2.6500	52.29	2.6064
-7.79	2.6504	60.96	2.5846
-5.80	2.6508	68.19	2.5608
-4.50	2.6509	76.87	2.5235
-3.15	2.6511	84.10	2.4834
-1.60	2.6511	92.78	2.4213
		100.00	2.3550

Groundwater Mounding Analysis (Hantush Method using Glover's Solution)



Company: Allen & Major
 Project: Associates, Inc
 444 E Central St
 Franklin MA (UIS-1)
 Analyst: Carlton Quinn
 Date: 07/23/2025

Recharge Basin Dimensions

Length (w): 288.43 ft
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 Bottom Area: 8,653 ft²
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Recharge Rate Calculations

Duration (t): 1 d
 Volume (V): 21,847 ft³
 Rate (R): 2.53 ft/d
 Total Simulation Time: 4 d

Aquifer Characteristics

Hydraulic Conductivity (Kh): 82.7 ft/d
 Drainable Porosity(Sy): 0.3
 Saturated Thickness (h): 7.8 ft

Plot Geometry

X-Coordinate: 0 ft
 Y-Coordinate: 0 ft
 Left Side Plot Distance (Dl): 100 ft
 Right Side Plot Distance (Dr): 100 ft
 Plot Angle From Y-Axis (Φ): 0
 Constant Head Boundary: No

<u>Time (d)</u>	<u>Height (ft)</u>
0.00	0.0000
0.01	0.1070
0.05	0.3343
0.10	0.5991
0.15	0.8354
0.22	1.0629
0.30	1.2942
0.40	1.5408
0.52	1.8193
0.70	2.1603
1.00	2.6512
1.04	2.4135
1.14	2.0785
1.29	1.7845
1.46	1.5692
1.67	1.3908
1.90	1.2321
2.19	1.0842
2.57	0.9405
3.10	0.7935
4.00	0.6266