DRAINAGE REPORT

For

NextGrid Mescalbean, LLC

PROPOSED

"Proposed Solar Array – Parcel 1"

160 Maple Street
Bellingham, Massachusetts
Norfolk County

Prepared by:

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April 13, 2023
#W201257

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I. EXECUTIVE SUMMARY

This report examines the changes in drainage that can be expected as the result of the installation of ground mounted photovoltaic panels and associated site improvements on the easterly side of Maple Street in the Town of Franklin, Massachusetts. A small portion of the area to the northwest is located in Bellingham, Massachusetts.

The Project, as described further herein, is proposed within the northern portion of the existing Maplegate Country Club property, hereinafter referred to as "Parcel 1". The Country Club property is proposed to be subdivided into three (3) parcels. Parcels 2 and 3 will be analyzed as part of a separate Stormwater Report.

This report analyzes approximately 74± acres of land associated with the existing Country Club, including a club house, paved driveway and parking areas, paved and gravel cart paths, golf greens, driving range, landscaping, resource areas, and wooded areas. The study area is surrounded by wooded land to the north, wooded land and Mine Brook to the east, Route 495 to the south, and commercial businesses, a solar field, and Maple Street to the west.

The Project includes the construction of a new 15,000± KW solar field along with new gravel access drives, associated utilities, and storm water management components. This report addresses a comparative analysis of the pre- and post-development site runoff conditions. Additionally, this report provides calculations documenting the design of the proposed stormwater conveyance/management system as illustrated within the accompanying Site Development Plans prepared by Bohler. The project will also provide erosion and sedimentation controls during the demolition and construction periods, as well as long term stabilization of the site.

For the purposes of this analysis the pre- and post-development drainage conditions were analyzed at one (1) "design point" where stormwater runoff currently drains to under existing conditions. This design point is described in further detail in **Section II** below. A summary of the existing and proposed conditions peak runoff rates and volumes for the 2-, 10-, 25-, and 100-year storms can be found in **Tables 1.1** and **1.2** below. In addition, the project has been designed in accordance with the Massachusetts Department of Environmental Protection (MassDEP) Stormwater Management Standards and the Town of Franklin Stormwater Management Regulations, as detailed herein.

Table 1.1: Design Point Peak Runoff Rate Summary

Point of Analysis	2-Year Storm			10-Year Storm			25-Year Storm			100-Year Storm		
	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ
DP1	41.28	37.10	**-4.18**	86.72	81.04	**-5.68**	112.99	106.71	**-6.28**	153.75	146.93	**-6.82**

**Flows are represented in cubic feet per second (cfs)*

Table 1.2: Design Point Volume Summary

Point of Analysis	2-Year Storm			10-Year Storm			25-Year Storm			100-Year Storm		
	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ	Pre	Post	Δ
DP1	6.74	6.07	**-0.67**	13.61	12.64	**-0.97**	17.64	16.54	**-1.1**	23.96	22.70	**-1.26**

**Volumes are represented in acre feet (af)*

II. EXISTING SITE CONDITIONS

Existing Site Description

For the purposes of this analysis, the analyzed watershed boundaries have been established to include Parcel 1 and a portion of Parcel 2 to the south due to the existing drainage patterns. As such, the study area is approximately 74± acres.

The study area consists of land associated with the existing Maplegate Country Club located along the easterly side of Maple Street in the Town of Franklin, Massachusetts. A small portion of the area to the northwest is located in Bellingham, Massachusetts. Along with golf greens, the study area consists of a club house, paved driveways and parking areas, paved and gravel cart paths, driving range, landscaping, resource areas, and wooded areas.

On-Site Soil Information

Soils within the analyzed area consist of the following as classified by the Natural Resource Conservation Service (NRCS):

Table 2.1: Existing Soil Information

Soil Unit Symbol	Soil Name / Description	Hydrologic Soil Group (HSG)
51	Swansea muck	B/D
302B	Montauk fine sandy loam, extremely stony	C
317B	Scituate fine sandy loam, extremely stony	C

Majority of the soils at the site are mapped as Montauk and Scituate fine sandy loam, both classified by NRCS as Hydrologic Soil Group (HSG) 'C'. A portion of the soils along the eastern edge are mapped as Swansea muck and are classified as HSG 'B/D'. Refer to **Appendix C** for additional soil information.

Existing Collection and Conveyance

Runoff generated onsite drains overland to either a series of onsite water hazards / resource areas, which ultimately overflow and discharge to wetlands associated with Mine Brook, or directly to the associated wetlands. Slopes on the site range from 0.5%-50% with on-site elevations ranging from 252 in the southwest to 185 in the east.

Existing Watersheds and Design Point Information

For the purposes of this analysis, the pre- and post-development drainage conditions were analyzed at one (1) "design point" where stormwater runoff currently drains to under existing conditions. The existing site was subdivided into two (2) separate sub catchments, as described below, to analyze existing flow rates and volumes at the design point.

Sub-catchment E1a contains approximately 72.7± acres associated with Parcel 1, and consists of a club house, paved driveways and parking areas, paved and gravel cart paths, driving range, golf greens, resource areas, landscaping, and wooded areas. The associated curve number (CN) is calculated to be 75 with a time of concentration (Tc) of 45.4 minutes. Runoff from this sub-catchment flows overland to wetlands associated with Mine Brook (DP1).

Sub-catchment E1b contains approximately 1.3± acres of off-site areas associated with Parcel 2, consisting of paved cart paths, golf greens, landscaping, resource areas, and wooded areas. The associated CN is 75 with a Tc of 12.2 minutes. Runoff from this sub-catchment flows overland to wetlands associated with Mine Brook (DP1).

Refer to **Table 2.2** below for additional detail.

Table 2.2: Existing Sub-Catchment Summary

Sub-catchment Name	Total Area (acres)	Cover Description	Curve Number (CN)	Time of Concentration (Tc, minutes)
E1a	72.7±	Rooftops, pavement, gravel, grass, woods, water	75	45.4
E1b	1.3±	Pavement, grass, woods	75	12.2

Refer to **Tables 1.1** and **1.2** for the existing conditions peak rates of runoff and volumes. Refer to the Existing Conditions Drainage Map in **Appendix D** for a graphical representation of the existing drainage areas.

III. PROPOSED SITE CONDITIONS

Proposed Development Description

The Project includes the construction of a new 15,000± KW solar field along with new gravel access drives, associated utilities, and storm water management components located in the northern portion of the existing Maplegate Country Club property.

Proposed Development Collection and Conveyance

The proposed Project has been designed such that existing drainage patterns are maintained and runoff is conveyed in a manner consistent with existing conditions. Majority of the runoff generated onsite will continue to flow overland to either onsite water hazards / resource areas and overflow to wetlands associated with Mine Brook, or directly to associated wetlands. A portion of the site is proposed to flow overland to a new stormwater management basin, Basin 1 (B1) for peak rate attenuation and volume control. Proposed drainage infrastructure is limited to the proposed stormwater basin. Outlet protection sizing calculations are included in **Appendix F**.

The best management practices (BMPs) incorporated into the proposed stormwater management system have been designed to meets, or exceeds, the standards set forth in the Massachusetts Department of Environmental Protection Stormwater Handbook standards. Refer to **Section V** for additional information.

Proposed Watersheds and Design Point Information

The project has been designed to maintain existing drainage watersheds to the greatest extent possible, with the same design point described in **Section II** above. The site was subdivided into four (4) separate sub catchments for the proposed conditions, as described below, to analyze proposed flow rates and volumes at the design point.

Runoff from sub-catchments P1b and P1d flows overland following existing drainage patterns to Basin 1 for recharge/mitigation prior to discharge to DP1. Runoff from sub-catchments P1a and P1c flows overland following existing drainage patterns to DP1.

Refer to **Table 3.1** below for more information.

Table 3.1: Proposed Sub-catchment Summary

Sub-catchment Name	Total Area (acres)	Cover Description	Curve Number (CN)	Time of Concentration (Tc, minutes)	Hydrologic Routing
P1a	64.4±	Pavement, gravel, meadow, grass, woods, water	73	36.6	DP1
P1b	0.6±	Pavement, grass, woods	76	9.0	Basin 1 / DP1
P1c	0.7±	Pavement, grass, woods	75	8.4	DP1
P1d	8.3±	Meadow, gravel, basin bottom	73	15.9	Basin 1 / DP1

Refer to **Tables 1.1 and 1.2** for the calculated proposed conditions peak rates of runoff and volumes. Refer to the Proposed Conditions Drainage Map in **Appendix E** for a graphical representation of the proposed drainage areas.

IV. METHODOLOGY

Peak Flow Calculations

Methodology utilized to design the proposed stormwater management system includes compliance with the guidelines set forth in the latest edition of the Massachusetts DEP Stormwater Handbook. The pre- and post-development runoff rates being discharged from the site were computed using the HydroCAD computer program. The drainage area and outlet information were entered into the program, which routes storm flows based on NRCS TR-20 and TR-55 methods. The other components of the model were determined following standard NRCS procedures for Curve Numbers (CNs) and times of concentrations documented in the appendices of this report. The rainfall data utilized and listed below in **Table 4.1** below for stormwater calculations is based on Technical Paper-40. Refer to **Appendix F** for more information.

Table 4.1: Norfolk County Rainfall Intensities

Frequency	2 year	10 year	25 year	100 year
Rainfall* (inches)	3.20	4.70	5.50	6.70

*Values derived from Hydrology Handbook for Conservation Commissioners prepared by Mass DEP (TP-40 Maps)

The proposed stormwater management as designed will provide a decrease in peak rates and volumes of runoff from the proposed facility for the 2-, 10-, 25- and 100-year design storm events.

V. STORMWATER MANAGEMENT STANDARDS

The Project has been designed in accordance with the MassDEP Stormwater Management guidelines, Town of Franklin Stormwater Management Regulations, and Section 17-1 of the MassDEP Wetlands Program Policy for Photovoltaic System (PVS) Solar Array Review.

Standard #1: No New Untreated Discharges

There are no new untreated discharges proposed as part of the Project.

Standard #2: Peak Rate Attenuation

As outlined in **Tables 1.1** and **1.2**, the development of the site and the proposed stormwater management system have been designed so that post-development peak rates and volumes of runoff are below pre-development conditions for the 2-, 10-, 25- and 100-year storm events at all design points.

Standard #3: Recharge

The Project proposes to reduce the amount of impervious coverage across the study area by approximately 3± acres; therefore, Standard #3 is met. The post-development ground cover proposed under the PVS arrays consists of native vegetative cover, and gravel drives are proposed for access/maintenance. Stormwater runoff generated across a portion of the Project will flow overland to a proposed above ground stormwater basin for peak rate attenuation and volume control prior to discharge. The basin is expected to provide some recharge; however due to the NRCS classification of HSG 'C' soils onsite, infiltration is not used as a peak rate mitigator. Refer to **Appendix F** of this report for supporting calculations.

Onsite soil testing will be completed prior to construction to confirm soil type and depth to estimated seasonal high groundwater at the stormwater basin. Infiltration is not used as a peak rate mitigator; therefore, a ground water mounding analysis is not required.

Standard #4: Water Quality

Gravel drives are proposed for access/maintenance of the PVS system, and post-development ground cover under the proposed PVS arrays consists of native vegetative cover. Existing impervious areas to remain include a portion of the existing driveway and wetland crossings that will be repurposed and reutilized for access and maintenance purposes. Existing drainage

patterns are maintained for runoff associated with the existing driveway, which ultimately sheet flows over 200+ linear feet of vegetation prior to discharge to associated wetlands.

Due to the proposed use of the site as a solar field, the number of vehicles accessing the site and providing maintenance will be significantly reduced compared to current conditions. In addition, the Project proposes to significantly reduce the use of fertilizers and pesticides that would otherwise be used to support golf course vegetation.

Therefore, the Project proposes to improve water quality generated from the site by significantly reducing vehicular use, spreading of fertilizers and pesticides, and overall impervious coverage by approximately 3± acres. Accordingly, as per Section 17-1 of the MassDEP Wetlands Program Policy for PVS Solar Array Review, additional TSS removal above that which is anticipated to occur naturally, is not required.

Standard #5: Land Use with Higher Potential Pollutant Loads

Not Applicable for this project.

Standard #6: Critical Areas

A Zone II has been established and covers the eastern portion of the site. There are no new untreated paved surfaces discharging to the Zone II or to DP1; therefore, there are no pretreatment or structural BMPs proposed as part of the Project.

The Project proposes to improve post-development discharges from the site compared to pre-development conditions by significantly reducing vehicular use, spreading of fertilizers and pesticides, and overall impervious coverage by approximately 3± acres.

Standard #7: Redevelopment

The Project proposes to reduce the amount of impervious coverage across the study area by approximately 3± acres, and as a result proposes to improve post-development conditions by promoting recharge and improving water quality onsite. The proposed project is a redevelopment and has been designed in accordance with the Massachusetts Stormwater Management Regulations to meet the standards to the maximum extent practicable and the Town of Franklin Stormwater Management Regulations as they apply to redevelopment projects.

Standard #8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

The proposed project will provide construction period erosion and sedimentation controls as indicated within the site plan set provided for this project. This includes a proposed construction exit, protection around temporary material stock piles and various other techniques as outlined on the erosion and sediment control sheets. Additionally, the project is required to file a Notice of Intent with the US EPA and implement a Stormwater Pollution Prevention Plan (SWPPP) during the construction period. The SWPPP will be prepared prior to the start of construction and will be implemented by the site contractor under the guidance and responsibility of the project's proponent.

Standard #9: Operation and Maintenance Plan (O&M Plan)

An Operation and Maintenance (O&M) Plan for this site has been prepared and is included in **Appendix G** of this report. The O&M Plan includes a list of responsible parties and outlines procedures and time tables for the long-term operation and maintenance of the proposed site stormwater management system, including initial inspections upon completion of construction, and periodic monitoring of the system components, in accordance with established practices and the manufacturer's recommendations.

Standard #10: Prohibition of Illicit Discharges

The proposed stormwater system will only convey allowable non-stormwater discharges and will not contain any illicit discharges from prohibited sources. An Illicit Discharge Statement is included in **Appendix G** of this report.

VI. SUMMARY

In summary, the proposed stormwater management system illustrated on the drawings prepared by Bohler results in a reduction in peak rates and volumes of runoff from the study area when compared to pre-development conditions for the 2-, 10-, 25- and 100-year storm frequencies. The proposed stormwater improvements have been designed such that runoff is conveyed in a manner consistent with existing conditions and directed to locations to which runoff is currently flowing. The pre-development versus post-development stormwater discharge comparisons are contained in **Tables 1.1** and **1.2**.

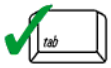
APPENDIX A: MASSACHUSETTS STORMWATER MANAGEMENT CHECKLIST

Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.[1] This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8[2]
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

[1] The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

[2] For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.

Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

Checklist for Stormwater Report

B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

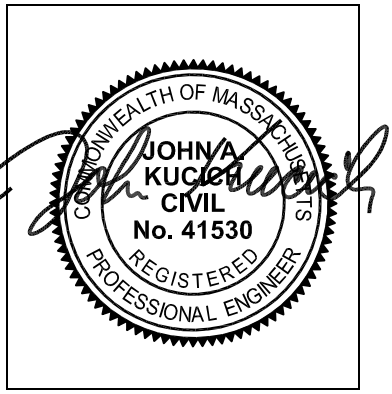
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature



4/13/23

Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

☐ New development

☒ Redevelopment

☐ Mix of New Development and Redevelopment

Checklist for Stormwater Report

Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

☒ No disturbance to any Wetland Resource Areas

☐ Site Design Practices (e.g. clustered development, reduced frontage setbacks)

☒ Reduced Impervious Area (Redevelopment Only)

☐ Minimizing disturbance to existing trees and shrubs

☐ LID Site Design Credit Requested:

- ☐ Credit 1
- ☐ Credit 2
- ☐ Credit 3

☐ Use of "country drainage" versus curb and gutter conveyance and pipe

☐ Bioretention Cells (includes Rain Gardens)

☐ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)

☐ Treebox Filter

☐ Water Quality Swale

☐ Grass Channel

☐ Green Roof

☒ Other (describe): Surface Stormwater Management Basin

Standard 1: No New Untreated Discharges

☒ No new untreated discharges

☒ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth

☒ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.

Checklist for Stormwater Report

Checklist (continued)

Standard 2: Peak Rate Attenuation

☐ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.

☐ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.

☒ Calculations provided to show that post-development peak discharge rates do not exceed pre-development rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24-hour storm.

Standard 3: Recharge

☐ Soil Analysis provided.

☒ Required Recharge Volume calculation provided.

☐ Required Recharge volume reduced through use of the LID site Design Credits.

☐ Sizing the infiltration, BMPs is based on the following method: Check the method used.

☐ Static ☐ Simple Dynamic ☐ Dynamic Field[1]

☐ Runoff from all impervious areas at the site discharging to the infiltration BMP.

☐ Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.

☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.

☐ Recharge BMPs have been sized to infiltrate the Required Recharge Volume *only* to the maximum extent practicable for the following reason:

☐ Site is comprised solely of C and D soils and/or bedrock at the land surface

☐ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000

☐ Solid Waste Landfill pursuant to 310 CMR 19.000

☐ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.

☐ Calculations showing that the infiltration BMPs will drain in 72 hours are provided.

☐ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

[1] 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.

Checklist for Stormwater Report

Checklist (continued)

Standard 3: Recharge (continued)

☐ The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10-year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.

☐ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.

☒ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.

☐ Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:

- ☐ is within the Zone II or Interim Wellhead Protection Area
- ☐ is near or to other critical areas
- ☐ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
- ☐ involves runoff from land uses with higher potential pollutant loads.

☐ The Required Water Quality Volume is reduced through use of the LID site Design Credits.

☐ Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.

Checklist for Stormwater Report

Checklist (continued)

Standard 4: Water Quality (continued)

☐ The BMP is sized (and calculations provided) based on:

- ☐ The ½" or 1" Water Quality Volume or
- ☐ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.

☐ The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.

☐ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

☐ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.

☐ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted ***prior to*** the discharge of stormwater to the post-construction stormwater BMPs.

☐ The NPDES Multi-Sector General Permit does ***not*** cover the land use.

☐ LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.

☐ All exposure has been eliminated.

☐ All exposure has ***not*** been eliminated and all BMPs selected are on MassDEP LUHPPL list.

☐ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

☐ The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.

☒ Critical areas and BMPs are identified in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

☒ The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

- ☐ Limited Project
- ☐ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- ☐ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- ☐ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- ☐ Bike Path and/or Foot Path
- ☒ Redevelopment Project
- ☐ Redevelopment portion of mix of new and redevelopment.

☐ Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

☒ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

☒ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.

Checklist for Stormwater Report

Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has ***not*** been included in the Stormwater Report but will be submitted ***before*** land disturbance begins.

☐ The project is ***not*** covered by a NPDES Construction General Permit.

☐ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.

☒ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

☒ The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:

- ☒ Name of the stormwater management system owners;
- ☒ Party responsible for operation and maintenance;
- ☒ Schedule for implementation of routine and non-routine maintenance tasks;
- ☐ Plan showing the location of all stormwater BMPs maintenance access areas;
- ☐ Description and delineation of public safety features;
- ☐ Estimated operation and maintenance budget; and
- ☒ Operation and Maintenance Log Form.

☐ The responsible party is ***not*** the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:

- ☐ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
- ☐ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

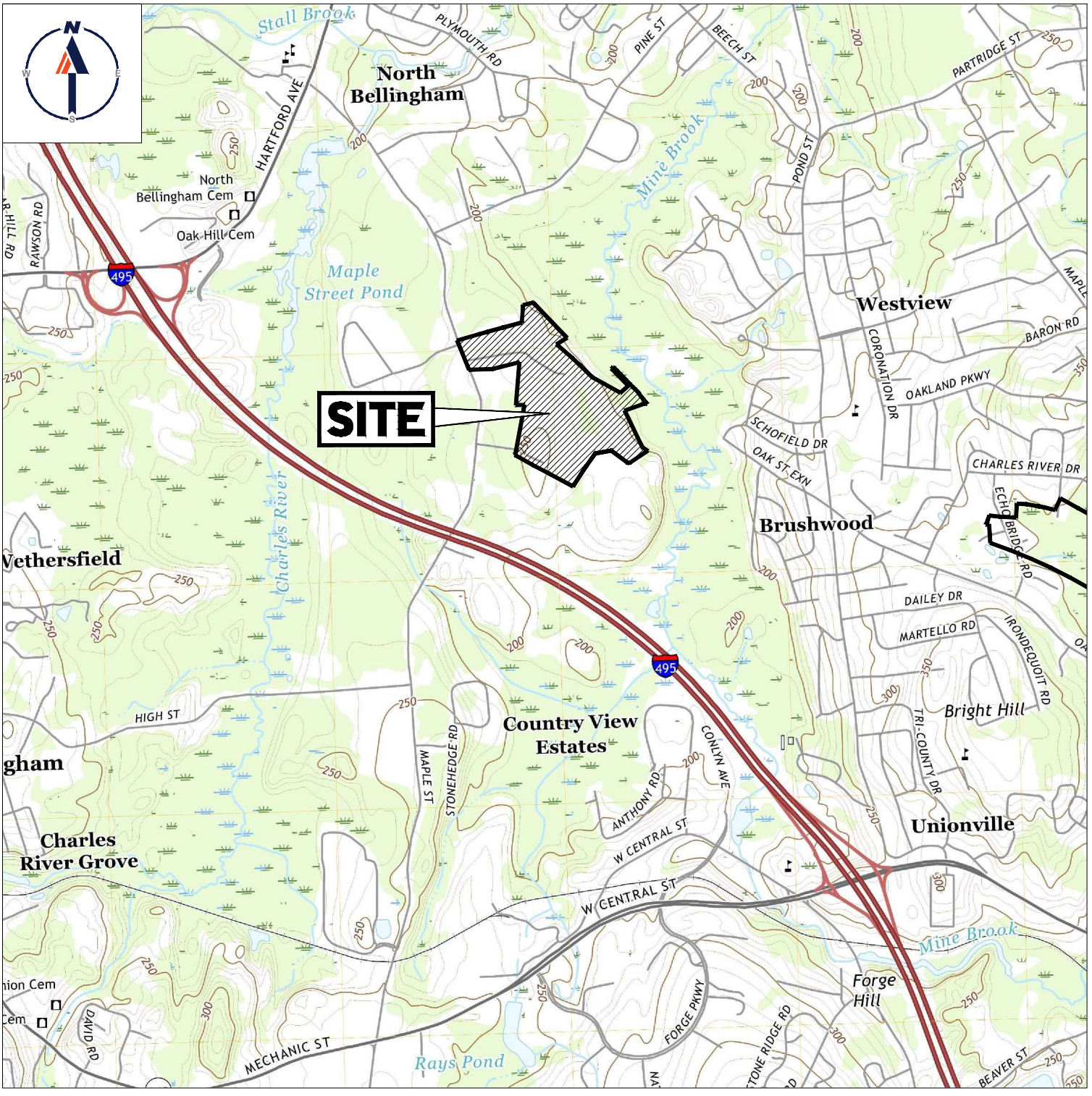
☒ The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;

☒ An Illicit Discharge Compliance Statement is attached;

☐ NO Illicit Discharge Compliance Statement is attached but will be submitted ***prior to*** the discharge of any stormwater to post-construction BMPs.

APPENDIX B: PROJECT LOCATION MAPS

- *USGS MAP*
- *FEMA FIRMETTE*



USGS MAP

SCALE: 1" = 2,000'
SOURCE: FRANKLIN MASSACHUSETTS USGS QUADRANGLE

National Flood Hazard Layer FIRMette

FEMA

71°26'52"W 42°6'50"N

71°26'15"W 42°6'23"N

0 250 500 1,000 1,500 2,000 Feet

1:6,000

Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) *Zone A, V, A99*
- With BFE or Depth *Zone AE, AO, AH, VE, AR*
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
- Future Conditions 1% Annual Chance Flood Hazard *Zone X*
- Area with Reduced Flood Risk due to Levee. See Notes. *Zone X*
- Area with Flood Risk due to Levee *Zone D*

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard *Zone X*
- Effective LOMRs
- Area of Undetermined Flood Hazard *Zone D*

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- 20.2 Cross Sections with 1% Annual Chance
- 17.5 Water Surface Elevation
- Coastal Transect
- 513 Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS
- Digital Data Available
- No Digital Data Available
- Unmapped

N

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

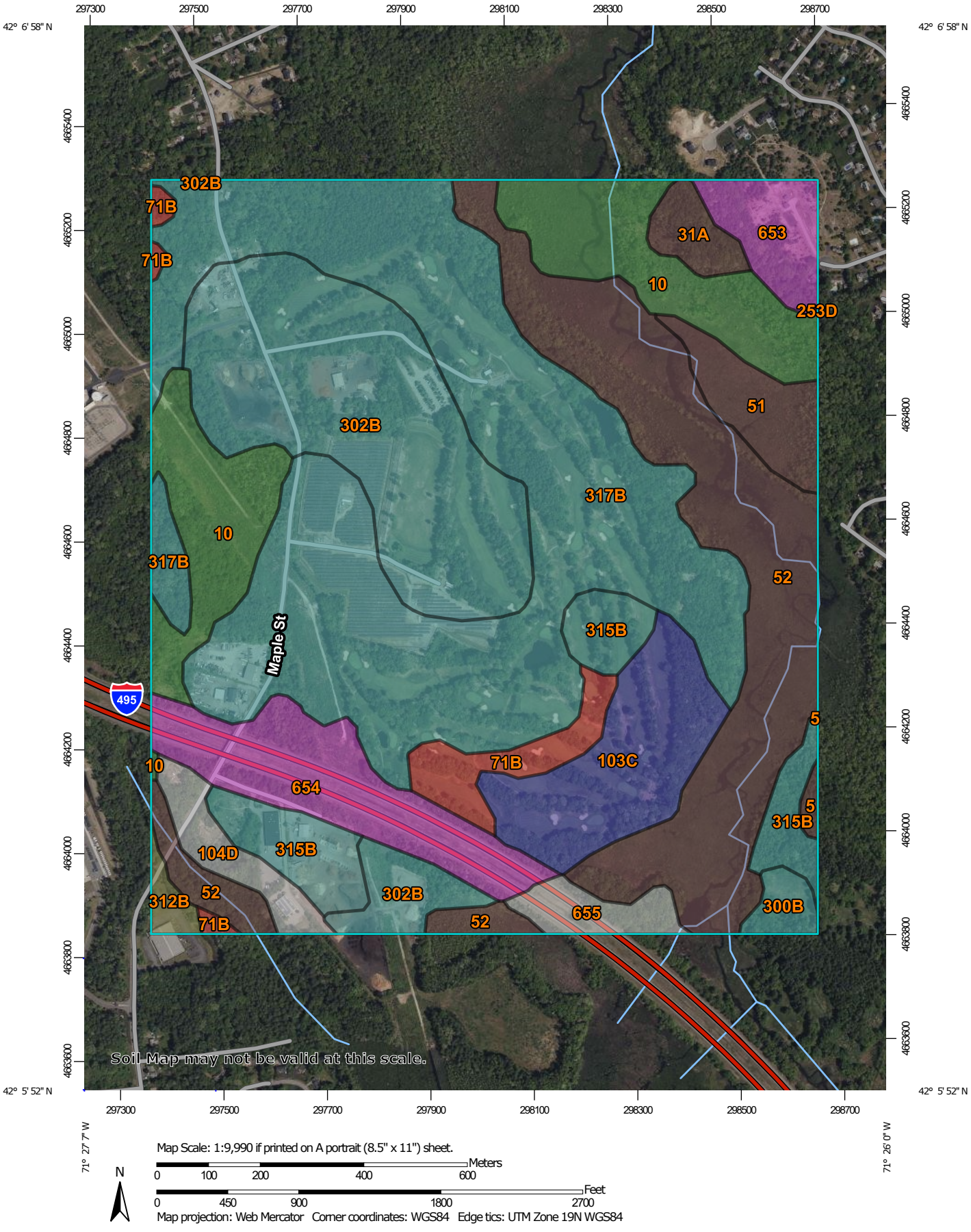
The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 3/8/2022 at 1:51 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

APPENDIX C: SOIL AND WETLAND INFORMATION

- *NCRS CUSTOM SOIL RESOURCE REPORT*

Hydrologic Soil Group—Norfolk and Suffolk Counties, Massachusetts
(Franklin, MA)



Map Scale: 1:9,990 if printed on A portrait (8.5" x 11") sheet.

N

Meters
0 100 200 400 600

Feet
0 450 900 1800 2700

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 19N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

Soil Rating Polygons

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Norfolk and Suffolk Counties, Massachusetts
Survey Area Data: Version 18, Sep 9, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 22, 2022—Jun 5, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
5	Saco silt loam, frequently ponded, 0 to 1 percent slopes, frequently flooded	B/D	1.0	0.2%
10	Scarboro and Birdsall soils, 0 to 3 percent slopes	A/D	44.4	9.6%
31A	Walpole sandy loam, 0 to 3 percent slopes	B/D	5.3	1.1%
51	Swansea muck, 0 to 1 percent slopes	B/D	10.6	2.3%
52	Freetown muck, 0 to 1 percent slopes	B/D	75.6	16.3%
71B	Ridgebury fine sandy loam, 3 to 8 percent slopes, extremely stony	D	10.8	2.3%
103C	Charlton-Hollis-Rock outcrop complex, 8 to 15 percent slopes	B	26.4	5.7%
104D	Hollis-Rock outcrop-Charlton complex, 15 to 35 percent slopes		8.0	1.7%
253D	Hinckley loamy sand, 15 to 35 percent slopes	A	0.0	0.0%
300B	Montauk fine sandy loam, 3 to 8 percent slopes	C	3.5	0.7%
302B	Montauk fine sandy loam, 0 to 8 percent slopes, extremely stony	C	70.6	15.2%
312B	Woodbridge fine sandy loam, 0 to 8 percent slopes, extremely stony	C/D	2.4	0.5%
315B	Scituate fine sandy loam, 3 to 8 percent slopes	C	20.5	4.4%
317B	Scituate fine sandy loam, 3 to 8 percent slopes, extremely stony	C	142.7	30.7%
653	Udorthents, sandy	A	9.9	2.1%
654	Udorthents, loamy	A	26.4	5.7%

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
655	Udorthents, wet substratum		6.2	1.3%
Totals for Area of Interest			**464.2**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

APPENDIX D: EXISTING CONDITIONS HYDROLOGIC ANALYSIS

- *EXISTING CONDITIONS DRAINAGE MAP*
- *EXISTING CONDITIONS HYDROCAD COMPUTATIONS*

LEGEND

- DP# — DESIGN POINT
- E# — EXISTING SUBCATCHMENT
- A/B/C/D — NRCS HYDROLOGIC SOIL GROUP RATING
- OVERALL ANALYSIS BOUNDARY
- SUBCATCHMENT BOUNDARY
- NRCS HYDROLOGIC SOIL GROUP BOUNDARY
- TIME OF CONCENTRATION

BOHLER

SITE CIVIL AND CONSULTING ENGINEERING
LAND SURVEYING
PROGRAM MANAGEMENT
LANDSCAPE ARCHITECTURE
SUSTAINABLE DESIGN
PERMITTING SERVICES
TRANSPORTATION SERVICES

REVISIONS

REV	DATE	COMMENT	DRAWN BY / CHECKED BY

811 Know what's below. Call before you dig.
ALWAYS CALL 811
It's fast. It's free. It's the law.

PRELIMINARY

THIS DRAWING IS INTENDED FOR MUNICIPAL AND/OR AGENCY REVIEW AND APPROVAL. IT IS NOT INTENDED AS A CONSTRUCTION DOCUMENT UNLESS INDICATED OTHERWISE.

PROJECT No.:	W201257
DRAWN BY:	EVD / CMC / OCR
CHECKED BY:	GD
DATE:	04/13/2023
CAD I.D.:	W201257_EX

PROJECT:

PROPOSED SITE PLAN DOCUMENTS

FOR

NEXTGRID MESCALBEAN LLC

PROPOSED SOLAR FARM

LOTS INCLUDED (PARCEL #):239-009, F 239-010, G 239-010, H 239-010, I 239-010, 239-012, 237-36-37, 237-36, E 239-010, PORTION OF D 239-010160 MAPLE STREET,
TOWN OF BELLINGHAM & FRANKLIN, NORFOLK COUNTY, MASSACHUSETTS

BOHLER
352 TURNPIKE ROAD
SOUTHBOROUGH, MA 01772
Phone: (508) 480-9900
www.BohlerEngineering.com

SHEET TITLE:

EXISTING CONDITIONS DRAINAGE AREA MAP

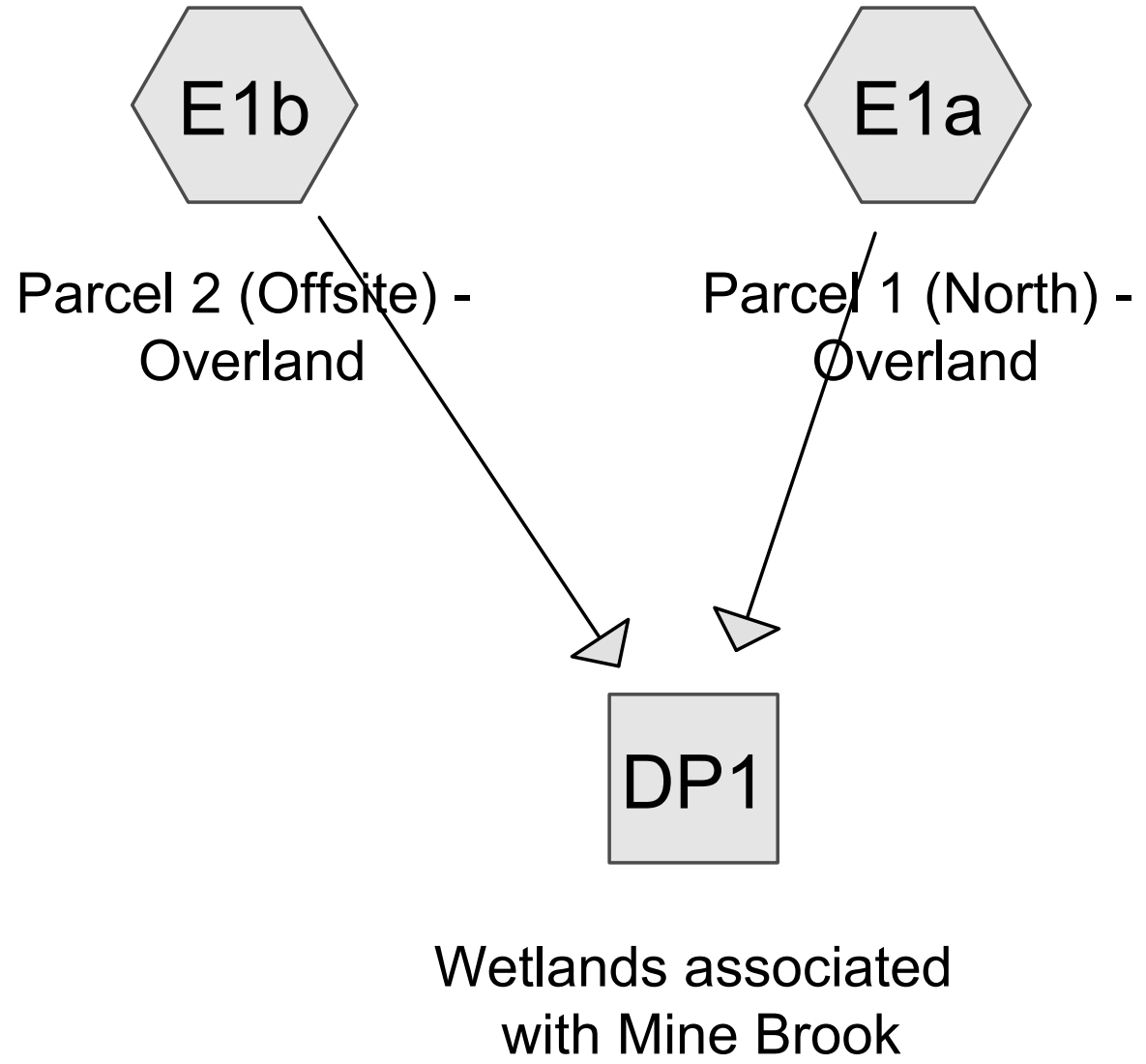
SHEET NUMBER:

EXDAM

ORG. DATE - 04/13/2023

CC

P:\2021\W201257\TECHNICAL\STORMWATER\W201257_EX_---_LAYOUT_EXDAM EXIST. WATERSHED-24X36



Subcat Reach Pond Link

Routing Diagram for W201257-EX-North
Prepared by Bohler, Printed 3/30/2023

Area Listing (selected nodes)

Area (acres)	CN	Description (subcatchment-numbers)
43.717	74	>75% Grass cover, Good, HSG C (E1a, E1b)
2.029	89	Gravel roads, HSG C (E1a)
3.088	98	Paved parking, HSG C (E1a, E1b)
0.142	98	Roofs, HSG C (E1a)
1.753	98	Water Surface, HSG C (E1a)
21.405	70	Woods, Good, HSG C (E1a, E1b)
1.847	77	Woods, Good, HSG D (E1a)
73.981	**75**	**TOTAL AREA**

Soil Listing (selected nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
72.134	HSG C	E1a, E1b
1.847	HSG D	E1a
0.000	Other	
73.981		**TOTAL AREA**

Ground Covers (selected nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	43.717	0.000	0.000	43.717	>75% Grass cover, Good	E1a, E1b
0.000	0.000	2.029	0.000	0.000	2.029	Gravel roads	E1a
0.000	0.000	3.088	0.000	0.000	3.088	Paved parking	E1a, E1b
0.000	0.000	0.142	0.000	0.000	0.142	Roofs	E1a
0.000	0.000	1.753	0.000	0.000	1.753	Water Surface	E1a
0.000	0.000	21.405	1.847	0.000	23.252	Woods, Good	E1a, E1b
0.000	**0.000**	**72.134**	**1.847**	**0.000**	**73.981**	**TOTAL AREA**	

Time span=1.00-48.00 hrs, dt=0.05 hrs, 941 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1a: Parcel 1 (North) - Runoff Area=72.718 ac 6.71% Impervious Runoff Depth=1.09"
Flow Length=1,822' Tc=45.4 min CN=75 Runoff=40.94 cfs 6.629 af

Subcatchment E1b: Parcel 2 (Offsite) - Runoff Area=1.263 ac 7.92% Impervious Runoff Depth=1.09"
Flow Length=676' Tc=12.2 min CN=75 Runoff=1.24 cfs 0.115 af

Reach DP1: Wetlands associated with Mine Brook Inflow=41.28 cfs 6.744 af
Outflow=41.28 cfs 6.744 af

Total Runoff Area = 73.981 ac Runoff Volume = 6.744 af Average Runoff Depth = 1.09"
93.26% Pervious = 68.998 ac 6.74% Impervious = 4.983 ac

Summary for Subcatchment E1a: Parcel 1 (North) - Overland

Runoff = 40.94 cfs @ 12.67 hrs, Volume= 6.629 af, Depth= 1.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-yr Rainfall=3.20"

Area (ac)	CN	Description
0.142	98	Roofs, HSG C
2.988	98	Paved parking, HSG C
2.029	89	Gravel roads, HSG C
42.700	74	>75% Grass cover, Good, HSG C
21.259	70	Woods, Good, HSG C
1.753	98	Water Surface, HSG C
1.847	77	Woods, Good, HSG D
72.718	75	Weighted Average
67.835		93.29% Pervious Area
4.883		6.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0	50	0.0060	0.04		**Sheet Flow, 249.7-249.4 (A-B)** Woods: Light underbrush n= 0.400 P2= 3.20"
2.1	44	0.0050	0.35		**Shallow Concentrated Flow, 249.4-249.2 (B-C)** Woodland Kv= 5.0 fps
20.2	1,314	0.0240	1.08		**Shallow Concentrated Flow, 249.2-218 (C-D)** Short Grass Pasture Kv= 7.0 fps
0.5	111	0.0360	3.85		**Shallow Concentrated Flow, 218-214 (D-E)** Paved Kv= 20.3 fps
1.7	211	0.0900	2.10		**Shallow Concentrated Flow, 214-195 (E-F)** Short Grass Pasture Kv= 7.0 fps
0.9	92	0.1090	1.65		**Shallow Concentrated Flow, 195-185 (F-G)** Woodland Kv= 5.0 fps
45.4	1,822	Total			

Summary for Subcatchment E1b: Parcel 2 (Offsite) - Overland

Runoff = 1.24 cfs @ 12.18 hrs, Volume= 0.115 af, Depth= 1.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-yr Rainfall=3.20"

Area (ac)	CN	Description
0.146	70	Woods, Good, HSG C
0.100	98	Paved parking, HSG C
1.017	74	>75% Grass cover, Good, HSG C
1.263	75	Weighted Average
1.163		92.08% Pervious Area
0.100		7.92% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.0300	0.17		**Sheet Flow, 230-228.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
2.1	153	0.0290	1.19		**Shallow Concentrated Flow, 228.5-224 (B-C)** Short Grass Pasture Kv= 7.0 fps
3.2	227	0.0570	1.19		**Shallow Concentrated Flow, 224-211 (C-D)** Woodland Kv= 5.0 fps
1.4	166	0.0780	1.95		**Shallow Concentrated Flow, 211-198 (D-E)** Short Grass Pasture Kv= 7.0 fps
0.7	80	0.1440	1.90		**Shallow Concentrated Flow, 198-186.5 (E-F)** Woodland Kv= 5.0 fps
12.2	676	Total			

Summary for Reach DP1: Wetlands associated with Mine Brook

Inflow Area = 73.981 ac, 6.74% Impervious, Inflow Depth = 1.09" for 2-yr event
Inflow = 41.28 cfs @ 12.66 hrs, Volume= 6.744 af
Outflow = 41.28 cfs @ 12.66 hrs, Volume= 6.744 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs

Time span=1.00-48.00 hrs, dt=0.05 hrs, 941 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1a: Parcel 1 (North) - Runoff Area=72.718 ac 6.71% Impervious Runoff Depth=2.21"
Flow Length=1,822' Tc=45.4 min CN=75 Runoff=86.02 cfs 13.382 af

Subcatchment E1b: Parcel 2 (Offsite) - Runoff Area=1.263 ac 7.92% Impervious Runoff Depth=2.21"
Flow Length=676' Tc=12.2 min CN=75 Runoff=2.63 cfs 0.232 af

Reach DP1: Wetlands associated with Mine Brook Inflow=86.72 cfs 13.614 af
Outflow=86.72 cfs 13.614 af

Total Runoff Area = 73.981 ac Runoff Volume = 13.614 af Average Runoff Depth = 2.21"
93.26% Pervious = 68.998 ac 6.74% Impervious = 4.983 ac

Summary for Subcatchment E1a: Parcel 1 (North) - Overland

Runoff = 86.02 cfs @ 12.64 hrs, Volume= 13.382 af, Depth= 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-yr Rainfall=4.70"

Area (ac)	CN	Description
0.142	98	Roofs, HSG C
2.988	98	Paved parking, HSG C
2.029	89	Gravel roads, HSG C
42.700	74	>75% Grass cover, Good, HSG C
21.259	70	Woods, Good, HSG C
1.753	98	Water Surface, HSG C
1.847	77	Woods, Good, HSG D
72.718	75	Weighted Average
67.835		93.29% Pervious Area
4.883		6.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0	50	0.0060	0.04		**Sheet Flow, 249.7-249.4 (A-B)** Woods: Light underbrush n= 0.400 P2= 3.20"
2.1	44	0.0050	0.35		**Shallow Concentrated Flow, 249.4-249.2 (B-C)** Woodland Kv= 5.0 fps
20.2	1,314	0.0240	1.08		**Shallow Concentrated Flow, 249.2-218 (C-D)** Short Grass Pasture Kv= 7.0 fps
0.5	111	0.0360	3.85		**Shallow Concentrated Flow, 218-214 (D-E)** Paved Kv= 20.3 fps
1.7	211	0.0900	2.10		**Shallow Concentrated Flow, 214-195 (E-F)** Short Grass Pasture Kv= 7.0 fps
0.9	92	0.1090	1.65		**Shallow Concentrated Flow, 195-185 (F-G)** Woodland Kv= 5.0 fps
45.4	1,822	Total			

Summary for Subcatchment E1b: Parcel 2 (Offsite) - Overland

Runoff = 2.63 cfs @ 12.17 hrs, Volume= 0.232 af, Depth= 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-yr Rainfall=4.70"

Area (ac)	CN	Description
0.146	70	Woods, Good, HSG C
0.100	98	Paved parking, HSG C
1.017	74	>75% Grass cover, Good, HSG C
1.263	75	Weighted Average
1.163		92.08% Pervious Area
0.100		7.92% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.0300	0.17		**Sheet Flow, 230-228.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
2.1	153	0.0290	1.19		**Shallow Concentrated Flow, 228.5-224 (B-C)** Short Grass Pasture Kv= 7.0 fps
3.2	227	0.0570	1.19		**Shallow Concentrated Flow, 224-211 (C-D)** Woodland Kv= 5.0 fps
1.4	166	0.0780	1.95		**Shallow Concentrated Flow, 211-198 (D-E)** Short Grass Pasture Kv= 7.0 fps
0.7	80	0.1440	1.90		**Shallow Concentrated Flow, 198-186.5 (E-F)** Woodland Kv= 5.0 fps
12.2	676	Total			

Summary for Reach DP1: Wetlands associated with Mine Brook

Inflow Area = 73.981 ac, 6.74% Impervious, Inflow Depth = 2.21" for 10-yr event
Inflow = 86.72 cfs @ 12.64 hrs, Volume= 13.614 af
Outflow = 86.72 cfs @ 12.64 hrs, Volume= 13.614 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs

Time span=1.00-48.00 hrs, dt=0.05 hrs, 941 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1a: Parcel 1 (North) - Runoff Area=72.718 ac 6.71% Impervious Runoff Depth=2.86"
Flow Length=1,822' Tc=45.4 min CN=75 Runoff=112.08 cfs 17.334 af

Subcatchment E1b: Parcel 2 (Offsite) - Runoff Area=1.263 ac 7.92% Impervious Runoff Depth=2.86"
Flow Length=676' Tc=12.2 min CN=75 Runoff=3.43 cfs 0.301 af

Reach DP1: Wetlands associated with Mine Brook Inflow=112.99 cfs 17.635 af
Outflow=112.99 cfs 17.635 af

Total Runoff Area = 73.981 ac Runoff Volume = 17.635 af Average Runoff Depth = 2.86"
93.26% Pervious = 68.998 ac 6.74% Impervious = 4.983 ac

Summary for Subcatchment E1a: Parcel 1 (North) - Overland

Runoff = 112.08 cfs @ 12.63 hrs, Volume= 17.334 af, Depth= 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-yr Rainfall=5.50"

Area (ac)	CN	Description
0.142	98	Roofs, HSG C
2.988	98	Paved parking, HSG C
2.029	89	Gravel roads, HSG C
42.700	74	>75% Grass cover, Good, HSG C
21.259	70	Woods, Good, HSG C
1.753	98	Water Surface, HSG C
1.847	77	Woods, Good, HSG D
72.718	75	Weighted Average
67.835		93.29% Pervious Area
4.883		6.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0	50	0.0060	0.04		**Sheet Flow, 249.7-249.4 (A-B)** Woods: Light underbrush n= 0.400 P2= 3.20"
2.1	44	0.0050	0.35		**Shallow Concentrated Flow, 249.4-249.2 (B-C)** Woodland Kv= 5.0 fps
20.2	1,314	0.0240	1.08		**Shallow Concentrated Flow, 249.2-218 (C-D)** Short Grass Pasture Kv= 7.0 fps
0.5	111	0.0360	3.85		**Shallow Concentrated Flow, 218-214 (D-E)** Paved Kv= 20.3 fps
1.7	211	0.0900	2.10		**Shallow Concentrated Flow, 214-195 (E-F)** Short Grass Pasture Kv= 7.0 fps
0.9	92	0.1090	1.65		**Shallow Concentrated Flow, 195-185 (F-G)** Woodland Kv= 5.0 fps
45.4	1,822	Total			

Summary for Subcatchment E1b: Parcel 2 (Offsite) - Overland

Runoff = 3.43 cfs @ 12.17 hrs, Volume= 0.301 af, Depth= 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-yr Rainfall=5.50"

Area (ac)	CN	Description
0.146	70	Woods, Good, HSG C
0.100	98	Paved parking, HSG C
1.017	74	>75% Grass cover, Good, HSG C
1.263	75	Weighted Average
1.163		92.08% Pervious Area
0.100		7.92% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.0300	0.17		**Sheet Flow, 230-228.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
2.1	153	0.0290	1.19		**Shallow Concentrated Flow, 228.5-224 (B-C)** Short Grass Pasture Kv= 7.0 fps
3.2	227	0.0570	1.19		**Shallow Concentrated Flow, 224-211 (C-D)** Woodland Kv= 5.0 fps
1.4	166	0.0780	1.95		**Shallow Concentrated Flow, 211-198 (D-E)** Short Grass Pasture Kv= 7.0 fps
0.7	80	0.1440	1.90		**Shallow Concentrated Flow, 198-186.5 (E-F)** Woodland Kv= 5.0 fps
12.2	676	Total			

Summary for Reach DP1: Wetlands associated with Mine Brook

Inflow Area = 73.981 ac, 6.74% Impervious, Inflow Depth = 2.86" for 25-yr event
Inflow = 112.99 cfs @ 12.63 hrs, Volume= 17.635 af
Outflow = 112.99 cfs @ 12.63 hrs, Volume= 17.635 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs

Time span=1.00-48.00 hrs, dt=0.05 hrs, 941 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment E1a: Parcel 1 (North) - Runoff Area=72.718 ac 6.71% Impervious Runoff Depth=3.89"
Flow Length=1,822' Tc=45.4 min CN=75 Runoff=152.57 cfs 23.550 af

Subcatchment E1b: Parcel 2 (Offsite) - Runoff Area=1.263 ac 7.92% Impervious Runoff Depth=3.89"
Flow Length=676' Tc=12.2 min CN=75 Runoff=4.67 cfs 0.409 af

Reach DP1: Wetlands associated with Mine Brook Inflow=153.75 cfs 23.959 af
Outflow=153.75 cfs 23.959 af

Total Runoff Area = 73.981 ac Runoff Volume = 23.959 af Average Runoff Depth = 3.89"
93.26% Pervious = 68.998 ac 6.74% Impervious = 4.983 ac

Summary for Subcatchment E1a: Parcel 1 (North) - Overland

Runoff = 152.57 cfs @ 12.63 hrs, Volume= 23.550 af, Depth= 3.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-yr Rainfall=6.70"

Area (ac)	CN	Description
0.142	98	Roofs, HSG C
2.988	98	Paved parking, HSG C
2.029	89	Gravel roads, HSG C
42.700	74	>75% Grass cover, Good, HSG C
21.259	70	Woods, Good, HSG C
1.753	98	Water Surface, HSG C
1.847	77	Woods, Good, HSG D
72.718	75	Weighted Average
67.835		93.29% Pervious Area
4.883		6.71% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
20.0	50	0.0060	0.04		**Sheet Flow, 249.7-249.4 (A-B)** Woods: Light underbrush n= 0.400 P2= 3.20"
2.1	44	0.0050	0.35		**Shallow Concentrated Flow, 249.4-249.2 (B-C)** Woodland Kv= 5.0 fps
20.2	1,314	0.0240	1.08		**Shallow Concentrated Flow, 249.2-218 (C-D)** Short Grass Pasture Kv= 7.0 fps
0.5	111	0.0360	3.85		**Shallow Concentrated Flow, 218-214 (D-E)** Paved Kv= 20.3 fps
1.7	211	0.0900	2.10		**Shallow Concentrated Flow, 214-195 (E-F)** Short Grass Pasture Kv= 7.0 fps
0.9	92	0.1090	1.65		**Shallow Concentrated Flow, 195-185 (F-G)** Woodland Kv= 5.0 fps
45.4	1,822	Total			

Summary for Subcatchment E1b: Parcel 2 (Offsite) - Overland

Runoff = 4.67 cfs @ 12.17 hrs, Volume= 0.409 af, Depth= 3.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-yr Rainfall=6.70"

Area (ac)	CN	Description
0.146	70	Woods, Good, HSG C
0.100	98	Paved parking, HSG C
1.017	74	>75% Grass cover, Good, HSG C
1.263	75	Weighted Average
1.163		92.08% Pervious Area
0.100		7.92% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.0300	0.17		**Sheet Flow, 230-228.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
2.1	153	0.0290	1.19		**Shallow Concentrated Flow, 228.5-224 (B-C)** Short Grass Pasture Kv= 7.0 fps
3.2	227	0.0570	1.19		**Shallow Concentrated Flow, 224-211 (C-D)** Woodland Kv= 5.0 fps
1.4	166	0.0780	1.95		**Shallow Concentrated Flow, 211-198 (D-E)** Short Grass Pasture Kv= 7.0 fps
0.7	80	0.1440	1.90		**Shallow Concentrated Flow, 198-186.5 (E-F)** Woodland Kv= 5.0 fps
12.2	676	Total			

Summary for Reach DP1: Wetlands associated with Mine Brook

Inflow Area = 73.981 ac, 6.74% Impervious, Inflow Depth = 3.89" for 100-yr event
Inflow = 153.75 cfs @ 12.62 hrs, Volume= 23.959 af
Outflow = 153.75 cfs @ 12.62 hrs, Volume= 23.959 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs

APPENDIX E: PROPOSED CONDITIONS HYDROLOGIC ANALYSIS

- *PROPOSED CONDITIONS DRAINAGE MAP*
- *PROPOSED CONDITIONS HYDROCAD CALCULATIONS*

200 100 50 0 200

1"= 200'

LEGEND

- DP# — DESIGN POINT
- P# — PROPOSED SUBCATCHMENT
- B# — PROPOSED STORMWATER MANAGEMENT POND
- A/B/C/D — NRCS HYDROLOGIC SOIL GROUP RATING
- OVERALL ANALYSIS BOUNDARY
- SUBCATCHMENT BOUNDARY
- NRCS HYDROLOGIC SOIL GROUP BOUNDARY
- TIME OF CONCENTRATION

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PROJECT No.:	W201257
DRAWN BY:	EVD / CMC / OCR
CHECKED BY:	GD
DATE:	04/13/2023
CAD I.D.:	W201257_PR

PROJECT:

PROPOSED SITE PLAN DOCUMENTS

FOR

NEXTGRID
MESCALBEAN LLC

PROPOSED SOLAR FARM

LOTS INCLUDED (PARCEL #):239-009, F 239-010, G 239-010, H 239-010, I 239-010, 239-012, 237-36-37, 237-36, E 239-010, PORTION OF D 239-010160 MAPLE STREET,
TOWN OF BELLINGHAM & FRANKLIN, NORFOLK COUNTY, MASSACHUSETTS

BOHLER

352 TURNPIKE ROAD
SOUTHBOROUGH, MA 01772
Phone: (508) 480-9900

www.BohlerEngineering.com

SHEET TITLE:

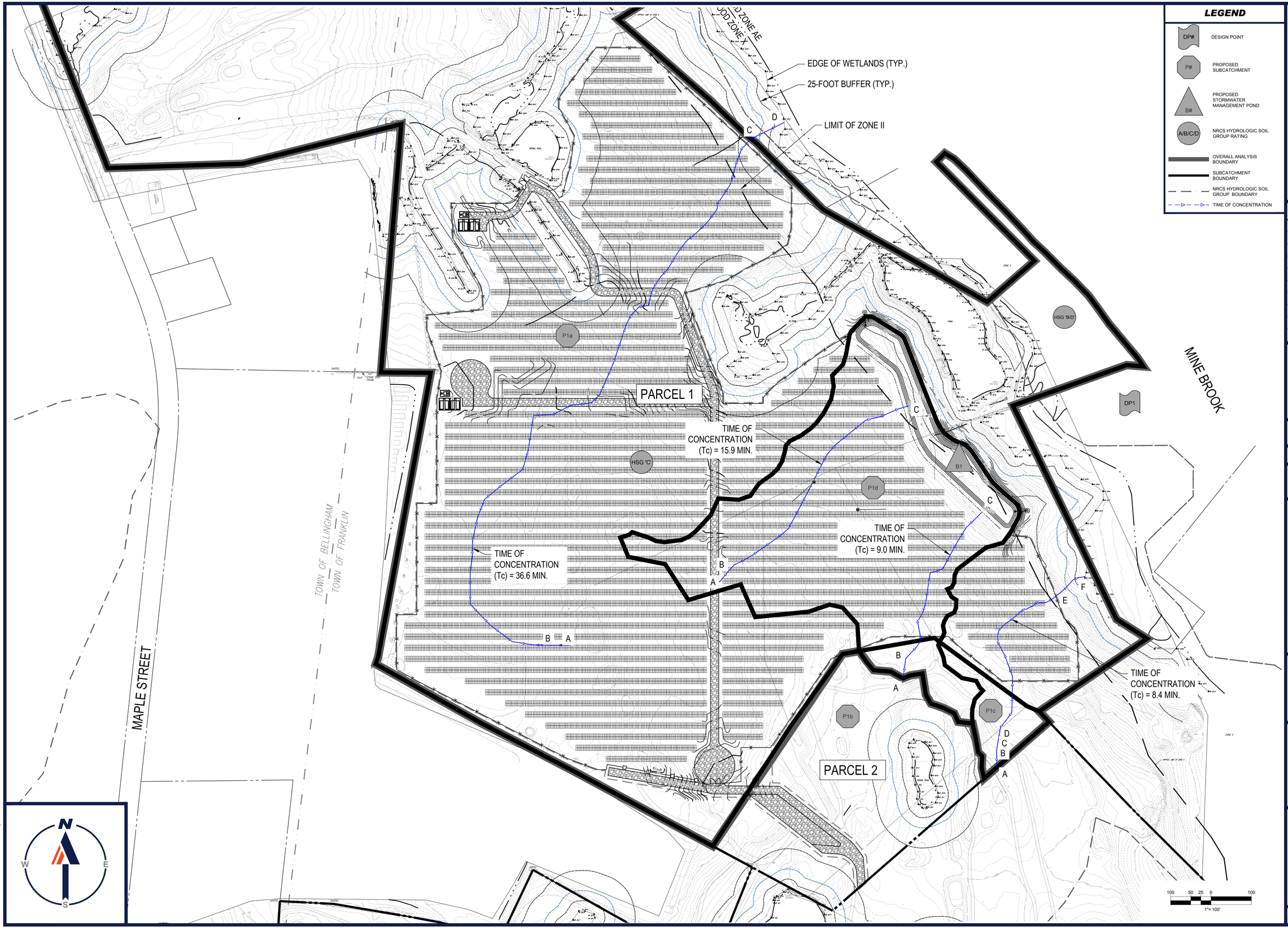
PROPOSED CONDITIONS DRAINAGE AREA MAP (A)

SHEET NUMBER:

PRDAM-A

ORG. DATE - 04/13/2023

P:\2021\201257\TECH\NCAL\STORMWATER\W201257_PR_---\LAYOUT PRDAM-A PROP. WATERSHED-24X36



LEGEND

- DP# — DESIGN POINT
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- A/B/C/D — NRCS HYDROLOGIC SOIL GROUP RATING
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- NRCS HYDROLOGIC SOIL GROUP BOUNDARY
- TIME OF CONCENTRATION

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352 TURNPIKE ROAD
SOUTHBOROUGH, MA 01772
Phone: (508) 480-9900

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SHEET TITLE:

PROPOSED CONDITIONS DRAINAGE AREA MAP (B)

SHEET NUMBER:

PRDAM-B

ORG. DATE - 04/13/2023

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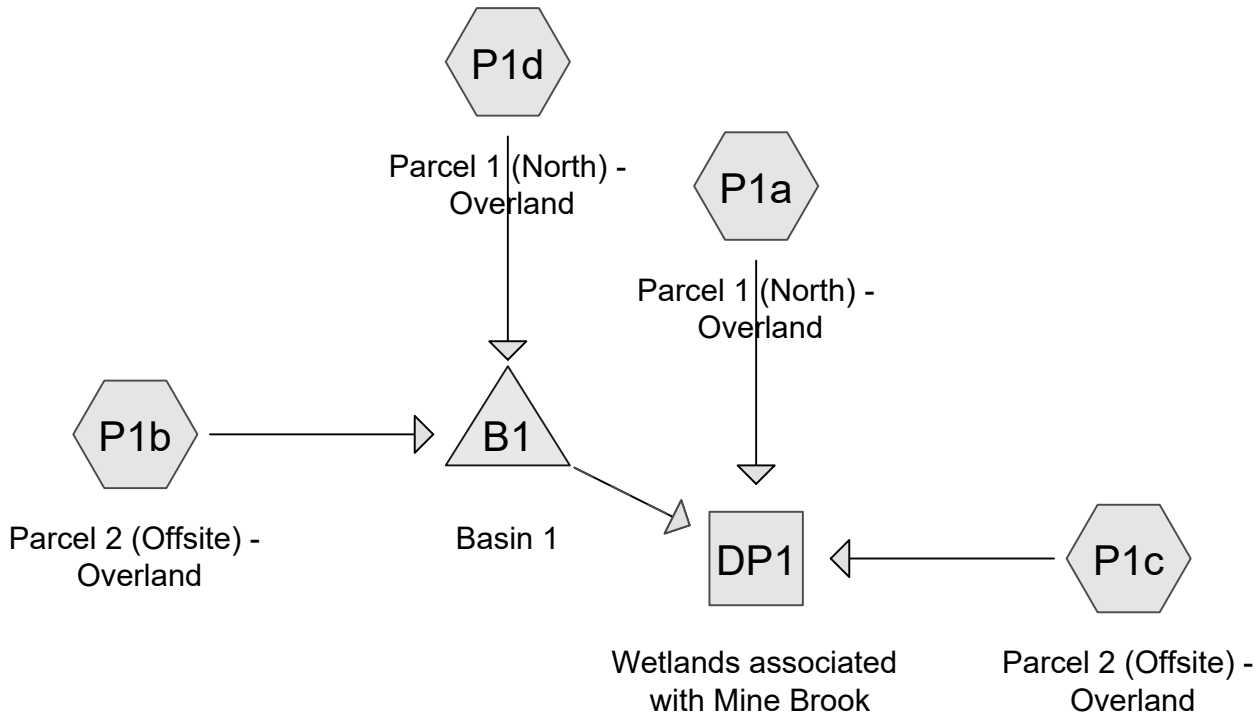

Subcat Reach Pond Link

Routing Diagram for W201257-PR-North
Prepared by Bohler, Printed 4/10/2023

Area Listing (selected nodes)

Area (acres)	CN	Description (subcatchment-numbers)
18.225	74	>75% Grass cover, Good, HSG C (P1a, P1b, P1c)
0.570	98	Basin Bottom, 0% imp, HSG C (P1d)
1.588	89	Gravel roads, HSG C (P1a, P1d)
37.561	71	Meadow, non-grazed, HSG C (P1a, P1d)
0.710	98	Paved parking, HSG C (P1a, P1b, P1c)
1.753	98	Water Surface, HSG C (P1a)
11.727	70	Woods, Good, HSG C (P1a, P1b, P1c)
1.847	77	Woods, Good, HSG D (P1a)
73.981	**73**	**TOTAL AREA**

Soil Listing (selected nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
72.134	HSG C	P1a, P1b, P1c, P1d
1.847	HSG D	P1a
0.000	Other	
73.981		**TOTAL AREA**

Ground Covers (selected nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	18.225	0.000	0.000	18.225	>75% Grass cover, Good	P1a, P1b, P1c
0.000	0.000	0.570	0.000	0.000	0.570	Basin Bottom, 0% imp	P1d
0.000	0.000	1.588	0.000	0.000	1.588	Gravel roads	P1a, P1d
0.000	0.000	37.561	0.000	0.000	37.561	Meadow, non-grazed	P1a, P1d
0.000	0.000	0.710	0.000	0.000	0.710	Paved parking	P1a, P1b, P1c
0.000	0.000	1.753	0.000	0.000	1.753	Water Surface	P1a
0.000	0.000	11.727	1.847	0.000	13.574	Woods, Good	P1a, P1b, P1c
0.000	**0.000**	**72.134**	**1.847**	**0.000**	**73.981**	**TOTAL AREA**	

Pipe Listing (selected nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	B1	201.50	201.25	28.0	0.0089	0.013	12.0	0.0	0.0

Time span=1.00-48.00 hrs, dt=0.05 hrs, 941 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentP1a: Parcel 1 (North) - Runoff Area=64.438 ac 3.67% Impervious Runoff Depth=0.98"
Flow Length=1,823' Tc=36.6 min CN=73 Runoff=35.68 cfs 5.277 af

SubcatchmentP1b: Parcel 2 (Offsite) - Runoff Area=0.595 ac 9.24% Impervious Runoff Depth=1.15"
Flow Length=485' Tc=9.0 min CN=76 Runoff=0.69 cfs 0.057 af

SubcatchmentP1c: Parcel 2 (Offsite) - Runoff Area=0.668 ac 6.74% Impervious Runoff Depth=1.09"
Flow Length=615' Tc=8.4 min CN=75 Runoff=0.74 cfs 0.061 af

SubcatchmentP1d: Parcel 1 (North) - Runoff Area=8.280 ac 0.00% Impervious Runoff Depth=0.98"
Flow Length=690' Tc=15.9 min CN=73 Runoff=6.51 cfs 0.678 af

Reach DP1: Wetlands associated with Mine Brook Inflow=37.10 cfs 6.071 af
Outflow=37.10 cfs 6.071 af

Pond B1: Basin 1 Peak Elev=202.48' Storage=12,309 cf Inflow=7.03 cfs 0.735 af
Primary=1.34 cfs 0.733 af Secondary=0.00 cfs 0.000 af Outflow=1.34 cfs 0.733 af

Total Runoff Area = 73.981 ac Runoff Volume = 6.074 af Average Runoff Depth = 0.99"
96.67% Pervious = 71.518 ac 3.33% Impervious = 2.463 ac

Summary for Subcatchment P1a: Parcel 1 (North) - Overland

Runoff = 35.68 cfs @ 12.56 hrs, Volume= 5.277 af, Depth= 0.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-yr Rainfall=3.20"

Area (ac)	CN	Description
0.610	98	Paved parking, HSG C
1.488	89	Gravel roads, HSG C
29.951	71	Meadow, non-grazed, HSG C
17.208	74	>75% Grass cover, Good, HSG C
11.581	70	Woods, Good, HSG C
1.753	98	Water Surface, HSG C
1.847	77	Woods, Good, HSG D
64.438	73	Weighted Average
62.075		96.33% Pervious Area
2.363		3.67% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.3	50	0.0060	0.06		**Sheet Flow, 249.7-249.4 (A-B)** Grass: Dense n= 0.240 P2= 3.20"
22.4	1,681	0.0320	1.25		**Shallow Concentrated Flow, 249.4-195 (B-C)** Short Grass Pasture Kv= 7.0 fps
0.9	92	0.1090	1.65		**Shallow Concentrated Flow, 195-185 (C-D)** Woodland Kv= 5.0 fps
36.6	1,823	Total			

Summary for Subcatchment P1b: Parcel 2 (Offsite) - Overland

Runoff = 0.69 cfs @ 12.14 hrs, Volume= 0.057 af, Depth= 1.15"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-yr Rainfall=3.20"

Area (ac)	CN	Description
0.091	70	Woods, Good, HSG C
0.055	98	Paved parking, HSG C
0.449	74	>75% Grass cover, Good, HSG C
0.595	76	Weighted Average
0.540		90.76% Pervious Area
0.055		9.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.0300	0.17		**Sheet Flow, 230-228.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
4.2	435	0.0610	1.73		**Shallow Concentrated Flow, 228.5-202 (B-C)** Short Grass Pasture Kv= 7.0 fps
9.0	485	Total			

Summary for Subcatchment P1c: Parcel 2 (Offsite) - Overland

Runoff = 0.74 cfs @ 12.13 hrs, Volume= 0.061 af, Depth= 1.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-yr Rainfall=3.20"

Area (ac)	CN	Description
0.055	70	Woods, Good, HSG C
0.045	98	Paved parking, HSG C
0.568	74	>75% Grass cover, Good, HSG C
0.668	75	Weighted Average
0.623		93.26% Pervious Area
0.045		6.74% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	42	0.0520	0.21		**Sheet Flow, 238.7-236.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
0.1	8	0.0750	1.37		**Shallow Concentrated Flow, 236.5-235.9 (B-C)** Woodland Kv= 5.0 fps
0.4	28	0.0670	1.29		**Shallow Concentrated Flow, 235.9-234 (C-D)** Woodland Kv= 5.0 fps
3.8	458	0.0810	1.99		**Shallow Concentrated Flow, 234-197 (D-E)** Short Grass Pasture Kv= 7.0 fps
0.8	79	0.1200	1.73		**Shallow Concentrated Flow, 197-187.5 (E-F)** Woodland Kv= 5.0 fps
8.4	615	Total			

Summary for Subcatchment P1d: Parcel 1 (North) - Overland

Runoff = 6.51 cfs @ 12.24 hrs, Volume= 0.678 af, Depth= 0.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 2-yr Rainfall=3.20"

	Area (ac)	CN	Description
	7.610	71	Meadow, non-grazed, HSG C
*	0.570	98	Basin Bottom, 0% imp, HSG C
	0.100	89	Gravel roads, HSG C
	8.280	73	Weighted Average
	8.280		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5	50	0.0140	0.09		**Sheet Flow, 242-241.3 (A-B)** Grass: Dense n= 0.240 P2= 3.20"
6.4	640	0.0570	1.67		**Shallow Concentrated Flow, 241.3-205 (B-C)** Short Grass Pasture Kv= 7.0 fps
15.9	690	Total			

Summary for Reach DP1: Wetlands associated with Mine Brook

Inflow Area = 73.981 ac, 3.33% Impervious, Inflow Depth > 0.98" for 2-yr event
Inflow = 37.10 cfs @ 12.56 hrs, Volume= 6.071 af
Outflow = 37.10 cfs @ 12.56 hrs, Volume= 6.071 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs

Summary for Pond B1: Basin 1

Inflow Area = 8.875 ac, 0.62% Impervious, Inflow Depth = 0.99" for 2-yr event
Inflow = 7.03 cfs @ 12.23 hrs, Volume= 0.735 af
Outflow = 1.34 cfs @ 13.07 hrs, Volume= 0.733 af, Atten= 81%, Lag= 49.9 min
Primary = 1.34 cfs @ 13.07 hrs, Volume= 0.733 af
Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 202.48' @ 13.07 hrs Surf.Area= 26,367 sf Storage= 12,309 cf

Plug-Flow detention time= 179.8 min calculated for 0.732 af (100% of inflow)
Center-of-Mass det. time= 179.1 min (1,052.4 - 873.3)

Volume	Invert	Avail.Storage	Storage Description
#1	202.00'	91,179 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
202.00	24,471	0	0
203.00	28,387	26,429	26,429
204.00	32,361	30,374	56,803
205.00	36,391	34,376	91,179

Device	Routing	Invert	Outlet Devices
#1	Primary	201.50'	**12.0" Round Culvert X 2.00** L= 28.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 201.50' / 201.25' S= 0.0089 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	202.00'	**9.0" W x 4.0" H Vert. Orifice X 2.00** C= 0.600
#3	Device 1	203.75'	**24.0" x 24.0" Horiz. Grate X 2.00** C= 0.600 Limited to weir flow at low heads
#4	Secondary	204.00'	**20.0' long x 10.0' breadth Broad-Crested Rectangular Weir X 2.00** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Primary OutFlow Max=1.34 cfs @ 13.07 hrs HW=202.48' (Free Discharge)
└─**1=Culvert** (Passes 1.34 cfs of 4.75 cfs potential flow)
  ├─**2=Orifice** (Orifice Controls 1.34 cfs @ 2.68 fps)
  └─**3=Grate** (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 1.00 hrs HW=202.00' (Free Discharge)
└─**4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Time span=1.00-48.00 hrs, dt=0.05 hrs, 941 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentP1a: Parcel 1 (North) - Runoff Area=64.438 ac 3.67% Impervious Runoff Depth=2.05"
Flow Length=1,823' Tc=36.6 min CN=73 Runoff=78.44 cfs 10.996 af

SubcatchmentP1b: Parcel 2 (Offsite) - Runoff Area=0.595 ac 9.24% Impervious Runoff Depth=2.29"
Flow Length=485' Tc=9.0 min CN=76 Runoff=1.41 cfs 0.114 af

SubcatchmentP1c: Parcel 2 (Offsite) - Runoff Area=0.668 ac 6.74% Impervious Runoff Depth=2.21"
Flow Length=615' Tc=8.4 min CN=75 Runoff=1.56 cfs 0.123 af

SubcatchmentP1d: Parcel 1 (North) - Runoff Area=8.280 ac 0.00% Impervious Runoff Depth=2.05"
Flow Length=690' Tc=15.9 min CN=73 Runoff=14.37 cfs 1.413 af

Reach DP1: Wetlands associated with Mine Brook Inflow=81.04 cfs 12.643 af
Outflow=81.04 cfs 12.643 af

Pond B1: Basin 1 Peak Elev=203.11' Storage=29,453 cf Inflow=15.52 cfs 1.527 af
Primary=2.33 cfs 1.523 af Secondary=0.00 cfs 0.000 af Outflow=2.33 cfs 1.523 af

Total Runoff Area = 73.981 ac Runoff Volume = 12.646 af Average Runoff Depth = 2.05"
96.67% Pervious = 71.518 ac 3.33% Impervious = 2.463 ac

Summary for Subcatchment P1a: Parcel 1 (North) - Overland

Runoff = 78.44 cfs @ 12.53 hrs, Volume= 10.996 af, Depth= 2.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-yr Rainfall=4.70"

Area (ac)	CN	Description
0.610	98	Paved parking, HSG C
1.488	89	Gravel roads, HSG C
29.951	71	Meadow, non-grazed, HSG C
17.208	74	>75% Grass cover, Good, HSG C
11.581	70	Woods, Good, HSG C
1.753	98	Water Surface, HSG C
1.847	77	Woods, Good, HSG D
64.438	73	Weighted Average
62.075		96.33% Pervious Area
2.363		3.67% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.3	50	0.0060	0.06		**Sheet Flow, 249.7-249.4 (A-B)** Grass: Dense n= 0.240 P2= 3.20"
22.4	1,681	0.0320	1.25		**Shallow Concentrated Flow, 249.4-195 (B-C)** Short Grass Pasture Kv= 7.0 fps
0.9	92	0.1090	1.65		**Shallow Concentrated Flow, 195-185 (C-D)** Woodland Kv= 5.0 fps
36.6	1,823	Total			

Summary for Subcatchment P1b: Parcel 2 (Offsite) - Overland

Runoff = 1.41 cfs @ 12.13 hrs, Volume= 0.114 af, Depth= 2.29"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-yr Rainfall=4.70"

Area (ac)	CN	Description
0.091	70	Woods, Good, HSG C
0.055	98	Paved parking, HSG C
0.449	74	>75% Grass cover, Good, HSG C
0.595	76	Weighted Average
0.540		90.76% Pervious Area
0.055		9.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.0300	0.17		**Sheet Flow, 230-228.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
4.2	435	0.0610	1.73		**Shallow Concentrated Flow, 228.5-202 (B-C)** Short Grass Pasture Kv= 7.0 fps
9.0	485	Total			

Summary for Subcatchment P1c: Parcel 2 (Offsite) - Overland

Runoff = 1.56 cfs @ 12.12 hrs, Volume= 0.123 af, Depth= 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-yr Rainfall=4.70"

Area (ac)	CN	Description
0.055	70	Woods, Good, HSG C
0.045	98	Paved parking, HSG C
0.568	74	>75% Grass cover, Good, HSG C
0.668	75	Weighted Average
0.623		93.26% Pervious Area
0.045		6.74% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	42	0.0520	0.21		**Sheet Flow, 238.7-236.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
0.1	8	0.0750	1.37		**Shallow Concentrated Flow, 236.5-235.9 (B-C)** Woodland Kv= 5.0 fps
0.4	28	0.0670	1.29		**Shallow Concentrated Flow, 235.9-234 (C-D)** Woodland Kv= 5.0 fps
3.8	458	0.0810	1.99		**Shallow Concentrated Flow, 234-197 (D-E)** Short Grass Pasture Kv= 7.0 fps
0.8	79	0.1200	1.73		**Shallow Concentrated Flow, 197-187.5 (E-F)** Woodland Kv= 5.0 fps
8.4	615	Total			

Summary for Subcatchment P1d: Parcel 1 (North) - Overland

Runoff = 14.37 cfs @ 12.23 hrs, Volume= 1.413 af, Depth= 2.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 10-yr Rainfall=4.70"

	Area (ac)	CN	Description
	7.610	71	Meadow, non-grazed, HSG C
*	0.570	98	Basin Bottom, 0% imp, HSG C
	0.100	89	Gravel roads, HSG C
	8.280	73	Weighted Average
	8.280		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5	50	0.0140	0.09		**Sheet Flow, 242-241.3 (A-B)** Grass: Dense n= 0.240 P2= 3.20"
6.4	640	0.0570	1.67		**Shallow Concentrated Flow, 241.3-205 (B-C)** Short Grass Pasture Kv= 7.0 fps
15.9	690	Total			

Summary for Reach DP1: Wetlands associated with Mine Brook

Inflow Area = 73.981 ac, 3.33% Impervious, Inflow Depth = 2.05" for 10-yr event
Inflow = 81.04 cfs @ 12.53 hrs, Volume= 12.643 af
Outflow = 81.04 cfs @ 12.53 hrs, Volume= 12.643 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs

Summary for Pond B1: Basin 1

Inflow Area = 8.875 ac, 0.62% Impervious, Inflow Depth = 2.06" for 10-yr event
Inflow = 15.52 cfs @ 12.22 hrs, Volume= 1.527 af
Outflow = 2.33 cfs @ 13.15 hrs, Volume= 1.523 af, Atten= 85%, Lag= 56.0 min
Primary = 2.33 cfs @ 13.15 hrs, Volume= 1.523 af
Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 203.11' @ 13.15 hrs Surf.Area= 28,807 sf Storage= 29,453 cf

Plug-Flow detention time= 187.7 min calculated for 1.522 af (100% of inflow)
Center-of-Mass det. time= 187.7 min (1,039.0 - 851.3)

Volume	Invert	Avail.Storage	Storage Description
#1	202.00'	91,179 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
202.00	24,471	0	0
203.00	28,387	26,429	26,429
204.00	32,361	30,374	56,803
205.00	36,391	34,376	91,179

Device	Routing	Invert	Outlet Devices
#1	Primary	201.50'	**12.0" Round Culvert X 2.00** L= 28.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 201.50' / 201.25' S= 0.0089 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	202.00'	**9.0" W x 4.0" H Vert. Orifice X 2.00** C= 0.600
#3	Device 1	203.75'	**24.0" x 24.0" Horiz. Grate X 2.00** C= 0.600 Limited to weir flow at low heads
#4	Secondary	204.00'	**20.0' long x 10.0' breadth Broad-Crested Rectangular Weir X 2.00** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Primary OutFlow Max=2.33 cfs @ 13.15 hrs HW=203.11' (Free Discharge)
└─**1=Culvert** (Passes 2.33 cfs of 7.56 cfs potential flow)
  ├─**2=Orifice** (Orifice Controls 2.33 cfs @ 4.66 fps)
  └─**3=Grate** (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 1.00 hrs HW=202.00' (Free Discharge)
└─**4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Time span=1.00-48.00 hrs, dt=0.05 hrs, 941 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentP1a: Parcel 1 (North) - Runoff Area=64.438 ac 3.67% Impervious Runoff Depth=2.68"
Flow Length=1,823' Tc=36.6 min CN=73 Runoff=103.58 cfs 14.385 af

SubcatchmentP1b: Parcel 2 (Offsite) - Runoff Area=0.595 ac 9.24% Impervious Runoff Depth=2.95"
Flow Length=485' Tc=9.0 min CN=76 Runoff=1.82 cfs 0.146 af

SubcatchmentP1c: Parcel 2 (Offsite) - Runoff Area=0.668 ac 6.74% Impervious Runoff Depth=2.86"
Flow Length=615' Tc=8.4 min CN=75 Runoff=2.03 cfs 0.159 af

SubcatchmentP1d: Parcel 1 (North) - Runoff Area=8.280 ac 0.00% Impervious Runoff Depth=2.68"
Flow Length=690' Tc=15.9 min CN=73 Runoff=19.07 cfs 1.848 af

Reach DP1: Wetlands associated with Mine Brook Inflow=106.71 cfs 16.536 af
Outflow=106.71 cfs 16.536 af

Pond B1: Basin 1 Peak Elev=203.48' Storage=40,395 cf Inflow=20.48 cfs 1.995 af
Primary=2.75 cfs 1.992 af Secondary=0.00 cfs 0.000 af Outflow=2.75 cfs 1.992 af

Total Runoff Area = 73.981 ac Runoff Volume = 16.539 af Average Runoff Depth = 2.68"
96.67% Pervious = 71.518 ac 3.33% Impervious = 2.463 ac

Summary for Subcatchment P1a: Parcel 1 (North) - Overland

Runoff = 103.58 cfs @ 12.52 hrs, Volume= 14.385 af, Depth= 2.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-yr Rainfall=5.50"

Area (ac)	CN	Description
0.610	98	Paved parking, HSG C
1.488	89	Gravel roads, HSG C
29.951	71	Meadow, non-grazed, HSG C
17.208	74	>75% Grass cover, Good, HSG C
11.581	70	Woods, Good, HSG C
1.753	98	Water Surface, HSG C
1.847	77	Woods, Good, HSG D
64.438	73	Weighted Average
62.075		96.33% Pervious Area
2.363		3.67% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.3	50	0.0060	0.06		**Sheet Flow, 249.7-249.4 (A-B)** Grass: Dense n= 0.240 P2= 3.20"
22.4	1,681	0.0320	1.25		**Shallow Concentrated Flow, 249.4-195 (B-C)** Short Grass Pasture Kv= 7.0 fps
0.9	92	0.1090	1.65		**Shallow Concentrated Flow, 195-185 (C-D)** Woodland Kv= 5.0 fps
36.6	1,823	Total			

Summary for Subcatchment P1b: Parcel 2 (Offsite) - Overland

Runoff = 1.82 cfs @ 12.13 hrs, Volume= 0.146 af, Depth= 2.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-yr Rainfall=5.50"

Area (ac)	CN	Description
0.091	70	Woods, Good, HSG C
0.055	98	Paved parking, HSG C
0.449	74	>75% Grass cover, Good, HSG C
0.595	76	Weighted Average
0.540		90.76% Pervious Area
0.055		9.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.0300	0.17		**Sheet Flow, 230-228.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
4.2	435	0.0610	1.73		**Shallow Concentrated Flow, 228.5-202 (B-C)** Short Grass Pasture Kv= 7.0 fps
9.0	485	Total			

Summary for Subcatchment P1c: Parcel 2 (Offsite) - Overland

Runoff = 2.03 cfs @ 12.12 hrs, Volume= 0.159 af, Depth= 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-yr Rainfall=5.50"

Area (ac)	CN	Description
0.055	70	Woods, Good, HSG C
0.045	98	Paved parking, HSG C
0.568	74	>75% Grass cover, Good, HSG C
0.668	75	Weighted Average
0.623		93.26% Pervious Area
0.045		6.74% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	42	0.0520	0.21		**Sheet Flow, 238.7-236.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
0.1	8	0.0750	1.37		**Shallow Concentrated Flow, 236.5-235.9 (B-C)** Woodland Kv= 5.0 fps
0.4	28	0.0670	1.29		**Shallow Concentrated Flow, 235.9-234 (C-D)** Woodland Kv= 5.0 fps
3.8	458	0.0810	1.99		**Shallow Concentrated Flow, 234-197 (D-E)** Short Grass Pasture Kv= 7.0 fps
0.8	79	0.1200	1.73		**Shallow Concentrated Flow, 197-187.5 (E-F)** Woodland Kv= 5.0 fps
8.4	615	Total			

Summary for Subcatchment P1d: Parcel 1 (North) - Overland

Runoff = 19.07 cfs @ 12.22 hrs, Volume= 1.848 af, Depth= 2.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 25-yr Rainfall=5.50"

	Area (ac)	CN	Description
	7.610	71	Meadow, non-grazed, HSG C
*	0.570	98	Basin Bottom, 0% imp, HSG C
	0.100	89	Gravel roads, HSG C
	8.280	73	Weighted Average
	8.280		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5	50	0.0140	0.09		**Sheet Flow, 242-241.3 (A-B)** Grass: Dense n= 0.240 P2= 3.20"
6.4	640	0.0570	1.67		**Shallow Concentrated Flow, 241.3-205 (B-C)** Short Grass Pasture Kv= 7.0 fps
15.9	690	Total			

Summary for Reach DP1: Wetlands associated with Mine Brook

Inflow Area = 73.981 ac, 3.33% Impervious, Inflow Depth = 2.68" for 25-yr event
Inflow = 106.71 cfs @ 12.52 hrs, Volume= 16.536 af
Outflow = 106.71 cfs @ 12.52 hrs, Volume= 16.536 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs

Summary for Pond B1: Basin 1

Inflow Area = 8.875 ac, 0.62% Impervious, Inflow Depth = 2.70" for 25-yr event
Inflow = 20.48 cfs @ 12.22 hrs, Volume= 1.995 af
Outflow = 2.75 cfs @ 13.23 hrs, Volume= 1.992 af, Atten= 87%, Lag= 60.9 min
Primary = 2.75 cfs @ 13.23 hrs, Volume= 1.992 af
Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 203.48' @ 13.23 hrs Surf.Area= 30,279 sf Storage= 40,395 cf

Plug-Flow detention time= 204.5 min calculated for 1.989 af (100% of inflow)
Center-of-Mass det. time= 204.6 min (1,048.1 - 843.5)

Volume	Invert	Avail.Storage	Storage Description
#1	202.00'	91,179 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
202.00	24,471	0	0
203.00	28,387	26,429	26,429
204.00	32,361	30,374	56,803
205.00	36,391	34,376	91,179

Device	Routing	Invert	Outlet Devices
#1	Primary	201.50'	**12.0" Round Culvert X 2.00** L= 28.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 201.50' / 201.25' S= 0.0089 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	202.00'	**9.0" W x 4.0" H Vert. Orifice X 2.00** C= 0.600
#3	Device 1	203.75'	**24.0" x 24.0" Horiz. Grate X 2.00** C= 0.600 Limited to weir flow at low heads
#4	Secondary	204.00'	**20.0' long x 10.0' breadth Broad-Crested Rectangular Weir X 2.00** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Primary OutFlow Max=2.75 cfs @ 13.23 hrs HW=203.48' (Free Discharge)
└─**1=Culvert** (Passes 2.75 cfs of 9.05 cfs potential flow)
  ├─**2=Orifice** (Orifice Controls 2.75 cfs @ 5.51 fps)
  └─**3=Grate** (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 1.00 hrs HW=202.00' (Free Discharge)
└─**4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Time span=1.00-48.00 hrs, dt=0.05 hrs, 941 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentP1a: Parcel 1 (North) - Runoff Area=64.438 ac 3.67% Impervious Runoff Depth=3.68"
Flow Length=1,823' Tc=36.6 min CN=73 Runoff=142.89 cfs 19.750 af

SubcatchmentP1b: Parcel 2 (Offsite) - Runoff Area=0.595 ac 9.24% Impervious Runoff Depth=3.99"
Flow Length=485' Tc=9.0 min CN=76 Runoff=2.46 cfs 0.198 af

SubcatchmentP1c: Parcel 2 (Offsite) - Runoff Area=0.668 ac 6.74% Impervious Runoff Depth=3.89"
Flow Length=615' Tc=8.4 min CN=75 Runoff=2.76 cfs 0.216 af

SubcatchmentP1d: Parcel 1 (North) - Runoff Area=8.280 ac 0.00% Impervious Runoff Depth=3.68"
Flow Length=690' Tc=15.9 min CN=73 Runoff=26.32 cfs 2.538 af

Reach DP1: Wetlands associated with Mine Brook Inflow=146.93 cfs 22.698 af
Outflow=146.93 cfs 22.698 af

Pond B1: Basin 1 Peak Elev=203.91' Storage=53,836 cf Inflow=28.22 cfs 2.736 af
Primary=6.47 cfs 2.732 af Secondary=0.00 cfs 0.000 af Outflow=6.47 cfs 2.732 af

Total Runoff Area = 73.981 ac Runoff Volume = 22.702 af Average Runoff Depth = 3.68"
96.67% Pervious = 71.518 ac 3.33% Impervious = 2.463 ac

Summary for Subcatchment P1a: Parcel 1 (North) - Overland

Runoff = 142.89 cfs @ 12.51 hrs, Volume= 19.750 af, Depth= 3.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-yr Rainfall=6.70"

Area (ac)	CN	Description
0.610	98	Paved parking, HSG C
1.488	89	Gravel roads, HSG C
29.951	71	Meadow, non-grazed, HSG C
17.208	74	>75% Grass cover, Good, HSG C
11.581	70	Woods, Good, HSG C
1.753	98	Water Surface, HSG C
1.847	77	Woods, Good, HSG D
64.438	73	Weighted Average
62.075		96.33% Pervious Area
2.363		3.67% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.3	50	0.0060	0.06		**Sheet Flow, 249.7-249.4 (A-B)** Grass: Dense n= 0.240 P2= 3.20"
22.4	1,681	0.0320	1.25		**Shallow Concentrated Flow, 249.4-195 (B-C)** Short Grass Pasture Kv= 7.0 fps
0.9	92	0.1090	1.65		**Shallow Concentrated Flow, 195-185 (C-D)** Woodland Kv= 5.0 fps
36.6	1,823	Total			

Summary for Subcatchment P1b: Parcel 2 (Offsite) - Overland

Runoff = 2.46 cfs @ 12.13 hrs, Volume= 0.198 af, Depth= 3.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-yr Rainfall=6.70"

Area (ac)	CN	Description
0.091	70	Woods, Good, HSG C
0.055	98	Paved parking, HSG C
0.449	74	>75% Grass cover, Good, HSG C
0.595	76	Weighted Average
0.540		90.76% Pervious Area
0.055		9.24% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.8	50	0.0300	0.17		**Sheet Flow, 230-228.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
4.2	435	0.0610	1.73		**Shallow Concentrated Flow, 228.5-202 (B-C)** Short Grass Pasture Kv= 7.0 fps
9.0	485	Total			

Summary for Subcatchment P1c: Parcel 2 (Offsite) - Overland

Runoff = 2.76 cfs @ 12.12 hrs, Volume= 0.216 af, Depth= 3.89"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-yr Rainfall=6.70"

Area (ac)	CN	Description
0.055	70	Woods, Good, HSG C
0.045	98	Paved parking, HSG C
0.568	74	>75% Grass cover, Good, HSG C
0.668	75	Weighted Average
0.623		93.26% Pervious Area
0.045		6.74% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	42	0.0520	0.21		**Sheet Flow, 238.7-236.5 (A-B)** Grass: Short n= 0.150 P2= 3.20"
0.1	8	0.0750	1.37		**Shallow Concentrated Flow, 236.5-235.9 (B-C)** Woodland Kv= 5.0 fps
0.4	28	0.0670	1.29		**Shallow Concentrated Flow, 235.9-234 (C-D)** Woodland Kv= 5.0 fps
3.8	458	0.0810	1.99		**Shallow Concentrated Flow, 234-197 (D-E)** Short Grass Pasture Kv= 7.0 fps
0.8	79	0.1200	1.73		**Shallow Concentrated Flow, 197-187.5 (E-F)** Woodland Kv= 5.0 fps
8.4	615	Total			

Summary for Subcatchment P1d: Parcel 1 (North) - Overland

Runoff = 26.32 cfs @ 12.22 hrs, Volume= 2.538 af, Depth= 3.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Type III 24-hr 100-yr Rainfall=6.70"

	Area (ac)	CN	Description
	7.610	71	Meadow, non-grazed, HSG C
*	0.570	98	Basin Bottom, 0% imp, HSG C
	0.100	89	Gravel roads, HSG C
	8.280	73	Weighted Average
	8.280		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
9.5	50	0.0140	0.09		**Sheet Flow, 242-241.3 (A-B)** Grass: Dense n= 0.240 P2= 3.20"
6.4	640	0.0570	1.67		**Shallow Concentrated Flow, 241.3-205 (B-C)** Short Grass Pasture Kv= 7.0 fps
15.9	690	Total			

Summary for Reach DP1: Wetlands associated with Mine Brook

Inflow Area = 73.981 ac, 3.33% Impervious, Inflow Depth = 3.68" for 100-yr event
Inflow = 146.93 cfs @ 12.52 hrs, Volume= 22.698 af
Outflow = 146.93 cfs @ 12.52 hrs, Volume= 22.698 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs

Summary for Pond B1: Basin 1

Inflow Area = 8.875 ac, 0.62% Impervious, Inflow Depth = 3.70" for 100-yr event
Inflow = 28.22 cfs @ 12.21 hrs, Volume= 2.736 af
Outflow = 6.47 cfs @ 12.78 hrs, Volume= 2.732 af, Atten= 77%, Lag= 34.1 min
Primary = 6.47 cfs @ 12.78 hrs, Volume= 2.732 af
Secondary = 0.00 cfs @ 1.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 1.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 203.91' @ 12.78 hrs Surf.Area= 31,995 sf Storage= 53,836 cf

Plug-Flow detention time= 206.5 min calculated for 2.729 af (100% of inflow)
Center-of-Mass det. time= 206.9 min (1,041.2 - 834.4)

Volume	Invert	Avail.Storage	Storage Description
#1	202.00'	91,179 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
202.00	24,471	0	0
203.00	28,387	26,429	26,429
204.00	32,361	30,374	56,803
205.00	36,391	34,376	91,179

Device	Routing	Invert	Outlet Devices
#1	Primary	201.50'	**12.0" Round Culvert X 2.00** L= 28.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 201.50' / 201.25' S= 0.0089 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	202.00'	**9.0" W x 4.0" H Vert. Orifice X 2.00** C= 0.600
#3	Device 1	203.75'	**24.0" x 24.0" Horiz. Grate X 2.00** C= 0.600 Limited to weir flow at low heads
#4	Secondary	204.00'	**20.0' long x 10.0' breadth Broad-Crested Rectangular Weir X 2.00** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64

Primary OutFlow Max=6.44 cfs @ 12.78 hrs HW=203.91' (Free Discharge)
└─**1=Culvert** (Passes 6.44 cfs of 10.45 cfs potential flow)
  ├─**2=Orifice** (Orifice Controls 3.18 cfs @ 6.35 fps)
  └─**3=Grate** (Weir Controls 3.27 cfs @ 1.30 fps)

Secondary OutFlow Max=0.00 cfs @ 1.00 hrs HW=202.00' (Free Discharge)
└─**4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

APPENDIX F: STORMWATER CALCULATIONS

- *MA STANDARD #3 – RECHARGE CALCULATIONS*
- *RAINFALL DATA*
- *PIPE SIZING CALCULATIONS*
- *OUTLET PROTECTION SIZING*
- *RIP RAP SIZING CALCULATIONS*

Proposed Solar Array - Parcel 1
160 Maple Street
Bellingham, MA
Bohler Job Number: W201257
April 13, 2023

MA DEP Standard 3: Recharge Volume Calculations

Required Recharge Volume - A Soils (0.60 in.)	
Existing Site Impervious Area (ac)	0.000
Proposed Site Impervious Area (ac)	0.000
Proposed Increase in Site Impervious Area (ac)	0.000
Recharge Volume Required (cf)	**0**

Required Recharge Volume - B Soils (0.35 in.)	
Existing Site Impervious Area (ac)	0.000
Proposed Site Impervious Area (ac)	0.000
Proposed Increase in Site Impervious Area (ac)	0.000
Recharge Volume Required (cf)	**0**

Required Recharge Volume - C Soils (0.25 in.)	
Existing Site Impervious Area (ac)	5.259
Proposed Site Impervious Area (ac)	2.298
Proposed Increase in Site Impervious Area (ac)	-2.961
Recharge Volume Required (cf)	**0**

Required Recharge Volume - D Soils (0.10 in.)	
Existing Site Impervious Area (ac)	0.000
Proposed Site Impervious Area (ac)	0.000
Proposed Increase in Site Impervious Area (ac)	0.000
Recharge Volume Required (cf)	**0**

Total Recharge Volume Required (cf)	**0**

Recharge Volume Adjustment Factor	
Impervious Area Directed to Infiltration BMP (ac)	**0.000**
%Impervious Directed to Infiltration BMP	
Adjustment Factor	
Adjusted Total Recharge Volume Required (cf)	

Provided Recharge Volume*	
Basin 1 (B1)	0
Total Recharge Volume Provided (cf)	**0**

Input Required

*Volume provided below lowest outlet in cubic feet (cf)

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F-1. Rainfall Data for Massachusetts from *Rainfall Frequency Atlas of the United States* (TP-40)

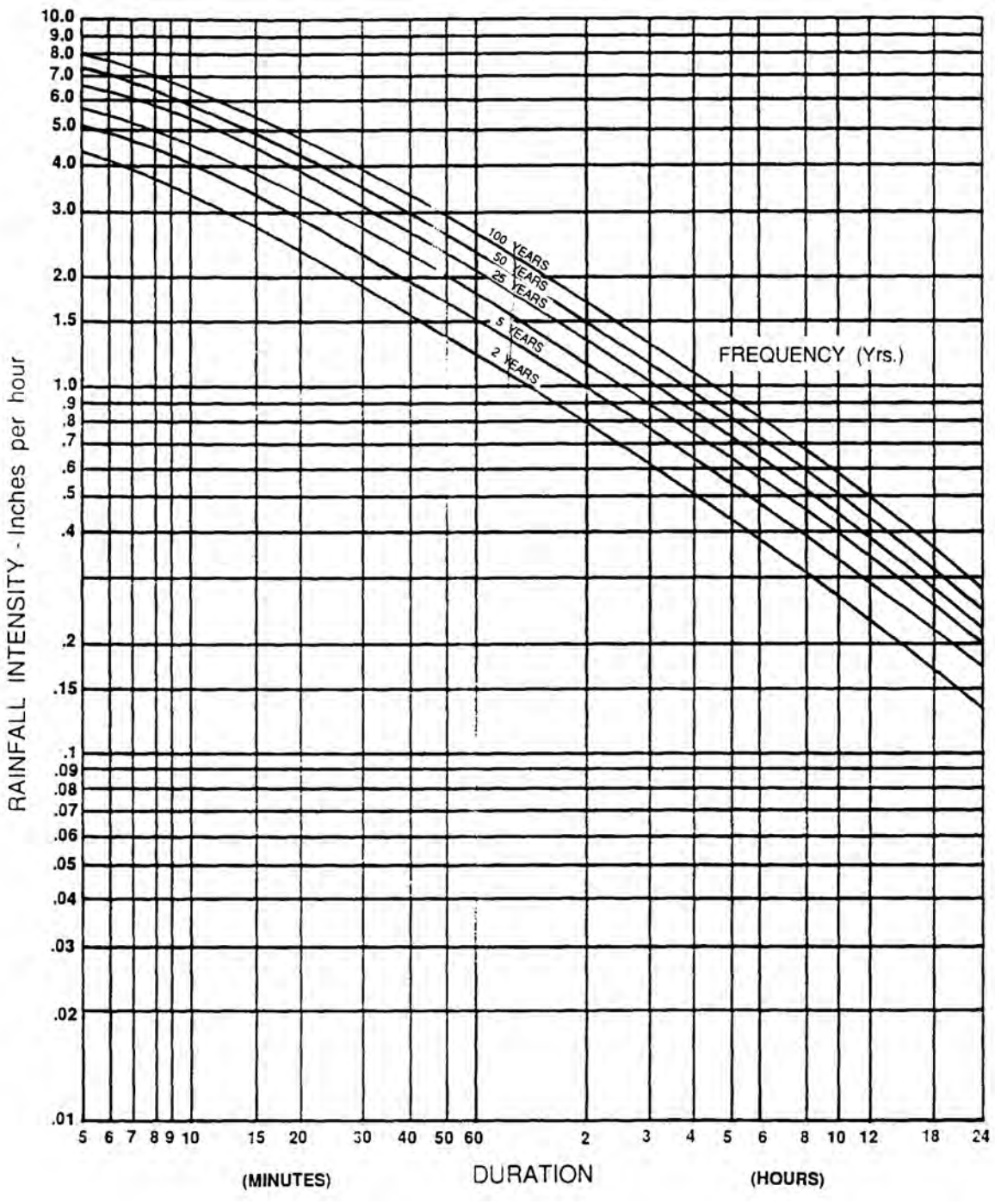
■ Users of this Handbook should note that current MA DEP written guidance (see DEP Waterlines newsletter -- Fall 2000) requires the use of TP-40 Rainfall Data for calculations under the Wetlands Protection Regulations and the Stormwater Management Policy. More stringent design storms may be used under a local bylaw or ordinance. However, DEP will continue to require the use of TP-40 in any case it reviews under the Wetlands Protection Act and Stormwater Management Policy.

Adjusted Technical Paper 40 Design Storms for 24-hour Event by County

County Name	1-yr 24-hr	2-yr 24-hr	5-yr 24-hr	10-yr 24-hr	25-yr 24-hr	50-yr 24-hr	100-yr 24-hr
Barnstable	2.5	3.6	4.5	4.8	5.7	6.4	7.1
Berkshire	2.5	2.9	3.8	4.4	5.1	5.9	6.4
Bristol	2.5	3.4	4.3	4.8	5.6	6.3	7.0
Dukes	2.5	3.6	4.6	4.9	5.8	6.5	7.2
Essex	2.5	3.1	3.9	4.5	5.4	5.9	6.5
Franklin	2.5	2.9	3.8	4.3	5.1	5.8	6.2
Hampden	2.5	3.0	4.0	4.6	5.3	6.0	6.5
Hampshire	2.5	3.0	3.9	4.5	5.2	5.9	6.4
Middlesex	2.5	3.1	4.0	4.5	5.3	5.9	6.5
Nantucket	2.5	3.6	4.6	4.9	5.8	6.5	7.2
Norfolk	2.5	3.2	4.1	4.7	5.5	6.1	6.7
Plymouth	2.5	3.4	4.3	4.7	5.6	6.2	7.0
Suffolk	2.5	3.2	4.0	4.6	5.5	6.0	6.6
Worcester	2.5	3.0	4.0	4.5	5.3	5.9	6.5

Exhibit 8-14

Intensity – Duration – Frequency Curve for Worcester, MA



Source: TR55 - Urban Hydrology for Small Wetlands, NRCS

Proposed Solar Array - Parcel 1
160 Maple Street
Bellingham, MA
Bohler Job Number: W201257
April 13, 2023

Rational Pipe Sizing Calculations

Design Period Storm:		100	Year	Design Period Intensity*		8	in/hr									
LOCATION		IMPERVIOUS			OTHER			SUM CA	Tc (min)	I (in/hr)	Q (cfs)	D (in)	S (ft/ft)	Material	n	Q Full (cfs)	V Full (fps)
FROM	TO	A	C	CA	A	C	CA										
OCS100	FES100	Flow from HydroCAD 100-year storm event (split between outlets)									3.24	12	0.009	HDPE	0.012	3.66	4.66
OCS200	FES200	Flow from HydroCAD 100-year storm event (split between outlets)									3.24	12	0.009	HDPE	0.012	3.66	4.66

*Rainfall intensity provided by TR55 Exhibit 8-14, dated January 2006

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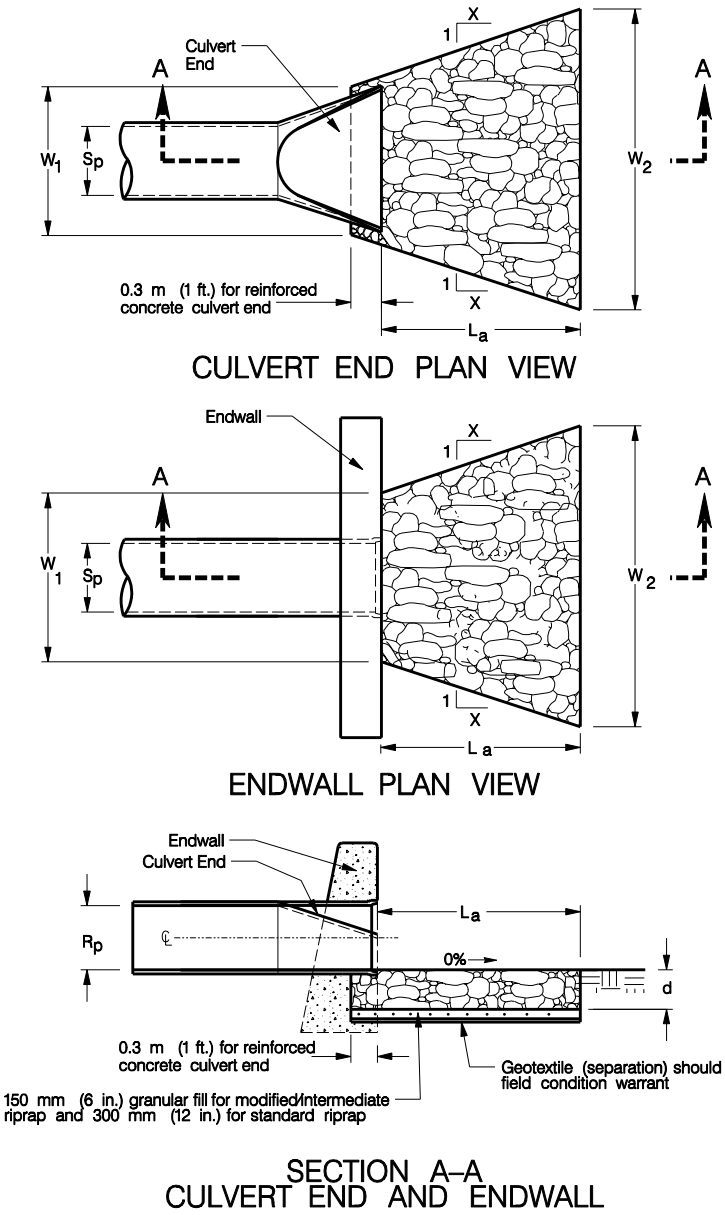
OUTLET PROTECTION - OUTLET VELOCITY ≤ 14 feet/sec

	OUTLET PIPE DIAMETER OR SPAN (in)									
DISCHARGE (cfs)	12	15	18	24	30	36	42	48	54	60
0-5	***10***	***10***		***USE***						
6	12	11								
7		13	***12***							
8		14	13	***12***		***MINIMUM***				
9			14	13						
10			15	13						
11			16	14				***LENGTH***		
12				14						
14				16	***14***					
16				17	15	***14***			***OUTLINED***	
18				18	16	15				
20					17	15	***14***			
22		***USE***			18	16	15			
24						17	15	***14***		
26						17	16	15		
28						18	16	15		
30						19	17	16		
35						20	18	17	***16***	
40			***PREFORMED***				20	18	17	***16***
45							21	19	18	16
50							22	20	18	17
55								21	19	18
60								22	20	19
65								24	21	20
70					***SCOUR***			25	22	20
75								26	23	21
80									24	22
90									26	24
100									28	25
110										27
125							***HOLE***			29
130										30

Table 11-12.1 - Length - L_a (feet)
Type A Riprap Apron

Notes: 1. Bold face outlined boxes indicate minimum L_a to be used for a given pipe diameter or span.
2. Rounding and interpolating are acceptable.



LEGEND

S_p = { Max. inside pipe span (non–circular sections) / Inside pipe diameter (circular sections)

R_p = { Max. inside pipe rise (non–circular sections) / Inside pipe diameter (circular sections)

L_a = Length of riprap apron measured from the end of culvert end section or face of endwall

d = { 300 mm (12 in.) Modified Riprap / 450 mm (18 in.) Intermediate Riprap / 900 mm (36 in.) Standard Riprap

	X	W_1	W_2
Type A Riprap Apron	3	$3S_p$	$3S_p+0.7$ La
Type B Riprap Apron	5	$3S_p$	$3S_p+0.4$ La

Figure 11-13 Type A and B Riprap Apron
(to be used where there is no defined channel downstream of the outlet)

Proposed Solar Array - Parcel 1
160 Maple Street
Bellingham, MA
Bohler Job Number: W201257
April 13, 2023

Rip Rap Sizing Calculations

Design Period Storm: **100** **Year**

Rip Rap Apron Sizing Calculations

Location	Pipe Size (in.)	Pipe Size (ft.)	Q (cfs)	TW (ft.)	V (fps)	W1 (ft.)	La (ft.)	W2 (ft.)	W3 (ft.)	Apron Type	Rip Rap Type
FES100	12	1.0	3.24	0.20	4.66	3.00	10	10	NA	A	Modified
FES200	12	1.0	3.24	0.20	4.66	3.00	10	10	NA	A	Modified

Based ConnDOT Drainage Manual - Type A, B, and C Riprap Aprons

Outlet Velocity (fps)
0-8 - Modified
8-10 - Intermediate
10-14 - Standard

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APPENDIX G: OPERATION AND MAINTENANCE

- *STORMWATER OPERATION AND MAINTENANCE PLAN*
- *INSPECTION REPORT*
- *INSPECTION AND MAINTENANCE LOG FORM*
- *LONG-TERM POLLUTION PREVENTION PLAN*
- *ILLICIT DISCHARGE STATEMENT*
- *SPILL PREVENTION*

STORMWATER OPERATION AND MAINTENANCE PLAN

Proposed Solar Array – Parcel 1
160 Maple Street
Bellingham, MA

RESPONSIBLE PARTY DURING CONSTRUCTION:

NextGrid Mescalbean, LLC
P.O. Box 775 #73069
San Francisco, CA

RESPONSIBLE PARTY POST CONSTRUCTION:

TBD

Construction Phase

During the construction phase, all erosion control devices and measures shall be maintained in accordance with the final record plans, local/state approvals and conditions, the EPA Construction General Permit and the Stormwater Pollution Prevention Plan (SWPPP) if applicable. Additionally, the maintenance of all erosion / siltation control measures during construction shall be the responsibility of the general contractor. Contact information of the OWNER and CONTRACTOR shall be listed in the SWPPP for this site. The SWPPP also includes information regarding construction period allowable and illicit discharges, housekeeping and emergency response procedures. Upon proper notice to the property owner, the Town/City or its authorized designee shall be allowed to enter the property at a reasonable time and in a reasonable manner for the purposes of inspection.

Post Development Controls

Once construction is completed, the post development stormwater controls are to be operated and maintained in compliance with the following permanent procedures (note that the continued implementation of these procedures shall be the responsibility of the Owner or its assignee):

1. Driveways: Sweep at least two (2) times per year and on a more frequent basis depending on sanding operations. All resulting sweepings shall be collected and properly disposed of off site in accordance with MADEP and other applicable requirements.

2. Catch basins, yard drains, manholes and piping: Inspect two (2) times per year and at the end of foliage and snow-removal seasons. These features shall be cleaned two (2) times per year or whenever the depth of deposits is greater than or equal to one half the depth from the bottom of the invert of the lowest pipe in the catch basin or underground system. Accumulated sediment and hydrocarbons present must be removed and properly disposed of off-site in accordance with MADEP and other applicable requirements.

3. Riprap apron / Scour Hole: Riprap and scour holes should be checked at least annually and after every major storm event (generally equal or greater to 3.0 inches in 24 hours) for displaced stones, slumping, and erosion at edges, especially downstream or downslope. If the riprap is damaged, it should be repaired before further damage can take place. Note and

repair any erosion, stone displacement or low spots in the areas. Woody vegetation should be removed from the riprap annually.

4. Stormwater Infiltration Basin: Preventative maintenance after every major storm event during the first three (3) months of operation and at least twice per year thereafter. Inspect structure and pretreatment BMP to ensure proper operation after every major storm event (generally equal or greater to 3.0 inches in 24 hours) for the first three months. Mow the buffer area, side slopes and basin bottom if grassed floor, rake if stone or sand bottom, remove trash and debris, remove grass clippings and accumulated organic matter. Any sediment removed shall be disposed of in accordance with MADEP and other applicable requirements.

All components of the stormwater system will be accessible by the owner or their assignee.

STORMWATER MANAGEMENT SYSTEM

POST-CONSTRUCTION INSPECTION REPORT

LOCATION:

Proposed Solar Array – Parcel 1
160 Maple Street
Bellingham, MA

RESPONSIBLE PARTY:

TBD

NAME OF INSPECTOR:	INSPECTION DATE:
Note Condition of the Following (sediment depth, debris, standing water, damage, etc.):	
Catch Basins / Yard Drains / Manholes / Piping:	
Discharge Points/ Flared End Sections / Rip Rap / Scour Holes:	
Stormwater Infiltration Basin:	
Other:	
Other:	
Note Recommended Actions to be taken on the Following (sediment and/or debris removal, repairs, etc.):	

Catch Basins / Yard Drains / Manholes / Piping:
Discharge Points/ Flared End Sections / Rip Rap / Scour Holes:
Stormwater Infiltration Basin:
Other:
Other:
Comments:

STORMWATER INSPECTION AND MAINTENANCE LOG FORM

Proposed Solar Array – Parcel 1
160 Maple Street - Bellingham, MA

Stormwater Management Practice	Responsible Party	Date	Maintenance Activity Performed

LONG-TERM POLLUTION PREVENTION PLAN

Proposed Solar Array – Parcel 1
160 Maple Street
Bellingham, MA

RESPONSIBLE PARTY DURING CONSTRUCTION:

NextGrid Mescalbean, LLC
P.O. Box 775 #73069
San Francisco, CA

RESPONSIBLE PARTY POST CONSTRUCTION:

TBD

For this site, the Long-Term Pollution Prevention Plan will consist of the following:

- The property owner shall be responsible for "good housekeeping" including proper periodic maintenance of driveways, landscaping, stormwater management components, etc.
- Proper storage and removal of solid waste (dumpsters), if applicable.
- Sweeping of paved driveways a minimum of twice per year with a commercial cleaning unit. Any sediment removed shall be disposed of in accordance with applicable local and state requirements.
- Regular inspections and maintenance of Stormwater Management System as noted in the "O&M Plan".
- Snow removal shall be the responsibility of the property owner. Snow shall not be plowed, dumped and/or placed in infiltration basins or similar stormwater controls. Salting and/or sanding of pavement / walkway areas during winter conditions shall only be done in accordance with all state/local requirements and approvals.
- No outdoor maintenance or washing of vehicles allowed.
- Trash and other debris shall be removed from all areas of the site at least twice yearly.
- In no case shall snow be disposed of or stored in resource areas (wetlands, floodplain, streams, or other water bodies).
- In no case shall snow be disposed of or stored in the stormwater management basin.

OPERATON AND MAINTENANCE TRAINING PROGRAM

The Owner will coordinate an annual in-house training session to discuss the Operations and Maintenance Plan, the Long-Term Pollution Prevention Plan, and the Spill Prevention Plan and response procedures. Annual training will include the following:

Discuss the Operations and Maintenance Plan

- Explain the general operations of the stormwater management system and its BMPs
- Identify potential sources of stormwater pollution and measures / methods of reducing or eliminating that pollution
- Emphasize good housekeeping measures

Discuss the Spill Prevention and Response Procedures

- Explain the process in the event of a spill
- Identify potential sources of spills and procedures for cleanup and /or reporting and notification
- Complete a yearly inventory or Materials Safety Data sheets of all tenants and confirm that no potentially harmful chemicals are in use.

ILLICIT DISCHARGE STATEMENT

Certain types of non-stormwater discharges are allowed under the U.S. Environmental Protection Agency Construction General Permit. These types of discharges will be allowed under the conditions that no pollutants will be allowed to come in contact with the water prior to or after its discharge. The control measures which have been outlined previously in this LTPPP will be strictly followed to ensure that no contamination of these non-storm water discharges takes place. Any existing illicit discharges, if discovered during the course of the work, will be reported to MassDEP and the local DPW, as applicable, to be addressed in accordance with their respective policies. No illicit discharges will be allowed in conjunction with the proposed improvements.

SPILL PREVENTION AND RESPONSE PROCEDURES (POST CONSTRUCTION)

In order to prevent or minimize the potential for a spill of Hazardous Substances or Oil or come into contact with stormwater, the following steps will be implemented:

1. All Hazardous Substances or Oil (such as pesticides, petroleum products, fertilizers, detergents, acids, paints, paint solvents, cleaning solvents, etc.) will be stored in a secure location, with their lids on, preferably under cover, when not in use.

2. The minimum practical quantity of all such materials will be kept on site.

3. A spill control and containment kit (containing, for example, absorbent materials, acid neutralizing powder, brooms, dust pans, mops, rags, gloves, goggles, plastic and metal trash containers, etc.) will be provided on site.

4. Manufacturer's recommended methods for spill cleanup will be clearly posted and site personnel will be trained regarding these procedures and the location of the information and cleanup supplies.

5. It is the OWNER's responsibility to ensure that all Hazardous Waste on site is disposed of properly by a licensed hazardous material disposal company. The OWNER is responsible for not exceeding Hazardous Waste storage requirements mandated by the EPA or state and local authorities.

In the event of a spill of Hazardous Substances or Oil, the following procedures should be followed:

1. All measures should be taken to contain and abate the spill and to prevent the discharge of the Hazardous Substance or Oil to stormwater or off-site. (The spill area should be kept well ventilated and personnel should wear appropriate protective clothing to prevent injury from contact with the Hazardous Substances.)

2. For spills of less than five (5) gallons of material, proceed with source control and containment, clean-up with absorbent materials or other applicable means unless an imminent hazard or other circumstances dictate that the spill should be treated by a professional emergency response contractor.

3. For spills greater than five (5) gallons of material immediately contact the MADEP at the toll-free 24-hour statewide emergency number: **1-888-304-1133**, the local fire department (**9-1-1**) and an approved emergency response contractor. Provide information on the type of material spilled, the location of the spill, the quantity spilled, and the time of the spill to the emergency response contractor or coordinator, and proceed with prevention, containment and/or clean-up if so desired. (Use the form provided, or similar).

4. If there is a Reportable Quantity (RQ) release, then the National Response Center should be notified immediately at (800) 424-8802; within 14 days a report should be submitted to the EPA regional office describing the release, the date and circumstances of the release and the steps taken to prevent another release. This Pollution Prevention Plan should be updated to reflect any such steps or actions taken and measures to prevent the same from reoccurring.

SPILL PREVENTION CONTROL AND COUNTERMEASURE FORM

Proposed Solar Array – Parcel 1
160 Maple Street
Bellingham, MA

Where a release containing a hazardous substance occurs, the following steps shall be taken by the facility manager and/or supervisor:

1. Immediately notify The Town Fire Department (at **9-1-1**)

2. All measures must be taken to contain and abate the spill and to prevent the discharge of the pollutant(s) to off-site locations, receiving waters, wetlands and/or resource areas.

3. Notify the Town of Franklin Health Department at (508) 520-4905 and the Town of Franklin Conservation Commission at (508) 520-4929.

4. Provide documentation from licensed contractor showing disposal and cleanup procedures were completed as well as details on chemicals that were spilled to the Town Health Department and Conservation Commission.

Date of spill:_____ Time:_____ Reported By:_____

Weather Conditions:_____

Material Spilled	**Location of Spill**	**Approximate Quantity of Spill (in gallons)**	**Agency(s) Notified**	**Date of Notification**

Cause of Spill: _____

_____

Measures Taken to Clean up Spill: _____

_____

Type of equipment: _____ Make: _____ Size: _____

License or S/N: _____

Location and Method of Disposal _____

_____

Procedures, method, and precautions instituted to prevent a similar occurrence from recurring: _____

_____

_____

Additional Contact Numbers:

- DEPARTMENT OF ENVIRONMENTAL PROTECTION (DEP) EMERGENCY PHONE: 1-888-304-1133
- NATIONAL RESPONSE CENTER PHONE: (800) 424-8802
- U.S. ENVIRONMENTAL PROTECTION AGENCY PHONE: (888) 372-7341