Notice of Intent

for 15 Liberty Way, Franklin MA 02038 (Map: 320, Parcel: 4)



DATE: May 25, 2023

ADDRESSED TO:

Municipal Building Franklin Conservation Commission 355 E. Central Street Franklin, MA 02038

PREPARED BY:

Goddard Consulting LLC 291 Main Street, Suite 8 Northborough, MA 01532

PREPARED FOR:

Atlantic Oliver 15 Liberty Way LLC c/o Oliver Street Capital 125 High Street, Suite 220 Boston, MA 02110

May 25, 2023



Municipal Building Franklin Conservation Commission 355 E. Central Street Franklin, MA 02038

Re: Notice of Intent - 15 Liberty Way, Franklin MA (Map: 320, Parcel: 4)

Dear Franklin Conservation Commission,

On behalf of Atlantic Oliver 15 Liberty Way LLC (the applicant), Goddard Consulting, LLC (representative) is hereby submitting this Notice of Intent (NOI) application for a project which consists of the expansion of a parking lot and associated stormwater management at 15 Liberty Way, Franklin MA. This report describes existing conditions, proposed conditions, wetlands present on site, and project compliance with the Franklin Wetlands Protection Bylaw. The project site contains no resource areas jurisdictional under the MA Wetlands Protection Act.

The only wetland resources affected by the proposed work are Isolated Vegetated Wetland (IVW) and associated 100' buffer zone. This NOI application is a filing only under the Town of Franklin's Wetlands Protection Bylaw. The original, one additional hardcopy, two full-sized sets of plans, and seven 11"x17" sets of plans have been submitted for your review, along with a digital submittal to both the Conservation Department and the Department of Public Works. A list of enclosed documents is as follows:

- NOI Application (WPA Form 3)
- Copy of Checks
- Affidavit of Service, Notification to Abutters, Certified Abutters List
- Orthophoto View of Site, Goddard Consulting LLC. 3/31/2023
- Orthophoto View of Site with DEP Wetlands, Goddard Consulting LLC. 3/31/2023
- Orthophoto View of Site with FEMA Flood Zones, Goddard Consulting LLC. 3/31/2023
- Orthophoto View of Site with NRCS Soil Survey, Goddard Consulting LLC. 3/31/2023
- USGS of Site, Goddard Consulting LLC. 3/31/2023
- Simple Wildlife Habitat Evaluation Form
- Stormwater Report (includes Stormwater Management Checklist), Level Design Group, 1/13/2023,
- Local Filing Fee Calculation Worksheet
- Resource Area Impact Summary, Property Access Signature Form & Application Process Signature Form
- Request for Variance
- BVW Determination Data Form
- Site Plans: Liberty Parking Expansion, Level Design Group, 1/13/2023

Sincerely, Goddard Consulting, LLC

hin Frathing

Chris Frattaroli Wetland Scientist

Cc: Daniel Campbell, Level Design Group, 249 South Street, Unit 1, Plainville, MA 02762 Atlantic Oliver 15 Liberty Way LLC, 125 High Street, Suite 220, Boston MA, 02110 MassDEP Central Regional Office, 8 New Bond Street, Worcester, MA 01606



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1.0 EXISTING CONDITIONS

The project site consists of a ± 15 -acre lot located at 15 Liberty Way in Franklin, MA (Map 320, Lot 4) within Franklin Industrial Park. The project site is currently in use as a commercial trucking warehouse. The northern portion of the lot consists of an area of forested upland. Approximately 58% of the parcel's ± 15 -acres is comprised of impervious cover including the 92,490SF warehouse building and paved surfaces.



Figure 1: Orthophoto of locus site.

1.1 <u>RESOURCE AREAS ON SITE</u>

The resource area onsite in proximity to the work area is a small IVW and is jurisdictional under the Franklin Wetlands Protection Bylaw, but not under the MA Wetlands Protection Act. This IVW is lined with riprap and was constructed inside a drainage easement as a stormwater management structure. This retention pond has a stormwater ditch (described in section 1.2 of this report) that leads to it, although presently little to no water reaches it via this conduit. The retention pond drains via a 24" reinforced concrete pipe to a larger underground stormwater management system that continues offsite.



Figure 2: Side-by-side historic aerials from 1995 and 2006. Tree clearing for drainage infrastructure is absent in 1995 imagery but visible in 2006 imagery. The offsite stormwater ditch is indicated in red. The location of the IVW is indicated in yellow.

According to historic aerial imagery available for the area, this stormwater infrastructure was constructed in the late 1990s or early 2000s, around the time the industrial park was being developed. No areas jurisdictional under the MA Wetlands Protection Act are present onsite. Therefore, the construction of this feature did not require filing under the WPA at the time of its construction, and similarly does not now.

This IVW is, however, jurisdictional under the Franklin Wetlands Protection Bylaw, thus necessitating this filing. Vegetative cover of the IVW is dominated by red maple (*Acer rubrum*), eastern cottonwood (*Populus deltoides*), Sitka willow (*Salix sitchensis*), multiflora rose (*Rosa multiflora*), sensitive fern (*Onoclea sensibilis*), common reed (*Phragmites australis*) and purple loosestrife (*Lythrum salicaria*). A soil sample taken from this IVW showed a fine sandy loam with matrix color 10YR6/2 from 0-6". At approximately 6", a restrictive layer of riprap is encountered. This IVW has not been flagged in the field. Its boundary is clearly defined by the rip-rap slope that contains it and its extent is accurately depicted on the attached site plans. A BVW Determination Data form for this area is attached to this submittal.



Figure 3: View of riprap lined IVW from concrete pipe outlet, facing north.



Figure 4: Alternate view of riprap lined IVW.

1.2 OTHER JURISDICTIONAL AREAS ON SITE

Offsite to the north of the parcel, there is an unmaintained stormwater ditch with periodic riprap breakers along its length. This ditch originates at a culvert that conveys stormwater from an adjacent property. If it was maintained in good repair, this feature would convey water to the IVW referced above. However, in its present condition, it does not convey water at all, resulting in stagnant water throughout its course. The Franklin Wetlands Protection Bylaw §181-4 defines a river or stream as "A naturally flowing body of water that empties into any ocean, lake or other river or stream that flows throughout the year." Therefore, it does not meet the definition of river or stream as defined in the Bylaw and is considered an IVW. This feature casts a jurisdictional buffer zone onto the project site.



Figure 5: Origin of offsite stormwater ditch.



Figure 6: Unmaintained riprap breaker interrupting stormwater ditch.

According to the MassGIS data layers for NHESP, this site is not within Estimated and/or Priority Habitat of Rare Wildlife. There are no potential or certified vernal pools located onsite. The site is not located in an Area of Critical Environmental Concern (ACEC) or an Outstanding Resource Waters Area (ORW). There is no FEMA Flood Zone mapped onsite. There is no DEP mapped wetland onsite.

1.3 NON-JURISDICTIONAL AREAS ON SITE

There is another manmade drainage ditch that runs through the work area, ultimately connecting to the IVW described in section 1.1 of this report. This ditch shows evidence of hand-dug spoils along its banks, discharges from a neighboring parking lot, and does not support a dominance of wetland vegetation. In fact, upland plant species including hay-scented fern (*Dennstaedtia punctilobula*) and Canada mayflower (*Maianthemum canadensis*) are present in this area. Therefore, this ditch is not considered a wetland resource and is not jurisdictional.



Figure 7: Non-jurisdictional drainage ditch in upland area with hay-scented fern and Canada mayflower visible.

2.0 PROPOSED PROJECT

The applicant is proposing to expand the existing parking lot towards the parcel's northern property boundary to provide necessary parking for the building – an increase of approximately 105,320SF. The expanded parking lot will provide stormwater management with an engineered drainage system leading to a Stormtech SC-740 subsurface stormwater collection chamber.



Figure 8: Proposed work area highlighted in yellow.

2.1 EROSION AND SEDIMENTATION CONTROLS

Erosion and Sedimentation (E&S) controls for the site will be utilized so that discharge of sediment is minimized. Implementing these practices limit the amount of storm water entering a disturbed area or trap sediment prior to stormwater leaving the site. Control methods include a 12" Siltsoxx wattle to be installed at the downhill limit of work, and erosion control fencing to be installed around the area to be excavated for the stormwater collection chamber. These E&S controls will minimize sediment migration downslope of the construction area. In addition, Dandy Bags or equivalent will be utilized on existing catch basins to capture sediment before it enters the storm drain. Further, any stockpiled materials will be located outside all jurisdictional areas, covered and stabilized to minimize contact with rain and wind, and have a perimeter sediment barrier.

E&S controls will be maintained weekly and after significant rain events throughout the duration of the construction, including the removal of accumulated sediment once accumulation reaches half the height of the wattle. All E&S controls will remain in place until the site is stabilized. Please see attached site plans for schematic details of E&S controls. The applicant will be responsible for inspecting and maintaining E&S controls and will keep a log of inspections and maintenance.

A portion of the construction activity will require breaching of E&S controls in order to place and backfill underground pipes. This temporary removal of E&S controls will occur only in non-storm events. All E&S controls will be left intact to the greatest extent possible, and E&S controls that need to be removed for such activities will be replaced appropriately before any rain event is forecast to begin.

2.2 STORMWATER MANAGEMENT

The project proposes the use of a Stormtech SC-740 subsurface detention basin to manage stormwater from the proposed parking lot expansion. All runoff from this impervious area will be directed to the subsurface detention basin via deep sump catch basins and trench drains. Overflow from this subsurface detention basin will discharge to the existing riprap lined IVW via HDPE pipe to mimic existing drainage patterns. Additional riprap supplementing the existing riprap lining of the IVW will be placed below the outlet of the two overflow pipes to minimize erosion potential. These two areas total 264SF of IVW alteration. Presently, the areas of riprap addition are almost completely free of woody growth and are primarily covered by oriental bittersweet (*Celastrus orbiculatus*) and common reed (*Phragmites australis*). See the attached Stormwater Report by Level Design Group LLC. dated January 13, 2023 for more information on the stormwater management system and its compliance.

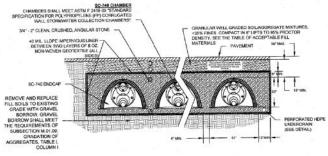


Figure 9: Schematic of Stormtech SC-740 system.



Figure 10: Two photos showing approximate location of proposed supplemental riprap in yellow.

2.3 MITIGATION FOR ALTERATIONS

The project proposes impacts to wetland resources and buffer zones in the form of riprap placement within the resource area, and installation of pavement within the 25-foot buffer zone. Total encroachment into the 25-foot buffer zone is approximately 5,000SF. As mitigation for this encroachment, the applicant proposes the management of invasive plant species in the vicinity of the IVW described in section 1.1 of this report. The area of proposed invasive species management is approximately 11,000SF. Invasive species present in this area include oriental bittersweet (*Celastrus orbiculatus*), multiflora rose (*Rosa multiflora*), purple loosestrife (*Lythrum salicaria*), and common reed (*Phragmites australis*). Please see attached request for variance for alternatives analysis regarding encroachment into resource areas and buffer zones.



Figure 11: Orthophoto of site showing invasive species management area.

2.4 CONSTRUCTION SEQUENCE

The sequence of construction activities will be as follows:

- 1. Install erosion and sediment controls.
- 2. Clear, grub and rough grade areas shown to be regraded.
- 3. Demo existing features in work area not shown to be saved and cut and cap utilities as needed.
- 4. Install stormwater management system and site utilities.
- 5. Install binder coat paving.
- 6. Install landscaping.
- 7. Fine grade site and loam and seed all disturbed areas.
- 8. Install topcoat pavement.
- 9. Project closeout.

3.0 REGULATORY COMPLIANCE WITH FRANKLIN'S WETLANDS PROTECTION BYLAW

3.1 FUNCTIONS AND CHARACTERISTICS STATEMENT

In accordance with §7.10.1 of the Town of Franklin Conservation Commission Regulations, please find below a summary of the proposed project's impacts on the functions and characteristics of floodplains and wetlands. It is Goddard's opinion that the project will have no significant individual or cumulative adverse effects on these functions and characteristics.

1. **Public Water Supplies** – The nearest public well (GP Well 10) is approximately 4,400 feet away from proposed construction activities, and nearly 2,000 feet from the nearest Zone II (primary aquifer recharge area).

2. **Private Water Supplies** – According to MassDEP's Well Drilling Database, the nearest private well that could be identified is located at 760 King Street, approximately 1,700 feet away from the work area. This distance is significantly greater than any setbacks for wells identified in MassDEP's Private Well Guidelines.

3. **Groundwater** – Groundwater onsite is reached at approximately 2.5 to 3 feet of depth, as determined with test pits. The subsurface detention basin proposed for installation was designed to minimize its depth to 2 feet to ensure proper treatment of stormwater runoff.

4. Flood Control – No work of any kind is proposed in any FEMA Flood Zones. Compensatory storage is not required to be addressed.

5. Erosion and Sedimentation – Erosion and sedimentation controls including Siltsoxx wattles, silt fencing and stone construction entrance will be in place and maintained in good condition throughout construction activities. The proposed stormwater system includes catch basins with separators, Stormceptor treatment units and sediment baffles to ensure proper treatment of stormwater that may carry sediment after construction is completed. Please see sections 2.2 and 2.3 of this report, and the attached Stormwater Report by Level Design Group, LLC, dated January 13, 2023 for further details.

6. **Storm Damage Prevention** – The proposed stormwater management system has been designed to handle the 2-, 10-, 25- and 100-year return periods for Franklin, as well as a 24-hour type III rainfall distribution. Further, nearly all sheet flow stormwater runoff presently flows to the IVW onsite, as it was designed as a stormwater management feature (see Existing Drainage Plan in attached Stormwater Report by Level Design Group, LLC, dated January 13, 2023). Therefore, the proposed stormwater management system will not result in a significant increase in stormwater reaching this feature. In addition, the system will attenuate peak rates of runoff, resulting in less damaging stormwater runoff for the site and surrounding areas.

7. Water Quality – There are no surface waters associated with the IVW onsite. The proposed stormwater management system primarily directs runoff to a subsurface detention basin and has been designed to ensure there are no detrimental water quality impacts to any surface waters in the area.

8. Water Pollution Control – During construction, erosion and sediment controls as described above will minimize any potential water pollution. The proposed stormwater management system has been designed to separate sediment and chemical pollutants such as oil and gas from the water it receives, which will ensure that such chemical pollutants are satisfactorily removed. No biological pollution (e.g. bacteria or viruses) is expected to occur as a result of construction activities or post-construction use of the site.

9. Fisheries - No work is proposed adjacent to ponds, perennial streams, or any other fishery habitat.

10. Shellfish - Not applicable in Franklin.

11. Wildlife Habitat – The natural community present on the site, as described in MassWildlife's Classification of the Natural Communities of Massachusetts, is White Pine – Oak Forest. This type of natural community is assigned an SRank of S5, which means it is "demonstrably secure in Massachusetts" and is "common, widespread, and abundant."

According to MassWildlife's BioMap, the work area is not considered either Core Habitat (areas that are critical for the long-term persistence of rare species, exemplary natural communities, and resilient ecosystems) or Critical Natural Landscape (large landscape blocks that are minimally impacted by development and buffers to core habitats and coastal areas). The Index of Ecological Integrity (IEI) provided by UMass Amherst, which quantifies wildlife habitat and biodiversity value, identifies the area as having 0% ecological integrity.

Although not required for this filing, a site visit was conducted for the purpose of completing a Simplified Wildlife Habitat Evaluation as described in the MA Wildlife Habitat Protection Guidance for Inland Wetlands. The site and proposed project meet the criteria for zero (0) of the twenty-three (23) items on this evaluation (see attached).

12. **Rare Species Habitat (including rare plant species)** – No rare species are mapped on or near the project site. The nearest Estimated Habitat of Rare Wildlife or Priority Habitat of Rare Species is approximately 2.5 miles from the site. There are no mapped potential or certified vernal pools onsite or on any abutting parcels.

13. Agriculture – The project site is not located in proximity to any agricultural operations. In addition, the soil unit mapped on site (Woodbridge fine sandy loam, 0 to 8 percent slopes, extremely stony - 312B) is considered not prime farmland by the NRCS Soil Survey.

14. Aquaculture – There are no aquaculture operations on or near the project site.

15. **Recreation** – Neither active nor passive recreational uses will be impacted by construction activities or postconstruction conditions on the site. The work area does not contain any playing fields, walking paths, swimming areas, or other recreational areas. Its position in an industrial park makes it a poor location for recreation of any kind.

4.0 CONCLUSION

In summary, Goddard Consulting believes that the proposed project will not have any adverse impacts on the interests protected by the Town of Franklin's Wetlands Protection Bylaw. The proposed project meets all regulatory compliance standards identified therein and has proposed mitigation for its impacts. Goddard Consulting respectfully requests that the Franklin Conservation Commission issue an Order of Conditions approving the proposed project.

Please feel free to contact us if you have any questions about this Notice of Intent submission.

Sincerely, Goddard Consulting LLC

Chris Frattaroli *Wetland Scientist*



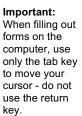
Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Provided by MassDEP:

WPA Form 3 – Notice of Intent

MassDEP File Number

Franklin Wetlands Protection Bylaw

Document Transaction Number Franklin





Note: Before completing this form consult your local Conservation Commission regarding any municipal bylaw or ordinance.

		5 C	City/Town
Α.	General Information		
۱.	Project Location (Note: electronic filers	will click on button to locate project	site):
	15 Liberty Way	Franklin	02038
	a. Street Address	b. City/Town	c. Zip Code
	Latituda and Langituday	42.05651470143494	-71.404532667272
	Latitude and Longitude:	d. Latitude	e. Longitude
	320	4	
	f. Assessors Map/Plat Number	g. Parcel /Lot Number	
2.	Applicant:		
	John	Adair	
	a. First Name	b. Last Name	
	Atlantic Oliver 15 Liberty Way LLC		
	c. Organization		
	125 High St. Suite 220		
	d. Street Address		
	Boston	MA	02110
	e. City/Town	f. State	g. Zip Code
		1. 01010	
	571-216-2878	_adair@oliverst.com	
3.	571-216-2878 h. Phone Number i. Fax Number Property owner (required if different from	adair@oliverst.com j. Email Address	than one owner
3.	571-216-2878 h. Phone Number i. Fax Number	adair@oliverst.com j. Email Address	than one owner
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3.	571-216-2878 h. Phone Number i. Fax Number Property owner (required if different from a. First Name c. Organization d. Street Address e. City/Town h. Phone Number i. Fax Number Representative (if any): Scott a. First Name Goddard Consulting LLC c. Company 291 Main St	adair@oliverst.com j. Email Address n applicant): Check if more b. Last Name f. State j. Email address Goddard	
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a. Total Fee Paid	b. State Fee Paid	C.

(\$600 bylaw) City/Town Fee Paid



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands

WPA Form 3 – Notice of Intent

MassDEP File Number Document Transaction Number Franklin City/Town

Franklin Wetlands Protection Bylaw

A. General Information (continued)

6. General Project Description:

Expansion of parking lot associated with warehouse building within Buffer Zone to IVW.

7a. Project Type Checklist: (Limited Project Types see Section A. 7b.)

1.	Single Family Home	2. Residential Subdivision
3.	Commercial/Industrial	4. Dock/Pier
5.	Utilities	6. Coastal engineering Structure
7.	Agriculture (e.g., cranberries, forestry)	8. Transportation
9.	Other	

7b. Is any portion of the proposed activity eligible to be treated as a limited project (including Ecological Restoration Limited Project) subject to 310 CMR 10.24 (coastal) or 310 CMR 10.53 (inland)?

1. Yes	If yes, describe which limited project applies to this project. (See 310 CMR
	10.24 and 10.53 for a complete list and description of limited project types)

2. Limited Project Type

If the proposed activity is eligible to be treated as an Ecological Restoration Limited Project (310 CMR10.24(8), 310 CMR 10.53(4)), complete and attach Appendix A: Ecological Restoration Limited Project Checklist and Signed Certification.

8. Property recorded at the Registry of Deeds for:

Norfolk	
a. County	b. Certificate # (if registered land)
39279	278
c. Book	d. Page Number

B. Buffer Zone & Resource Area Impacts (temporary & permanent)

- 1. Buffer Zone Only Check if the project is located only in the Buffer Zone of a Bordering Vegetated Wetland, Inland Bank, or Coastal Resource Area.
- 2. Inland Resource Areas (see 310 CMR 10.54-10.58; if not applicable, go to Section B.3, Coastal Resource Areas).

Check all that apply below. Attach narrative and any supporting documentation describing how the project will meet all performance standards for each of the resource areas altered, including standards requiring consideration of alternative project design or location.



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Provided by MassDEP:

WPA Form 3 – Notice of Intent

Franklin Wetlands Protection Bylaw

MassDEP File Number

Document Transaction Number Franklin City/Town

B. Buffer Zone & Resource Area Impacts (temporary & permanent) (cont'd)

	<u>Resour</u>	<u>ce Area</u>	Size of Proposed Alteration	Proposed Replacement (if any)
For all projects	a. 🗌	Bank	1. linear feet	2. linear feet
affecting other Resource Areas,	b. 🗌	Bordering Vegetated Wetland	1. square feet	2. square feet
please attach a narrative explaining how	c. 🗌	Land Under Waterbodies and	1. square feet	2. square feet
the resource area was delineated.		Waterways	3. cubic yards dredged	
	<u>Resour</u>	<u>ce Area</u>	Size of Proposed Alteration	Proposed Replacement (if any)
	d. 🗌	Bordering Land Subject to Flooding	1. square feet	2. square feet
		Cubject to Flooding		
			3. cubic feet of flood storage lost	4. cubic feet replaced
	e. 🗌	Isolated Land		
		Subject to Flooding	1. square feet	
			2. cubic feet of flood storage lost	3. cubic feet replaced
	f.	Riverfront Area	1. Name of Waterway (if available) - sp	ecify coastal or inland
	2.	Width of Riverfront Area	a (check one):	
		25 ft Designated I	Densely Developed Areas only	
		100 ft New agricu	Itural projects only	
		200 ft All other pro	ojects	
	3.	Total area of Riverfront A	rea on the site of the proposed proj	ect: square feet
		Dranged alteration of the		
	4.	Proposed alteration of the	Rivemont Area.	
	a.1	total square feet	b. square feet within 100 ft.	c. square feet between 100 ft. and 200 ft.
	5.	Has an alternatives analy	sis been done and is it attached to	this NOI?
	6.	Was the lot where the act	ivity is proposed created prior to Au	igust 1, 1996? □ Yes □ No
	3. 🗌 Coa	astal Resource Areas: (Se	ee 310 CMR 10.25-10.35)	
	Note:	for coastal riverfront areas	s, please complete Section B.2.f. a	bove.
	⊠F	ranklin Bylaw Is	olated Vegetated Wet	land 264 SF



Massachusetts Department of Environmental ProtectionProvided by MassDEP:Bureau of Resource Protection - WetlandsMassDEP File NumberWPA Form 3 – Notice of IntentDocument Transaction

Franklin Wetlands Protection Bylaw

Document Transaction Number Franklin City/Town

B. Buffer Zone & Resource Area Impacts (temporary & permanent) (cont'd)

Check all that apply below. Attach narrative and supporting documentation describing how the project will meet all performance standards for each of the resource areas altered, including standards requiring consideration of alternative project design or location.

Online Users: Include your document		Resource Area		Size of Proposed	d Alteration	Proposed Replacement (if any)
transaction number		a. 🗌	Designated Port Areas	Indicate size ur	nder Land Under	the Ocean, below
(provided on your receipt page) with all		b. 🗌	Land Under the Ocean	1. square feet		
supplementary information you submit to the				2. cubic yards dredge	ed	
Department.		c. 🗌	Barrier Beach	Indicate size und	ler Coastal Beac	ches and/or Coastal Dunes below
		d. 🗌	Coastal Beaches	1. square feet	<u> </u>	2. cubic yards beach nourishment
		e. 🗌	Coastal Dunes	1. square feet		2. cubic yards dune nourishment
				Size of Proposed	d Alteration	Proposed Replacement (if any)
		f.	Coastal Banks	1. linear feet		
		g. 🗌	Rocky Intertidal Shores	1. square feet		
		h. 🗌	Salt Marshes	1. square feet		2. sq ft restoration, rehab., creation
		i. 🗌	Land Under Salt Ponds	1. square feet		
				2. cubic yards dredge	ed	
		j. 🗌	Land Containing Shellfish	1. square feet		
		k. 🗌	Fish Runs			ks, inland Bank, Land Under the r Waterbodies and Waterways,
				1. cubic yards dredge	ed	
		I. 🗌	Land Subject to Coastal Storm Flowage	1. square feet		
	4.	If the p	storation/Enhancement roject is for the purpose of footage that has been ente	restoring or enhan	•	esource area in addition to the ve, please enter the additional
		a. square	e feet of BVW		b. square feet of S	alt Marsh
	5.	🗌 Pro	pject Involves Stream Cross	sings		
		a. numbe	er of new stream crossings		b. number of repla	cement stream crossings



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands

WPA Form 3 – Notice of Intent

MassDEP File Number

Document Transaction Number

Franklin City/Town

Franklin Wetlands Protection Bylaw

C. Other Applicable Standards and Requirements

This is a proposal for an Ecological Restoration Limited Project. Skip Section C and complete Appendix A: Ecological Restoration Limited Project Checklists – Required Actions (310 CMR 10.11).

Streamlined Massachusetts Endangered Species Act/Wetlands Protection Act Review

 Is any portion of the proposed project located in Estimated Habitat of Rare Wildlife as indicated on the most recent Estimated Habitat Map of State-Listed Rare Wetland Wildlife published by the Natural Heritage and Endangered Species Program (NHESP)? To view habitat maps, see the Massachusetts Natural Heritage Atlas or go to http://maps.massgis.state.ma.us/PRI_EST_HAB/viewer.htm.

a. 🗌 Yes 🛛 No	If yes, include proof of mailing or hand delivery of NOI to:
	Natural Heritage and Endangered Species Program Division of Fisheries and Wildlife
August 2021 b. Date of map	1 Rabbit Hill Road Westborough, MA 01581

If yes, the project is also subject to Massachusetts Endangered Species Act (MESA) review (321 CMR 10.18). To qualify for a streamlined, 30-day, MESA/Wetlands Protection Act review, please complete Section C.1.c, and include requested materials with this Notice of Intent (NOI); *OR* complete Section C.2.f, if applicable. *If MESA supplemental information is not included with the NOI, by completing Section 1 of this form, the NHESP will require a separate MESA filing which may take up to 90 days to review (unless noted exceptions in Section 2 apply, see below).*

- c. Submit Supplemental Information for Endangered Species Review*
 - 1. Dercentage/acreage of property to be altered:
 - (a) within wetland Resource Area

percentage/acreage

(b) outside Resource Area

percentage/acreage

- 2. Assessor's Map or right-of-way plan of site
- 2. Project plans for entire project site, including wetland resource areas and areas outside of wetlands jurisdiction, showing existing and proposed conditions, existing and proposed tree/vegetation clearing line, and clearly demarcated limits of work **
 - (a) Project description (including description of impacts outside of wetland resource area & buffer zone)
 - (b) \square Photographs representative of the site

^{*} Some projects **not** in Estimated Habitat may be located in Priority Habitat, and require NHESP review (see <u>https://www.mass.gov/ma-</u> endangered-species-act-mesa-regulatory-review).

Priority Habitat includes habitat for state-listed plants and strictly upland species not protected by the Wetlands Protection Act.

^{**} MESA projects may not be segmented (321 CMR 10.16). The applicant must disclose full development plans even if such plans are not required as part of the Notice of Intent process.



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands

WPA Form 3 – Notice of Intent

MassDEP File Number Document Transaction Number Franklin City/Town

Franklin Wetlands Protection Bylaw

C. Other Applicable Standards and Requirements (cont'd)

(c) MESA filing fee (fee information available at <u>https://www.mass.gov/how-to/how-to-file-for-a-mesa-project-review</u>).

Make check payable to "Commonwealth of Massachusetts - NHESP" and *mail to NHESP* at above address

Projects altering 10 or more acres of land, also submit:

- (d) Vegetation cover type map of site
- (e) Project plans showing Priority & Estimated Habitat boundaries
- (f) OR Check One of the Following
- 1. Project is exempt from MESA review. Attach applicant letter indicating which MESA exemption applies. (See 321 CMR 10.14, <u>https://www.mass.gov/service-details/exemptions-from-review-for-projectsactivities-in-priority-habitat</u>; the NOI must still be sent to NHESP if the project is within estimated habitat pursuant to 310 CMR 10.37 and 10.59.)

2. 🗌	Separate MESA review ongoing.		
2.	Separate MLSA review ongoing.	a. NHESP Tracking #	b. Date submitted to NHESP

- 3. Separate MESA review completed. Include copy of NHESP "no Take" determination or valid Conservation & Management Permit with approved plan.
- 3. For coastal projects only, is any portion of the proposed project located below the mean high water line or in a fish run?

a. \square Not applicable – project is in inland resource area only	b. 🗌 Yes 📃 No
---	---------------

If yes, include proof of mailing, hand delivery, or electronic delivery of NOI to either:

South Shore - Cohasset to Rhode Island border, and	North Shore - Hull to New Hampshire border:
the Cape & Islands:	

Division of Marine Fisheries -Southeast Marine Fisheries Station Attn: Environmental Reviewer 836 South Rodney French Blvd. New Bedford, MA 02744 Email: <u>dmf.envreview-south@mass.gov</u> Division of Marine Fisheries -North Shore Office Attn: Environmental Reviewer 30 Emerson Avenue Gloucester, MA 01930 Email: dmf.envreview-north@mass.gov

Also if yes, the project may require a Chapter 91 license. For coastal towns in the Northeast Region, please contact MassDEP's Boston Office. For coastal towns in the Southeast Region, please contact MassDEP's Southeast Regional Office.

c	Is this an a	quaculture	project?
υ.	13 1113 111 1	quadantare	

d. 🗌	Yes	\boxtimes	No
a. 🔛	165	\sim	INC

If yes, include a copy of the Division of Marine Fisheries Certification Letter (M.G.L. c. 130, § 57).

		assachusetts Department of Environmental Protection	Provided by MassDEP:			
		reau of Resource Protection - Wetlands	MassDEP File Number			
$\sim \wedge$	V	/PA Form 3 – Notice of Intent	Document Transaction Number			
	F	ranklin Wetlands Protection Bylaw	Franklin			
			City/Town			
	C.	Other Applicable Standards and Requirements	(cont'd)			
	4.	Is any portion of the proposed project within an Area of Critical Environ	nmental Concern (ACEC)?			
Online Users: Include your document		a. Yes No If yes, provide name of ACEC (see instruction Website for ACEC locations). Note: electronic				
transaction number		b. ACEC				
(provided on your receipt page) with all	5.	Is any portion of the proposed project within an area designated as an (ORW) as designated in the Massachusetts Surface Water Quality Sta				
supplementary		a. 🗌 Yes 🖾 No				
information you submit to the Department.	6.	 Is any portion of the site subject to a Wetlands Restriction Order under the Inland Wetlands Restriction Act (M.G.L. c. 131, § 40A) or the Coastal Wetlands Restriction Act (M.G.L. c. 130, § 105)? 				
		a. 🗌 Yes 🖾 No				
	7.	Is this project subject to provisions of the MassDEP Stormwater Mana	gement Standards?			
		 a. Yes. Attach a copy of the Stormwater Report as required by the Standards per 310 CMR 10.05(6)(k)-(q) and check if: 1. Applying for Low Impact Development (LID) site design cr 	-			
		Stormwater Management Handbook Vol. 2, Chapter 3)				
		2. \square A portion of the site constitutes redevelopment				
		3. Proprietary BMPs are included in the Stormwater Manage	ment System.			
		b. No. Check why the project is exempt:				
		1. Single-family house				
		2. Emergency road repair				
		3. Small Residential Subdivision (less than or equal to 4 sing or equal to 4 units in multi-family housing project) with no				
	D.	Additional Information				
		This is a proposal for an Ecological Restoration Limited Project. Skip S Appendix A: Ecological Restoration Notice of Intent – Minimum Requir 10.12).				

Applicants must include the following with this Notice of Intent (NOI). See instructions for details.

Online Users: Attach the document transaction number (provided on your receipt page) for any of the following information you submit to the Department.

- 1. USGS or other map of the area (along with a narrative description, if necessary) containing sufficient information for the Conservation Commission and the Department to locate the site. (Electronic filers may omit this item.)
- 2. Plans identifying the location of proposed activities (including activities proposed to serve as a Bordering Vegetated Wetland [BVW] replication area or other mitigating measure) relative to the boundaries of each affected resource area.



Massachusetts Department of Environmental Protection Provided by MassDEP:

Bureau of Resource Protection - Wetlands

WPA Form 3 – Notice of Intent

MassDEP File Number Document Transaction Number Franklin City/Town

Franklin Wetlands Protection Bylaw

D. Additional Information (cont'd)

- 3. Identify the method for BVW and other resource area boundary delineations (MassDEP BVW Field Data Form(s), Determination of Applicability, Order of Resource Area Delineation, etc.), and attach documentation of the methodology.
- 4. \boxtimes List the titles and dates for all plans and other materials submitted with this NOI.

a. Plan Title		
Level Design Group	Daniel Campbell	
b. Prepared By	c. Signed and Stamped by	
1/13/2023	1"=40'	
d. Final Revision Date	e. Scale	

f. Additional Plan or Document Title

g. Date

- 5. If there is more than one property owner, please attach a list of these property owners not listed on this form.
- 6. Attach proof of mailing for Natural Heritage and Endangered Species Program, if needed.
- 7. Attach proof of mailing for Massachusetts Division of Marine Fisheries, if needed.
- 8. Attach NOI Wetland Fee Transmittal Form
- 9. \square Attach Stormwater Report, if needed.

E. Fees

1. Fee Exempt: No filing fee shall be assessed for projects of any city, town, county, or district of the Commonwealth, federally recognized Indian tribe housing authority, municipal housing authority, or the Massachusetts Bay Transportation Authority.

Applicants must submit the following information (in addition to pages 1 and 2 of the NOI Wetland Fee Transmittal Form) to confirm fee payment:

007581	5/19/2023		
2. Municipal Check Number	3. Check date		
4. State Check Number	5. Check date		
Sarah	Campbell		
6. Payor name on check: First Name	7. Payor name on check: Last Name		



Massachusetts Department of Environmental Protection Problem Bureau of Resource Protection - Wetlands

WPA Form 3 – Notice of Intent

Town of Franklin Wetlands Protection Bylaw

roi	vided by MassDEP:
	MassDEP File Number
	Document Transaction Number
	Franklin
	City/Town

F. Signatures and Submittal Requirements

I hereby certify under the penalties of perjury that the foregoing Notice of Intent and accompanying plans, documents, and supporting data are true and complete to the best of my knowledge. I understand that the Conservation Commission will place notification of this Notice in a local newspaper at the expense of the applicant in accordance with the wetlands regulations, 310 CMR 10.05(5)(a).

I further certify under penalties of perjury that all abutters were notified of this application, pursuant to the requirements of M.G.L. c. 131, § 40. Notice must be made by Certificate of Mailing or in writing by hand delivery or certified mail (return receipt requested) to all abutters within 100 feet of the property line of the project location.

(A de accent)
1. Signature of Applicant (Representative of Oliver Street Capital)
Same as applicant
3. Signature of Property Owner (if different)
5. Signature of Representative (Scott God and, Goddard Consulting LLC)

5/23/2023 2. Date

4. Date/ 5/23/

For Conservation Commission:

Two copies of the completed Notice of Intent (Form 3), including supporting plans and documents, two copies of the NOI Wetland Fee Transmittal Form, and the city/town fee payment, to the Conservation Commission by certified mail or hand delivery.

For MassDEP:

One copy of the completed Notice of Intent (Form 3), including supporting plans and documents, one copy of the NOI Wetland Fee Transmittal Form, and a **copy** of the state fee payment to the MassDEP Regional Office (see Instructions) by certified mail or hand delivery.

Other:

If the applicant has checked the "yes" box in any part of Section C, Item 3, above, refer to that section and the Instructions for additional submittal requirements.

The original and copies must be sent simultaneously. Failure by the applicant to send copies in a timely manner may result in dismissal of the Notice of Intent.



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands **NOI Wetland Fee Transmittal Form**

Franklin Wetlands Protection Bylaw

important: when
filling out forms
on the computer,
use only the tab
key to move your
cursor - do not
use the return
key.
-

2.

.

A. Applicant Information

1. Location of Project: 15 Liberty Way

a. Street Address	b. City/Town	
(no check to DEP, town check #007581)	(\$600 bylaw only)	
c. Check number	d. Fee amount	
Applicant Mailing Address:		
a. First Name	b. Last Name	
Atlantic Oliver 15 Liberty Way LLC		
c. Organization		
125 High Street Suite 220		
d. Mailing Address		
Boston	MA	02110
e. City/Town	f. State	g. Zip Code
h. Phone Number i. Fax Number Property Owner (if different):	j. Email Address	
a. First Name	b. Last Name	
c. Organization		
d. Mailing Address		
e. City/Town	f. State	g. Zip Code

Franklin

3.	Property (Owner (if	different):
----	------------	---------	----	-------------

c. Organization		
d. Mailing Address		
e. City/Town	f. State	g. Zip Code
	 j. Email Address	

To calculate filing fees, refer to the category fee list and examples in the instructions for filling out WPA Form 3 (Notice of Intent).

B. Fees

Fee should be calculated using the following process & worksheet. Please see Instructions before filling out worksheet.

Step 1/Type of Activity: Describe each type of activity that will occur in wetland resource area and buffer zone.

Step 2/Number of Activities: Identify the number of each type of activity.

Step 3/Individual Activity Fee: Identify each activity fee from the six project categories listed in the instructions.

Step 4/Subtotal Activity Fee: Multiply the number of activities (identified in Step 2) times the fee per category (identified in Step 3) to reach a subtotal fee amount. Note: If any of these activities are in a Riverfront Area in addition to another Resource Area or the Buffer Zone, the fee per activity should be multiplied by 1.5 and then added to the subtotal amount.

Step 5/Total Project Fee: Determine the total project fee by adding the subtotal amounts from Step 4.

Step 6/Fee Payments: To calculate the state share of the fee, divide the total fee in half and subtract \$12.50. To calculate the city/town share of the fee, divide the total fee in half and add \$12.50.



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands NOI Wetland Fee Transmittal Form

Franklin Wetlands Protection Bylaw

B. Fees (continued)

Step 1/Type of Activity	Step 2/Number of Activities	Step 3/Individual Activity Fee	Step 4/Subtotal Activity Fee
Commercial/industrial base fee - see local filing fee worksheet	<u>1</u>	\$600	\$600
	·		
	Step 5/Tc	otal Project Fee:	
	Step 6/I	Fee Payments:	
	Total	Project Fee:	(\$600 bylaw only) a. Total Fee from Step 5
	State share	of filing Fee:	none b. 1/2 Total Fee less \$ 12.50
	City/Town share	of filling Fee:	(\$600 bylaw only) c. 1/2 Total Fee plus \$12.50

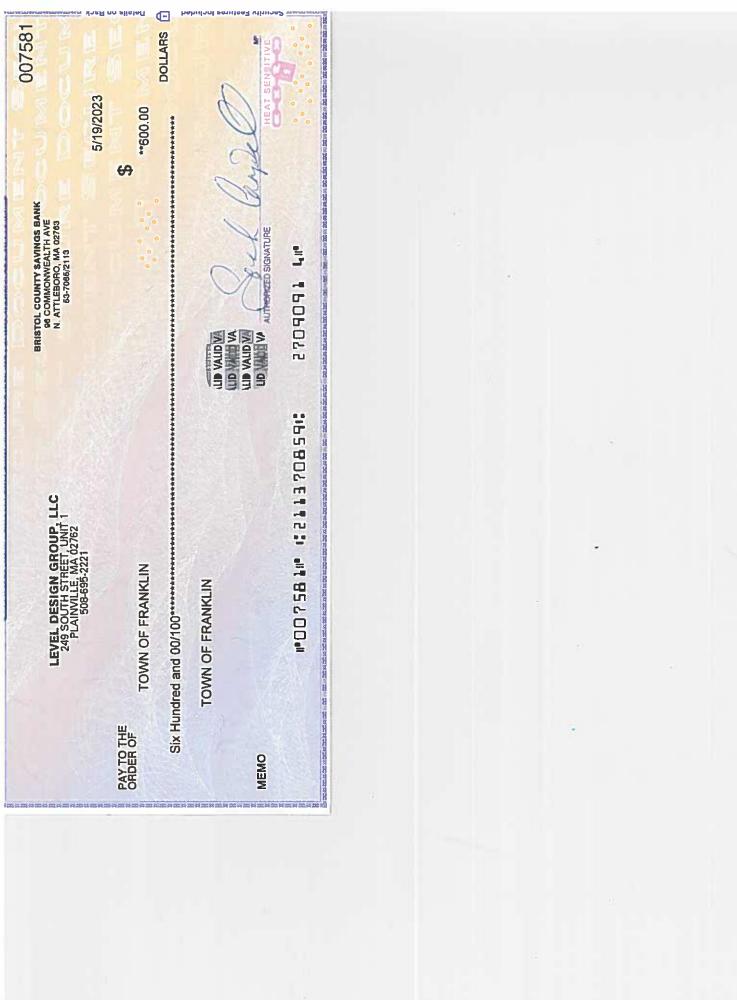
C. Submittal Requirements

a.) Complete pages 1 and 2 and send with a check or money order for the state share of the fee, payable to the Commonwealth of Massachusetts.

Department of Environmental Protection Box 4062 Boston, MA 02211

b.) **To the Conservation Commission:** Send the Notice of Intent or Abbreviated Notice of Intent; a **copy** of this form; and the city/town fee payment.

To MassDEP Regional Office (see Instructions): Send a copy of the Notice of Intent or Abbreviated Notice of Intent; a **copy** of this form; and a **copy** of the state fee payment. (E-filers of Notices of Intent may submit these electronically.)



AFFIDAVIT OF SERVICE

Under the Franklin Wetlands Protection Bylaw:

I, Chris Frattaroli, hereby certify under the pains and penalties of perjury that on May 24, 2023, I gave notification to abutters in Compliance with the second paragraph of Massachusetts General Law Chapter 131, Section 40, the DEP Guide to Abutter Notification dating April 8, 1994, and the Franklin Wetlands Protection Bylaw in connection with the following matter:

A Notice of Intent (NOI) was filed under the Franklin Wetlands Protection Bylaw by Oliver Street Capital with the Franklin Conservation Commission on May 24, 2023 for the property located at 15 Liberty Way, Franklin MA (Map: 320, Parcel: 4)

The form of the notification, and the list of abutters to whom it was given, and their addresses, are attached to this Affidavit of Service.

(Name)

(Date)

Chris Frattaroli, Goddard Consulting LLC

5/24/23

Notification to Abutters

By Hand Delivery, Certified Mail (return receipt requested), or Certificates of Mailing

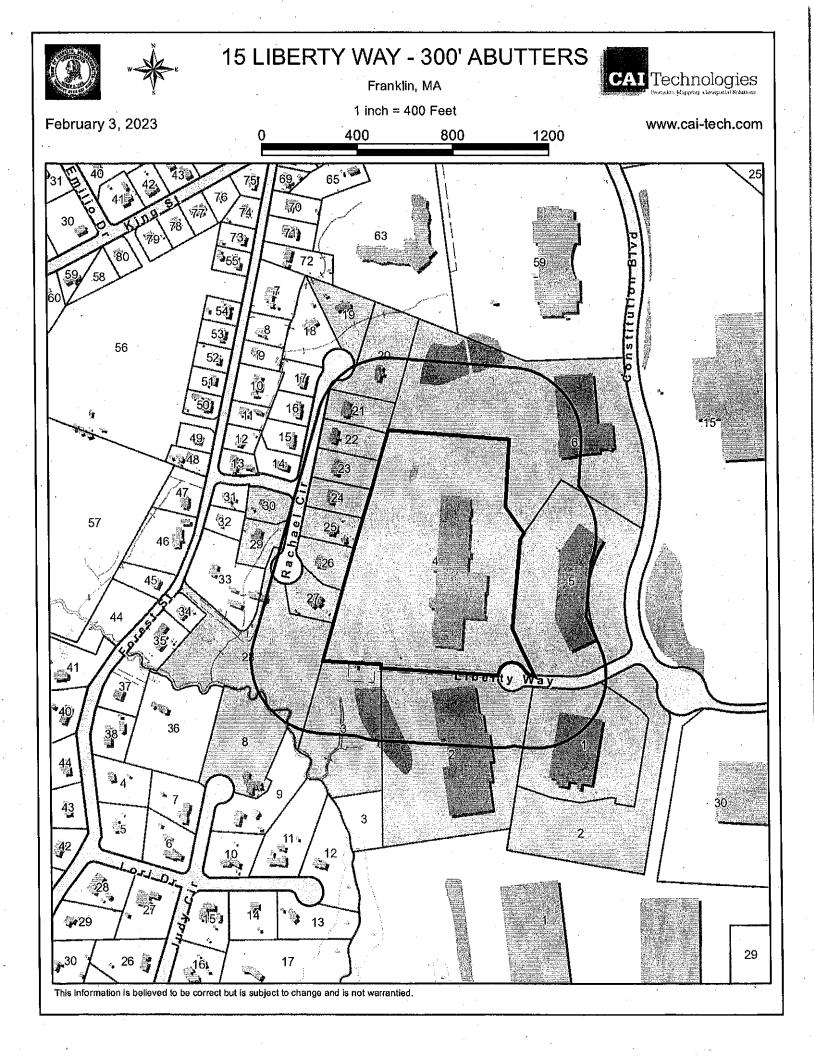
This is a notification required by law. You are receiving this notification because you have been identified as the owner of land abutting another parcel of land for which certain activities are proposed. Those activities require a permit under the Franklin Wetlands Protection Bylaw.

In accordance with the Franklin Wetlands Protection Bylaw (§181-6), you are hereby notified that:

A. A Notice of Intent was filed with the Franklin Conservation Commission on May 24, 2023, seeking permission to remove, fill, dredge, or alter an area subject to protection under the Franklin Wetlands Protection Bylaw. The following is a description of the proposed activity/activities:

Expansion of an existing parking lot and stormwater management system associated with a warehouse building within a locally jurisdictional Isolated Vegetated Wetland and its buffer zone.

- B. The name of the applicant is: Atlantic Oliver 15 Liberty Way LLC
- C. The address of the land where the activity is proposed is: 15 Liberty Way, Franklin MA (Map 320, Parcel 4)
- D. Copies of the Notice of Intent may be examined or obtained at the office of the Franklin Conservation Commission, located at 355 E. Central Street, Franklin, MA. The regular business hours of the Commission are Monday-Thursday, 8am-4pm and Friday, 8am-1pm, and the Commission may be reached at (508) 520-4929.
- E. Copies of the Notice of Intent may be obtained from the applicant or their representative by calling Goddard Consulting, LLC at (508) 393-3784. An administrative fee may be applied for providing copies of the NOI and plans.
- F. Information regarding the date, time, and location of the public hearing regarding the Notice of Intent may be obtained from the Franklin Conservation Commission. Notice of the public hearing will be published at least five business days in advance, in the Franklin Observer.



300 foot Abutters List Report Franklin, MA February 03, 2023

Subject Property:

Parcel Number: CAMA Number: Property Address:	320-004-000 320-004-000-000 15 LIBERTY WAY	Mailing Address:	ATLANTIC OLIVER 15 LIBERTY WAY LLC C/O OLIVER STREET CAPITAL LLC 125 HIGH ST - SUITE 220 BOSTON, MA 02110
Abutters:			
Parcel Number: CAMA Number: Property Address:	320-001-000 320-001-000-000 10 LIBERTY WAY	Mailing Address:	LRF2 BOS LIBERTY WAY LLC C/O LONGPOINT REALTY PARTNERS 13218 WEST BROWARD BOULEVARD PLANTATION, FL 33325
Parcel Number: CAMA Number: Property Address:	320-002-000 320-002-000-000 20 LIBERTY WAY	Mailing Address:	STANNAH PROPERTY LLC C/O LINCOLN PROPERTY COMPANY 25 MALL RD SUITE 402 BURLINGTON, MA 01803
Parcel Number: CAMA Number: Property Address:	320-003-000 320-003-000-000 32 LIBERTY WAY	Mailing Address:	FRANKLIN TOWN OF 355 EAST CENTRAL STREET FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	320-004-000 320-004-000-000 15 LIBERTY WAY	Mailing Address:	ATLANTIC OLIVER 15 LIBERTY WAY LLC C/O OLIVER STREET CAPITAL LLC 125 HIGH ST - SUITE 220 BOSTON, MA 02110
Parcel Number: CAMA Number: Property Address:	320-005-000 320-005-000-000 101 CONSTITUTION BLVD	Mailing Address:	101 CONSTITUTION BLVD LLC C/O KEYPOINT PARTNERS LLC ONE VAN DE GRAAFF DRIVE - SUITE 402 BURLINGTON, MA 01803
Parcel Number: CAMA Number: Property Address:	320-006-000 320-006-000-000 77 CONSTITUTION BLVD	Mailing Address:	IRON MOUNTAIN INFO MANAGEMENT ONE FEDERAL ST BOSTON, MA 02110
Parcel Number: CAMA Number: Property Address:	320-019-000 320-019-000-000 1 RACHAEL CIR	Mailing Address:	MURPHEY ROBERT W MURPHEY JANICE L 1 RACHAEL CIR FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	320-020-000 320-020-000-000 2 RACHAEL CIR	Mailing Address:	PECORA BRIAN F & INNA TRS PECORA LIVING TRUST 2 RACHAEL CIR FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	320-021-000 320-021-000-000 4 RACHAEL CIR	Mailing Address:	COOKE ROBERT D COOKE PATRICIA E 4 RACHAEL CIR FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	320-022-000 320-022-000-000 6 RACHAEL CIR	Mailing Address:	HARDING GRANT E MURPHY MAURA B 6 RACHAEL CIR FRANKLIN, MA 02038

CAI Technologies

www.cai-tech.com This information is believed to be correct but is subject to change and is not warrantied.

Abutters List Report - Franklin, MA

Page 1 of 2

Fran	0 foot Abutters List I klin, MA uary 03, 2023	Report	
Parcel Number: CAMA Number: Property Address:	320-023-000 320-023-000-000 8 RACHAEL CIR	Mailing Address:	SMITH ANDREW J JR SMITH PAULA E 8 RACHAEL CIR FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	320-024-000 320-024-000-000 10 RACHAEL CIR	Mailing Address:	PISANI CHRISTOPHER J PISANI ANNMARIE 10 RACHAEL CIR FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	320-025-000 320-025-000-000 12 RACHAEL CIR	Mailing Address:	CROOK JAMES CORMIER VALERIE 12 RACHAEL CIR FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	320-026-000 320-026-000-000 14 RACHAEL CIR	Mailing Address:	PATIL KEDAR SANJAY MEHTA JANVI DHARMESHKUMAR 14 RACHAEL CIR FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	320-027-000 320-027-000-000 16 RACHAEL CIR	Mailing Address:	GLYNN DEBORAH A 16 RACHAEL CIR FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	320-028-000 320-028-000-000 17 RACHAEL CIR	Mailing Address:	LORUSSO JOSEPH A LORUSSO JESSICA A 17 RACHAEL CIR FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	320-029-000 320-029-000-000 15 RACHAEL CIR	Mailing Address:	MINOR PETER W MINOR NADINE R 15 RACHAEL CIR FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	320-030-000 320-030-000-000 1 LORUSSO DR	Mailing Address:	REBELLO STEPHEN E REBELLO CHRISTINE A 1 LORUSSO DR FRANKLIN, MA 02038
Parcel Number: CAMA Number: Property Address:	329-002-000 329-002-000-000 105 CONSTITUTION BLVD	Mailing Address:	APG FRANKLIN LLC C/O AVENTINE PROPERTY GROUP IN 111 SOUTH WACKER DR SUITE 3350 CHICAGO, IL 60606
Parcel Number: CAMA Number: Property Address:	329-008-000 329-008-000-000 10 JUDY CIR	Mailing Address:	CROMWELL, JILL K TR FLECK, ROBERT A TR ROBERT & JILL FLECK LIV TRUST 10 JUDY CIR FRANKLIN, MA 02038

Junill. Doyle, 2-3-2023

CAI Technologies

www.cai-tech.com This information is believed to be correct but is subject to change and is not warrantied.

Abutters List Report - Franklin, MA

Page 2 of 2

101 CONSTITUTION BLVD LLC C/O KEYPOINT PARTNERS LLC ONE VAN DE GRAAFF DRIVE -SUITE 402 BURLINGTON, MA 01803

APG FRANKLIN LLC C/O AVENTINE PROPERTY GRO 111 SOUTH WACKER DR SUITE 3350 CHICAGO, IL 60606

ATLANTIC OLIVER 15 LIBERT C/O OLIVER STREET CAPITAL 125 HIGH ST - SUITE 220 BOSTON, MA 02110

COOKE ROBERT D COOKE PATRICIA E 4 RACHAEL CIR FRANKLIN, MA 02038

CROMWELL, JILL K TR FLECK ROBERT & JILL FLECK LIV T 10 JUDY CIR FRANKLIN, MA 02038

CROOK JAMES CORMIER VALERIE 12 RACHAEL CIR FRANKLIN, MA 02038

FRANKLIN TOWN OF 355 EAST CENTRAL STREET FRANKLIN, MA 02038

GLYNN DEBORAH A 16 RACHAEL CIR FRANKLIN, MA 02038

HARDING GRANT E MURPHY MAURA B 6 RACHAEL CIR FRANKLIN, MA 02038

IRON MOUNTAIN INFO MANAGE ONE FEDERAL ST BOSTON, MA 02110

LORUSSO JOSEPH A LORUSSO JESSICA A 17 RACHAEL CIR FRANKLIN, MA 02038

LRF2 BOS LIBERTY WAY LLC C/O LONGPOINT REALTY PART 13218 WEST BROWARD BOULEVARD PLANTATION, FL 33325

MINOR PETER W MINOR NADINE R 15 RACHAEL CIR FRANKLIN, MA 02038

MURPHEY ROBERT W MURPHEY JANICE L 1 RACHAEL CIR FRANKLIN, MA 02038

PATIL KEDAR SANJAY MEHTA JANVI DHARMESHKUMAR 14 RACHAEL CIR FRANKLIN, MA 02038

PECORA BRIAN F & INNA TRS PECORA LIVING TRUST 2 RACHAEL CIR FRANKLIN, MA 02038

PISANI CHRISTOPHER J PISANI ANNMARIE 10 RACHAEL CIR FRANKLIN, MA 02038

REBELLO STEPHEN E REBELLO CHRISTINE A 1 LORUSSO DR FRANKLIN, MA 02038

SMITH ANDREW J JR SMITH PAULA E 8 RACHAEL CIR FRANKLIN, MA 02038

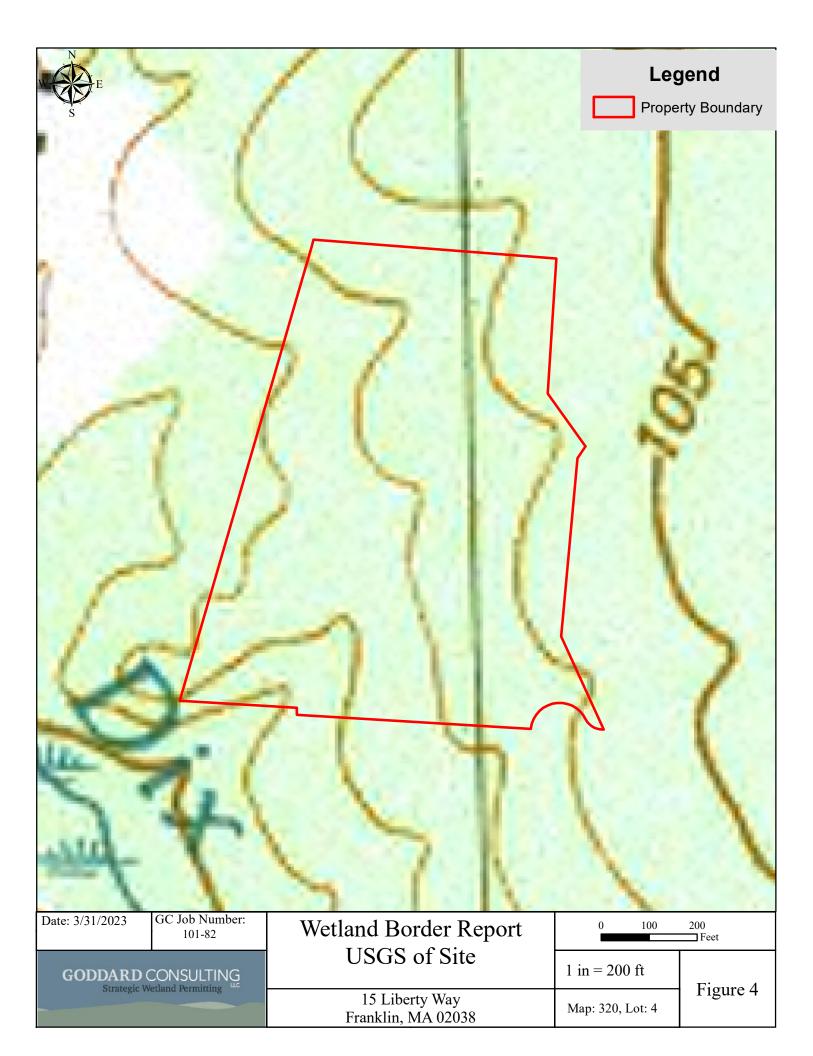
STANNAH PROPERTY LLC C/O LINCOLN PROPERTY COMP 25 MALL RD SUITE 402 BURLINGTON, MA 01803













Bureau of Resource Protection – Wetlands program

Wildlife Habitat Protection Guidance

Appendix A: Simplified Wildlife Habitat Evaluation

Project Information

Important:

When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.

15 Liberty Way, Franklin MA Project Location (from NOI) Chris Frattaroli, Ryan Roseer

Chris Frattaroli, Ryan Roseen

5/22/2023

Date

Name of Person Completing Form

Important Habitat Features

Direct alterations to the following important habitat features in resource areas may be permitted only if they will have no adverse effect (refer to Section V).

- Habitat for state-listed animal species (receipt of a positive opinion or permit from MNHESP shall be presumed to be correct. Do not refer to Section V).
- Sphagnum hummocks and pools suitable to serve as nesting habitat for four-toed salamanders
- Trees with large cavities (\geq 18" tree diameter at cavity entrance)
- Existing beaver, mink or otter dens
- Areas within 100 feet of existing beaver, mink or otter dens (if significant disturbance)
- Existing nest trees for birds that traditionally reuse nests (bald eagle, osprey, great blue heron)
- Land containing freshwater mussel beds
- Wetlands and waterbodies known to contain open water in winter with the capacity to serve as waterfowl winter habitat
- Turtle nesting areas
- Vertical sandy banks (bank swallows, rough-winged swallows or kingfishers)

The following habitat characteristics when not commonly encountered in the surrounding area:

- Stream bed riffle zones (e.g. in eastern MA)
- Springs
- Gravel stream bottoms (trout and salmon nesting substrate)
- Plunge pools (deep holes) in rivers or streams
- Medium to large, flat rock substrates in streams



Wildlife Habitat Protection Guidance

Appendix A: Simplified Wildlife Habitat Evaluation

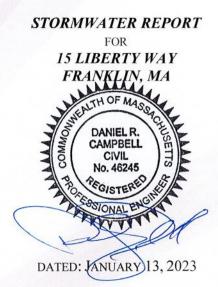
Activities

When any one of the following activities is proposed within resource areas, applicants should complete a Detailed Wildlife Habitat Evaluation (refer to Appendix B).

Activities located in mapped "Habitat of Potential Regional or Statewide Importance"

- Activities affecting certified or documented vernal pool habitat, including habitat within 100' of a certified or documented vernal pool when within a resource area
- Activities in bank, land under water, bordering land subject to flooding (presumed significant) where alterations are more than twice the size of thresholds
- Activities affecting vegetated wetlands >5000 sq. ft. occurring in resource areas other than Bordering Vegetated Wetland
- Activities affecting the sole connector between habitats >50 acres in size
- Installation of structures that prevent animal movement
- Activities for the purpose of bank stabilization using hard structure solutions that significantly affect ability of stream channel to shift and meander, or disrupt continuity in cover that would inhibit animal passage
- Dredging (greater than 5,000 sf)





Prepared By: Level Design Group, L.L.C. 249 South Street, Unit 1 Plainville, MA 02762

Applicant: Oliver Street Capital 125 High Street, Suite 220 Boston, MA 02110 617.279.2428

Property Owner: Oliver Street Capital 125 High Street, Suite 220 Boston, MA 02110

LDG Project No.: 2081.00



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HYDROLOGIC SUMMARY

METHODOLOGY

The HydroCAD computer program (Hydro CAD) was used to model the existing and proposed hydrology of the site and design a stormwater management system. HydroCAD generates flood hydrographs dependent upon the type of land use, vegetation, soil types, land slope, watershed areas and rainfall data. HydroCAD also takes into account the antecedent moisture condition of the soil. The peak rate of runoff and volume of runoff are projected for the input storm frequency events (design storms).

Rainfall data was obtained from the Northeast Regional Climate Center and are based on Extreme Precipitation Events for the 2-, 10-, 25- and 100-year return periods for Franklin, Massachusetts. A 24-hour type III rainfall distribution was used in the HydroCAD analysis as prescribed for New England by the Northeast Regional Climate Center. A copy of the precipitation table is included herein.

PRE-DEVELOPMENT CONDITIONS

The existing site property is located at 15 Liberty Way between Constitution Boulevard and Rachael Circle. The existing property is developed and contains an approximately 95,000 square foot warehouse, paved driveway, and paved parking areas for cars and for trailer trucks. For the purposes of hydrological calculations, the proposed development consists of approximately 105,320 square feet located at the rear of the existing warehouse as shown on the site plans.

The existing topography of the site generally slopes from east to west and directs runoff to an existing stormwater basin located onsite. The southeastern corner of the site slopes south directing runoff to the existing paved driveway and parking area to the site east of the existing warehouse. The land cover is mostly wooded area and also includes pavement from the rear portion of the existing driveway and some grass areas.

Test pits were dug on-site in the areas of the proposed detention basin to verify the groundwater elevation. Groundwater depth in the area of the detention basin varies between approximately 2.5-feet to 3-feet below the surface. The soil logs of the test pits are attached.

The on-site soils as classified by the Soil Survey for Norfolk County Massachusetts are:

312B – Woodbridge fine sandy loam; 0 to 8 percent slopes; Hydrologic Soil Group (HSG) C/D

See the attached SCS soils documentation herein for additional soil details

POST-DEVELOPMENT CONDITIONS

The Applicant is proposing construct a new paved area to provide additional driveway and trailer parking area at the rear of the site. The project will also include the installation of a new stormwater management system and associated grading for this development.



A new fully compliant stormwater management system has been designed to mitigate the impacts of the proposed site redevelopment. Runoff from the impervious areas will be directed to new catch basins and trench drains which will convey runoff to a subsurface detention basin. The subsurface detention basin will provide water quality treatment as well as attenuate peak rates of runoff.

A fully compliant stormwater management system for the entire site addressing compliance with the 10 MADEP Stormwater Standards will be part of the site redevelopment. Site improvements have been made to the maximum extent practicable in accordance with MADEP Stormwater Regulations.

STANDARD 1: Untreated Discharges

Stormwater Management Standard 1 requires that, "No new stormwater conveyances (e.g. outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth".

This standard is met by the proposed redevelopment not creating any new non-treated stormwater discharges and improving an existing residential site with no existing stormwater management system. All surface runoff from proposed impervious areas, with the exception of a redeveloped portion of the existing driveway, is collected in deep sump catch basins and trench drains and directed to a Stormceptor treatment unit prior to discharging to a subsurface detention basin. Overflow from the subsurface detention basin discharges from an outlet control structure to the existing detention basin through a 15" HPDE pipe. All discharges are designed to be placed in areas which mimic existing drainage flow patterns.

Redevelopment: The project has been designed to fully comply with Massachusetts Stormwater Regulations for Standard 1.

Full compliance with Standard 1 is required for new outfalls.

- What BMPs are proposed to ensure that all new discharges associated with the discharge are adequately treated? Proprietary Treatment Units, Subsurface detention basin
- What BMPs are proposed to ensure that no new discharges cause erosion in wetlands or waters of the Commonwealth? rip rap outlets are proposed in the existing stormwater basin designed to prevent erosion.
- Will the proposed discharge comply with all applicable requirements of the Massachusetts Clean Waters Act and the regulations promulgated thereunder at 314 CMR 3.00, 314 CMR 4.00 and 314 CMR 5.00? yes

Existing outfalls shall be brought into compliance with Standard 1 to the maximum extent practicable.

- Are there any existing discharges associated with the redevelopment project for which new treatment could be provided? No existing outfalls
- If so, the proponent shall specify the stormwater BMP retrofit measures that have been considered to ensure that the discharges are adequately treated and indicate the reasons for adopting or rejecting those measures. (See Section entitled "Retrofit of Existing BMPs".) N/A
- What BMPs have been considered to prevent erosion from existing stormwater discharges? N/A



STANDARD 2: Peak Rate Control and Flood Prevention

Stormwater Management Standard 2 requires that, "Stormwater management systems must be designed so that post-development peak discharge rates do not exceed pre-development peak discharge rates. This Standard may be waived for land subject to coastal storm flowage."

This standard is met by the proposed development mitigating the post-development peak discharge rates at the designated control points for all design storm events. This is accomplished by directing stormwater flow to a subsurface detention basin. Below is a description of the design points used in the hydrologic analysis and a summary of pre- and post- development discharge rates. The proposed development will reduce the peak rate of runoff at all the design points and provide ample groundwater recharge.

Redevelopment: The project has been designed to fully comply with Massachusetts Stormwater Regulations for Standard 2.

Compliance to the Maximum Extent Practicable:

- Does the redevelopment design meet Standard 2, comparing post-development to predevelopment conditions? Yes
- If not, the applicant shall document an analysis of alternative approaches for meeting the Standard. (See Menu of Strategies to Reduce Runoff and Peak Flows and/or Increase Recharge Menu included at the end of this chapter.) N/A

Improvement of existing conditions:

- Does the project reduce the volume and/or rate of runoff to less than current estimated conditions? Has the applicant considered all the alternatives for reducing the volume and/or rate of runoff from the site? (See Menu.) Yes
- Is the project located within a watershed subject to damage by flooding during the 2-year or 10year 24-hour storm event? If so, does the project design provide for attenuation of the 2-year and 10-year 24-hour storm event to less than current estimated conditions? Have measures been implemented to reduce the volume of runoff from the site resulting from the 2 year or 10 year 24 hour storm event? (See Menu.) N/A
- Is the project located adjacent to a water body or watercourse subject to adverse impacts from flooding during the 100-year 24-hour storm event? If so, are portions of the site available to increase flood storage adjacent to existing Bordering Land Subject to Flooding (BLSF)? N/A
- Have measures been implemented to attenuate peak rates of discharge during the 100-year 24hour storm event to less than the peak rates under current estimated conditions? Have measures been implemented to reduce the volume of runoff from the site resulting from the 100-year 24hour storm event? (See Menu.) Yes



SUMMARY OF PEAK STORMWATER RUNOFF (CFS)

Three design points were utilized to analyze the runoff characteristics of the site. Design Point 1 (DP-1) is the existing detention basin. Design Point 2 (DP-2) is the existing paved driveway and parking area to the east of the existing warehouse. Design Point 3 (DP-3) is the northwestern corner of the property. The pre- and post- development peak discharge rates for all analyzed design storms is summarized in the following table:

Design Point 1 (DP-1)			
Storm	Pre-Dev. Flow	Post-Dev. Flow	
2-yr	1.49 cfs	1.40 cfs	
10-yr	4.53 cfs	4.29 cfs	
25-yr	6.49 cfs	5.91 cfs	
100-yr	9.57 cfs	8.86 cfs	
	Design Point 2	(DP-2)	
	Pre-Dev. Flow	Post-Dev. Flow	
2-yr	0.41 cfs	0.41 cfs	
10-yr	0.94 cfs	0.70 cfs	
25-yr	1.26 cfs	0.86 cfs	
100-yr	1.73 cfs	1.11 cfs	
	Design Point 3	(DP-3)	
2-yr	0.05 cfs	0.03 cfs	
10-yr	0.13 cfs	0.08 cfs	
25-yr	0.19 cfs	0.10 cfs	
100-yr	0.27 cfs	0.14 cfs	

The net peak discharge for DP-1 is controlled by the subsurface detention basin and does not increase flows off site for any of the evaluated design storms. The net peak discharges for both DP-2 and DP-3 are un-controlled and flow off-site overland as occurs in existing conditions. Based on the proposed grading and the locations of new catch basins and trench drains, there is a reduction in total area contributing to both DP-2 and DP-3 and the proposed development does not increase flows to these design points for any of the evaluated design storms from existing conditions.

STANDARD 3: Recharge to Groundwater

Stormwater Management Standard 3 requires that, "Loss of annual recharge to groundwater shall be eliminated or minimized through the use of infiltration measures, including environmentally sensitive site design, low impact development techniques, best management practices, and good operation and maintenance. At a minimum, the annual recharge from the post-development site shall approximate the annual recharge from the pre-development conditions based on soil type. This Standard is met when the stormwater management system is designed to infiltrate the required recharge volume as determined in accordance with the Massachusetts Stormwater Handbook."



GROUND WATER RECHARGE

The proposed project will result in an increase in impervious area to a total of approximately 80,078 square feet. The on-site soils of the areas contributing to the proposed subsurface detention system as classified by the Soil Survey for Norfolk County Massachusetts are 312B – Woodbridge fine sandy loam 0 to 8 percent slopes with a hydrologic soil group classification of HSG C/D. On-site soil textures are sandy loam based on test pits performed by Level Design Group, LLC. Based on the HSA "C/D" soil classification, the shallow depth to groundwater, and the existing and proposed topography, infiltration systems conforming to Massachusetts Stormwater Regulations are not practical and no infiltration is proposed for the site. Instead, A Subsurface detention system is proposed to be wrapped in linear low density polyethylene liner.

ALTERNATIVE ANALYSIS

Due to site constraints including low permeability soils and shallow groundwater depth, infiltration is not proposed at the site. Porous pavement was considered as a potential alternative to provide infiltration, however, because the pavement will primarily be used by trailer trucks, this option was not practical. Site infiltration measures will be provided through both the proposed rip rap swale and the existing detention basin. No changes are proposed to the existing detention basin however it is being utilized as an outlet for the proposed stormwater improvements for the development. Post development flows to the basin are decreased from pre development conditions.

Redevelopment: The project has been designed comply with Massachusetts Stormwater Regulations for Standard 3 to the Maximum Extent Practicable.

Compliance to the Maximum Extent Practicable:

- Does the redevelopment design meet Standard 3, comparing post-development to predevelopment conditions? No
- If not, the applicant shall document an analysis of alternative approaches for meeting the Standard? See Alternatives Analysis above.
- What soil types are present on the site? Is the site is comprised solely of C and D soils and bedrock at the land surface? Solely of C and D soils
- Does the project include sites where recharge is proposed at or adjacent to an area classified as contaminated, sites where contamination has been capped in place, sites that have an Activity and Use Limitation (AUL) that precludes inducing runoff to the groundwater, pursuant to MGL Chapter 21E and the Massachusetts Contingency Plan 310 CMR 40.0000; sites that are the location of a solid waste landfill as defined in 310 CMR 19.000; or sites where groundwater from the recharge location flows directly toward a solid waste landfill or 21E site?¹ N/A
- Is the stormwater runoff from a land use with a higher potential pollutant load? N/A
- Is the discharge to the ground located within the Zone II or Interim Wellhead Protection Area of a public water supply? No
- Does the site have an infiltration rate greater than 2.4 inches per hour? No

Improvements to Existing Conditions:

¹ A mounding analysis is needed if a site falls within this category. See Volume 3.



- Does the project increase the required recharge volume over existing (developed) conditions? If so, can the project be redesigned to reduce the required recharge volume by decreasing impervious surfaces (make building higher, put parking under the building, narrower roads, sidewalks on only one side of street, etc.) or using low impact development techniques such as porous pavement? The site increases the recharge volume requirement. Porous pavement was considered as an alternative to provide infiltration but is not practical based on it being primarily utilized by trailer trucks.
- Is the project located within a basin or sub-basin that has been categorized as under high or medium stress by the Massachusetts Water Resources Commission, or where there is other evidence that there are rivers and streams experiencing low flow problems? If so, have measures been considered to replace the natural recharge lost as a result of the prior development? (See Menu.) N/A
- Has the applicant evaluated measures for reducing site runoff? (See Menu.) Yes, See Alternatives Analysis above.

STANDARD 4: 80% TSS Removal

Stormwater Management Standard 4 requires that, "Stormwater management systems must be designed to remove 80% of the average annual post-construction of Total Suspended Solids (TSS). This standard is met when:

- a. Suitable practices for source control and pollution prevention are identified in a long-term pollution prevention plan and thereafter are implemented and maintained;
- b. Stormwater BMPs are sized to capture the required water quality volume determined in accordance with the Massachusetts Stormwater Handbook and;
- c. Pretreatment is provided in accordance with the Massachusetts Stormwater Handbook"

To achieve the required 80% TSS removal, new stormwater BMP's will be installed. A subsurface detention basin is proposed which will collect runoff from all impervious areas within Drainage Area P-1a and provide the maximum level or stormwater treatment practicable. A proprietary stormwater treatment unit is proposed which provides treatment prior to discharge to the subsurface detention basin. MADEP TSS Removal Sheets are included herein which show 80% TSS removal by the proprietary stormwater treatment unit and the subsurface detention basin meeting the requirements for total treatment.

Water Quality Calculations:

CDS Stormwater Treatment Unit Sizing

Water quality treatment is provided by using CDS treatment units. The CDS Units are sized using the *Massachusetts Department of Environmental Protection Wetlands Program – Standard Method to Convert Required Water Quality Volume to a Discharge Rate for Sizing Flow Based Manufactured Proprietary Stormwater Treatment Practices.*

The water quality flow calculations are detailed below:

Flow to DHM-2/CDS

 $Q_{1.0}=(qu)(A)(WQV)$



qu=774 csm/in for a Tc of 0.1 hours (taken from Figure 2 of the Massachusetts Department of Environmental Protection Wetlands program - Standard Method to Convert Required Water Quality Volume to a Discharge Rate

A=1.633 Acres WQV=1.0 inches

 $Q_{1.0} = (774 \text{ csm/in}) (1.633 \text{ acres}) (0.0015625 \text{ sq. mi} I \text{ acre}) (1.0 \text{ inch})$

Q1.0 = 1.97 cfs < CDS Model 2020-5 Inlet Unit with a Treatment Capacity =2.20 cfs

Flow to CB-3/STC-450i

$Q_{1.0}=(qu)(A)(WQV)$

qu=774 csm/in for a Tc of 0.1 hours (taken from Figure 2 of the Massachusetts Department of Environmental Protection Wetlands program - Standard Method to Convert Required Water Quality Volume to a Discharge Rate

A=0.071 Acres WOV=1.0 inches

 $Q_{1.0} = (774 \text{ csm/in}) (0.071 \text{ acres}) (0.0015625 \text{ sq. mi} I \text{ acre}) (1.0 \text{ inch})$

Q1.0 = 0.09 cfs < STC-450i with a Treatment Capacity =0.40 cfs

Redevelopment: The project has been designed to fully comply with Massachusetts Stormwater Regulations for Standard 4.

Full compliance for any component that is not a redevelopment Full compliance with the long-term pollution plan requirement for new developments and redevelopments.

- Has the proponent developed a long-term pollution plan that fully meets the requirements of Standard 4? A Long Term Pollution Prevention Plan is provided
- Does the pollution prevention plan include the following source control measures?
 - Street sweeping yes
 - Proper management of snow, salt, sand and other deicing chemicals yes
 - Proper management of fertilizers, herbicides and pesticides yes
 - Stabilization of existing eroding surfaces yes

Compliance to the Maximum Extent Practicable for the other requirements:

- Does the redevelopment design provide for treatment of all runoff from existing (as well as new) impervious areas to achieve 80% TSS removal? If 80% TSS removal is not achieved, has the stormwater management system been designed to remove TSS to the maximum extent practicable? 80% TSS removal achieved
- Have the proposed stormwater BMPs been properly sized to capture the prescribed runoff volume? Yes



- One inch rule applies for discharge
 - within a Zone II or Interim Wellhead Protection Area, N/A
 - near or to another critical area, N/A
 - from a land use with a higher potential pollutant load N/A
 - to the ground where the infiltration rate is greater than 2.4 inches per hour N/A
- Has adequate pretreatment been proposed?
 - 44% TSS Removal Pretreatment Requirement applies if:
 - Stormwater runoff is from a land use with a higher potential pollutant load
 - Stormwater is discharged Yes, 44% TSS removal achieved (Zone II and rapid infiltration)
 - To the ground within the Zone II or Interim Wellhead Protection Area of a Public Water Supply N/A
 - To the ground with an infiltration rate greater than 2.4 inches per hour $N\!/\!A$
 - Near or to an Outstanding Resource Water, Special Resource Water, Cold-Water Fishery, Shellfish Growing Area, or Bathing Beach. -N/A

• If the stormwater BMPs do not meet all the requirements set forth above, the applicant shall document an analysis of alternative approaches for meeting the these requirements. (See Section on Retrofitting Existing BMPs (the "Retrofit Section"). N/A

Improvements to Existing Conditions:

- Have measures been provided to achieve at least partial compliance with the TSS removal standard? Fully complies
- Have any of the best management practices in the Retrofit Section been considered? N/A
- Have any of the following pollution prevention measures been considered? Operation & Maintenance and Long Term Pollution Prevention Plans have been prepared for the site in accordance with Massachusetts Stormwater regulations and are included in the Stormwater Report.
 - Reduction or elimination of winter sanding, where safe and prudent to do so
 - Tighter controls over the application of fertilizers, herbicides, and pesticides
 - o Landscaping that reduces the need for fertilizer, herbicides and pesticides
 - High frequency sweeping of paved surfaces using vacuum sweepers
 - Improved catch basin cleaning
 - Waterfowl control programs

Are there any discharges (new or existing) to impaired waters? If so, see TMDL section. N/A

STANDARD 5: Higher Potential Pollutant Loads

Stormwater Management Standard 5 requires that, "For land uses with higher potential pollutant loads, source control and pollution prevention shall be implemented in accordance with the Massachusetts Stormwater Handbook to eliminate or reduce the discharge of stormwater runoff from such land uses to the maximum extent practicable. If through source control and/or pollution prevention, all land uses with higher potential pollutant loads cannot be completely protected from exposure to rain, snow, snow melt and stormwater runoff, the proponent shall use the specific stormwater BMPs determined by the



Department to be suitable for such use as provided in the Massachusetts Stormwater Handbook. Stormwater discharges from land uses with higher potential pollutant loads shall also comply with the requirements of the Massachusetts Clean Waters Act, M.G.L. c. 2,§26-53, and the regulations promulgated thereunder at 314 CMF 3.00, 314 CMR 4.00 and 314 CMR 5.00."

The proposed use in not considered a use that would generate Higher Potential Pollutant Loads.

Redevelopment: The project use is not considered a use that would generate Higher Potential Pollutant Loads.

STANDARD 6: Critical Areas

Stormwater Management Standard 6 requires that Stormwater discharge to a Zone II Interim Wellhead Protection Area of a public water supply and stormwater discharges near any other critical area require the use of specific source control and pollution prevention measures and the specific stormwater best management practices determined by the Department to be suitable for managing discharges to such area, as provided in the Massachusetts Stormwater Handbook. A discharge near a critical area, if there is a strong likelihood of a significant impact occurring to said area, taking into account site-specific factors. Stormwater discharges to Outstanding Resource Waters or Special Resource Waters shall be set back from the receiving water and receive the highest and best practical method of treatment. A "stormwater discharge," as defined in 314 CMR 3.04(2)(a)1. or (b), to an Outstanding Resource Waters or Special Resource Waters shall comply with 314 CMF 3.00 and 314 CMR 4.00. Stormwater discharges to a Zone I or Zone A area prohibited unless essential to the operation of the public water supply."

The development site is not located within a Critical Area as defined by the Massachusetts Stormwater Handbook.

Redevelopment: The project is not located in a critical area as defined by the Massachusetts Stormwater Regulations for Standard 6.

STANDARD 7: Redevelopment and Other Projects Subject to the Standards only to the Maximum Extent Practicable

The definition of a Redevelopment Project under the definition provided in the MADEP Stormwater Handbook for Standard 7 is listed below:

"Development rehabilitation, expansion and phased projected on previously developed sites, provided that redevelopment results in no next increase in impervious area."

The proposed development is not considered a Redevelopment Project and fully complies with the requirements of the MADEP Stormwater Management Standards.

STANDARD 8: Erosion and Sediment Control

Stormwater Management Standard 8 requires that, "A plan to control construction-related impacts, including erosion sedimentation and other pollutant sources during construction and land disturbance



activities (construction period erosion, sedimentation, and pollution prevention plan), must be developed and implemented."

This standard is met through the type and style of construction. The existing driveway will, to the extent possible, remain intact until the proposed driveway is to be graded and paved. This will provide a pad for wheel cleaning prior to the vehicle exit on Washington Street. The site is self-contained with abutting properties draining overland to the rear of the subject property. Thereby there will be no additional erosion from this property to abutting properties. With careful construction the project will limit erosion potential through the development itself and no additional structural measures, passive or active, are proposed. A Construction Period Pollution and Erosion & Sedimentation Control Plan has also been prepared and is included as part of the Stormwater Report.

Redevelopment: The project has been designed to fully comply with Massachusetts Stormwater Regulations for Standard 8.

All redevelopment projects shall fully comply with Standard 8.

 Has the proponent submitted a construction period erosion, sedimentation and pollution prevention plan that meets the requirements of Standard 8?
 A draft Stormwater Pollution Prevention Plan is included with the stormwater report and will be finalized prior to the start of construction.

STANDARD 9: Operation and Maintenance

Stormwater Management Standard 9 requires that, "A long-term operation and maintenance plan must be developed and implemented to ensure that stormwater management systems function as designed".

This standard is fully met with development and implementation of an Operation and Maintenance Plan is included in Stormwater Management Report.

Redevelopment: The project has been designed to fully comply with Massachusetts Stormwater Regulations for Standard 9.

All redevelopment projects shall fully comply with Standard 9.

 Has the proponent submitted a long-term Operation and Maintenance plan that meets the requirements of Standard 9?
 O&M included in Stormwater Report

STANDARD 10: Illicit Discharges

Stormwater Management Standard 10 requires that, "All illicit discharges to the stormwater management system are prohibited".

This standard is fully met with development and implementation of a Long-Term Pollution Prevention which is included in the Stormwater Management Report. An Illicit Discharge statement has been prepared and is included herein.



Redevelopment: The project has been designed to fully comply with Massachusetts Stormwater Regulations for Standard 10.

All redevelopment projects shall fully comply with Standard 10.

- Are there any known or suspected illicit discharges to the stormwater management system at the redevelopment project site? No
- Has an illicit connection detection program been implemented using visual screening, dye or smoke testing? No
- Have an Illicit Discharge Compliance Statement and associated site map been submitted verifying that there are no illicit discharges to the stormwater management system at the site? Yes

Improvements to Existing Conditions:

• Once all illicit discharges are removed, has the proponent implemented any measures to prevent additional illicit discharges? N/A

CONCLUSION

The proposed redevelopment of this parcel will be a significant improvement to the area and to the resource area on and adjacent to the site. The proposed 2081redevelopment meets or exceeds the current MADEP Stormwater Management Standards and Guidelines and provides a stormwater management system that will maintain water quality while attenuating peak rates of runoff at the control points. This was achieved by using pretreatment BMPs and directing the stormwater runoff to a subsurface detention basin which attenuates peak flows while providing a high level of TSS removal. An Operation and Maintenance Plan for post-construction maintenance of the Stormwater Management System has been developed and is included with this report.



MADEP Stormwater Report Checklist



Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program Checklist for Stormwater Report

A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.



A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals.¹ This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard 8²
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

¹ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.

² For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.



B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

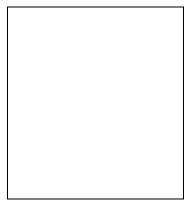
Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Long-term Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature

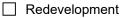


Signature and Date

Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

New development



Mix of New Development and Redevelopment



Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:

\boxtimes	No disturbance to any Wetland Resource Areas
	Site Design Practices (e.g. clustered development, reduced frontage setbacks)
	Reduced Impervious Area (Redevelopment Only)
	Minimizing disturbance to existing trees and shrubs
	LID Site Design Credit Requested:
	Credit 1
	Credit 2
	Credit 3
	Use of "country drainage" versus curb and gutter conveyance and pipe
	Bioretention Cells (includes Rain Gardens)
	Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
	Treebox Filter
	Water Quality Swale
	Grass Channel
	Green Roof
	Other (describe):
.	

Standard 1: No New Untreated Discharges

No new untreated discharges

- Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
- Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.



Checklist (continued)

Standard 2: Peak Rate Attenuation

- Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
- Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.

Calculations provided to show that post-development peak discharge rates do not exceed predevelopment rates for the 2-year and 10-year 24-hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24hour storm.

Standard 3: Recharge

Soil Analysis provided.

- Required Recharge Volume calculation provided.
- Required Recharge volume reduced through use of the LID site Design Credits.
- Sizing the infiltration, BMPs is based on the following method: Check the method used.

Static	Simple Dynamic
--------	----------------

Dynamic Field¹

- Runoff from all impervious areas at the site discharging to the infiltration BMP.
- Runoff from all impervious areas at the site is *not* discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.

Recharge BMPs have been sized to infiltrate the Required Recharge Volume.

\boxtimes	Recharge BMPs have been sized to infiltrate the Required Recharge Volume only to the maximum
	extent practicable for the following reason:

- Site is comprised solely of C and D soils and/or bedrock at the land surface
- M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
- Solid Waste Landfill pursuant to 310 CMR 19.000
- Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.
- Calculations showing that the infiltration BMPs will drain in 72 hours are provided.

	Property includes a M.	G.L. c. 21E site or a s	solid waste landfill a	nd a mounding ana	alysis is included.
--	------------------------	-------------------------	------------------------	-------------------	---------------------

¹ 80% TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.



Checklist (continued)

Standard 3: Recharge (continued)

The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.

Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
- A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
- Treatment BMPs subject to the 44% TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
 - is within the Zone II or Interim Wellhead Protection Area
 - is near or to other critical areas
 - is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
 - involves runoff from land uses with higher potential pollutant loads.
- The Required Water Quality Volume is reduced through use of the LID site Design Credits.
- Calculations documenting that the treatment train meets the 80% TSS removal requirement and, if applicable, the 44% TSS removal pretreatment requirement, are provided.



Checklist	(continued)
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Standard 4: Water Quality (continued)

- The BMP is sized (and calculations provided) based on:
 - ☐ The ½" or 1" Water Quality Volume or
 - The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.
- The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
- A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

- The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
- The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted **prior to** the discharge of stormwater to the post-construction stormwater BMPs.
- The NPDES Multi-Sector General Permit does *not* cover the land use.
- LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.
- All exposure has been eliminated.
- All exposure has *not* been eliminated and all BMPs selected are on MassDEP LUHPPL list.
- The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with >1000 vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

Standard 6: Critical Areas

- The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
- Critical areas and BMPs are identified in the Stormwater Report.



Checklist (continued)

Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

Limited	Project
---------	---------

- Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
- Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
- Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
- Bike Path and/or Foot Path
- Redevelopment Project
- Redevelopment portion of mix of new and redevelopment.
- Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.

☐ The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards 4-6 to the maximum extent practicable and (b) improves existing conditions.

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.



Checklist (continued)

Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

- ☐ The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has *not* been included in the Stormwater Report but will be submitted *before* land disturbance begins.
- The project is *not* covered by a NPDES Construction General Permit.
- The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
- The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

Standard 9: Operation and Maintenance Plan

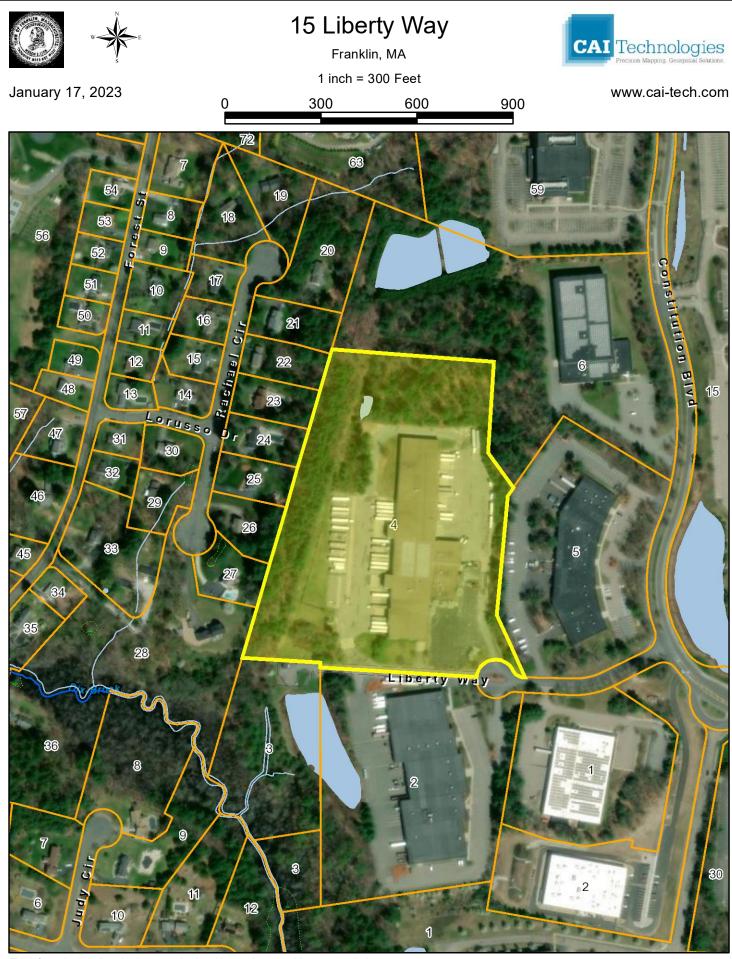
- The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
 - Name of the stormwater management system owners;
 - Party responsible for operation and maintenance;
 - Schedule for implementation of routine and non-routine maintenance tasks;
 - Plan showing the location of all stormwater BMPs maintenance access areas;
 - Description and delineation of public safety features;
 - Estimated operation and maintenance budget; and
 - Operation and Maintenance Log Form.
- The responsible party is *not* the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
 - A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
 - A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

Standard 10: Prohibition of Illicit Discharges

- The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
- An Illicit Discharge Compliance Statement is attached;
- NO Illicit Discharge Compliance Statement is attached but will be submitted *prior to* the discharge of any stormwater to post-construction BMPs.



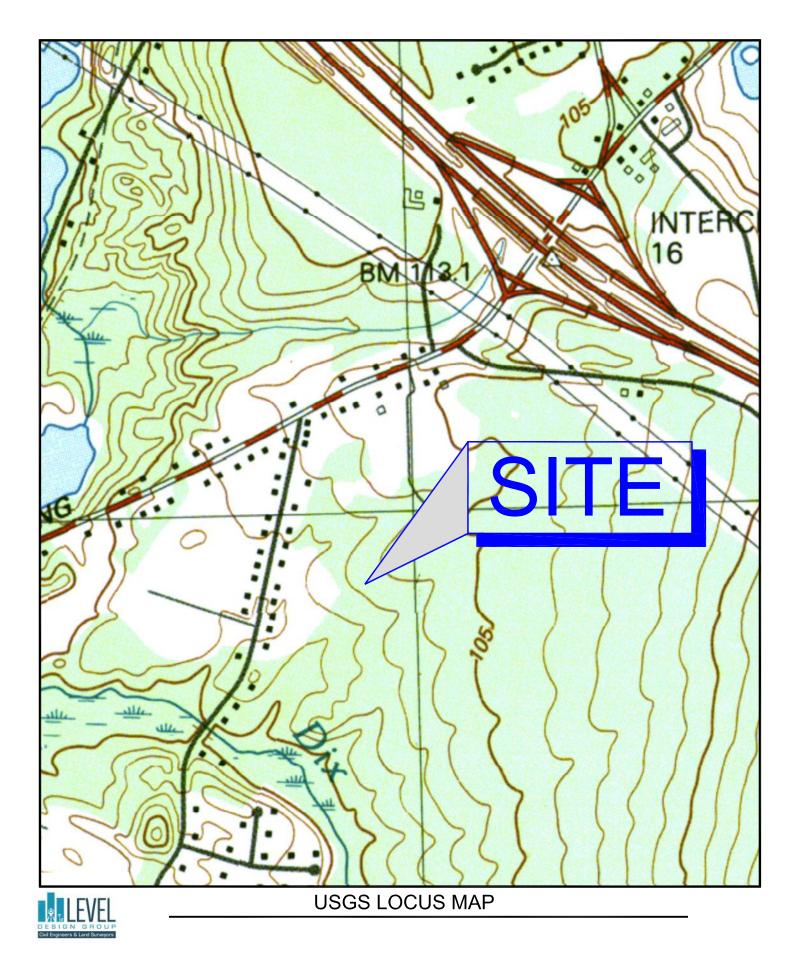
Aerial Photograph (MAGIS)



This information is believed to be correct but is subject to change and is not warrantied.



USGS Topographic Map (MAGIS)



CIVIL ENGINEERS AND LAND SURVEYORS 249 SOUTH STREET, UNIT 1 PLAINVILLE, MA 02762 508.695.2221 (F) 508.695.2219 WWW.LEVELDG.COM

15 LIBERTY WAY FRANKLIN, MA



On-Site Soils Documentation

Project No:	2081.00	Soil Evaluator	Adam Hunt E.I.T.	SE# 12794
Project:	15 Liberty Way Franklin	Temp	48F Rain	
Date of Testing	11/16/22			

Test Hole No.:	TH-1	Ground Elevation at Hole = 334.70
Time:	10:15 AM	Groundwater Elevation = 331.78

Depth (inches)	Horizon	Color	Texture	Redox Depth	Redox Color	Comments
0-10	А	10YR 2/2	SL			
10-24	В	10YR 5/6	SL			
24-108	C1	2.5Y 6/3	SL	35"		Very Compact
Weeping Observe	ed					
Standing Water C	bserved					
Redox Observed				35"		

Test Hole No.:	TH-2	Ground Elevation at Hole = 331.05
Time:	11:00 AM	Groundwater Elevation = 328.38

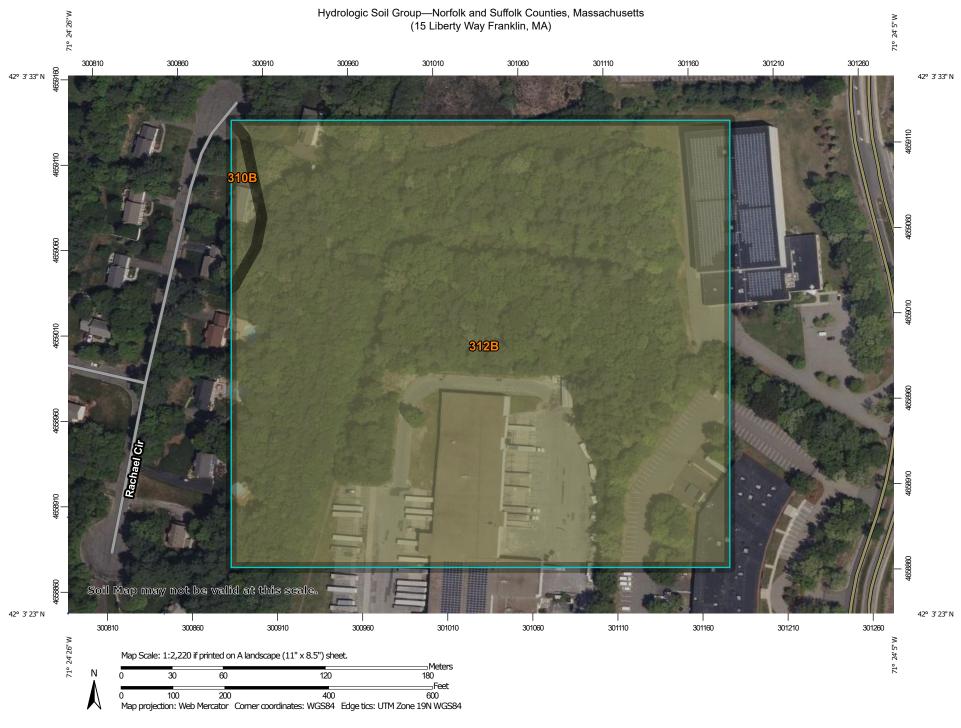
Depth (inches)	Horizon	Color	Texture	Redox Depth	Redox Color	Comments
0-12	А	10YR 2/2	SL			
12-24	В	10YR 5/6	SL			
24-120	C1	2.5Y 6/3	SL	32"		Very Compact
Weeping Observe	ed					
Standing Water C	Observed	none				
Redox Observed				32"		

 Test Hole No.:
 TH-3

 Time:
 11:45 AM

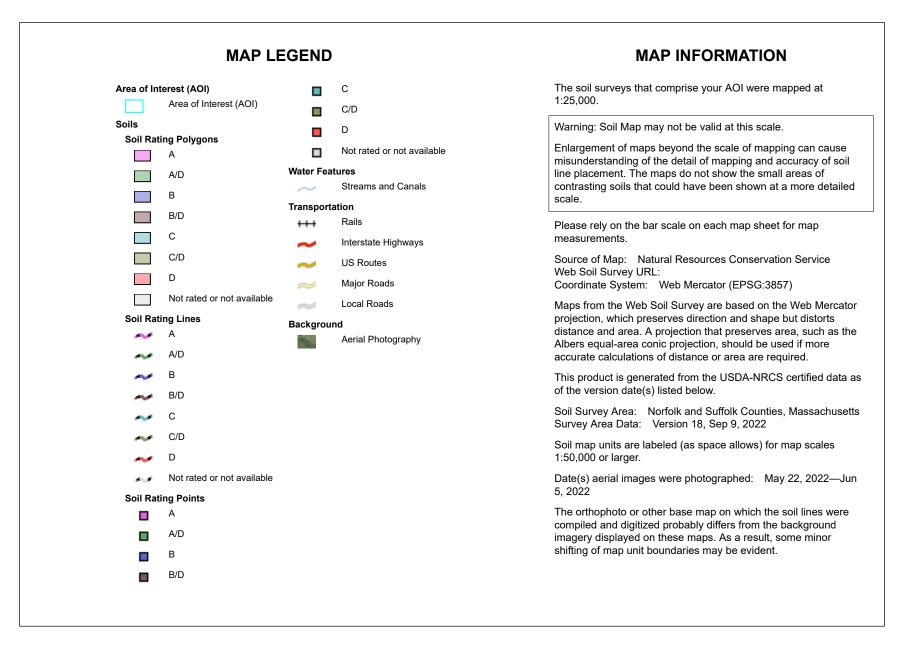
Ground Elevation at Hole = 328.50 Groundwater Elevation = 326.00

Depth (inches)	Horizon	Color	Texture	Redox Depth	Redox Color	Comments
0-12	А	10YR 2/2	SL			
12-28	В	10YR 5/6	SL			
28-72	C1	2.5Y 6/3	SL	30"		Very Compact
Weeping Observe	ed					
Standing Water C	bserved		I			
Redox Observed				30"		



USDA Natural Resources

Conservation Service



Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
310B	Woodbridge fine sandy loam, 3 to 8 percent slopes	C/D	0.3	1.5%
312B	Woodbridge fine sandy loam, 0 to 8 percent slopes, extremely stony	C/D	18.8	98.5%
Totals for Area of Intere	est		19.0	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher





MADEP TSS Removal Calculation Sheets

INSTRUCTIONS:

1. Sheet is nonautomated. Print sheet and complete using hand calculations. Column A and B: See MassDEP Structural BMP Table

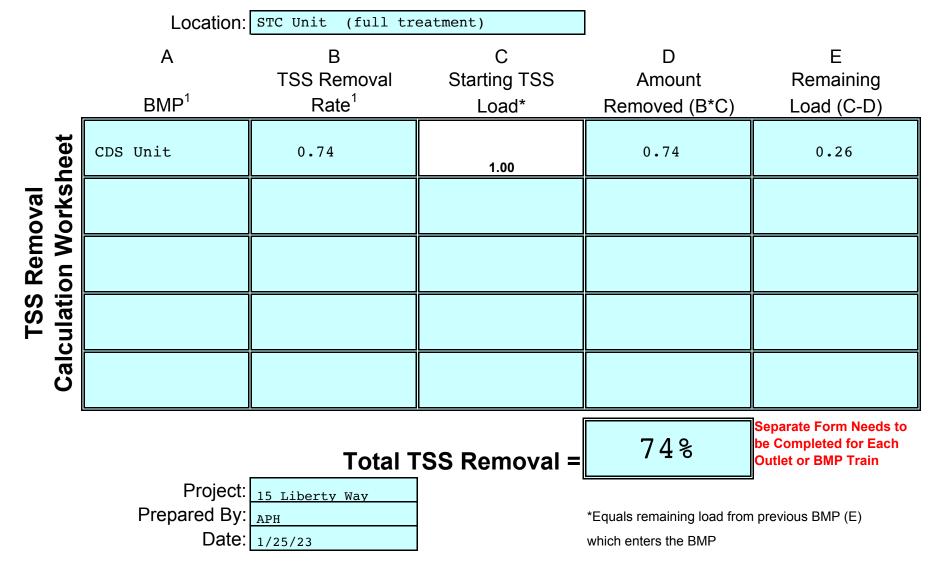
- 2. The calculations must be completed using the Column Headings specified in Chart and Not the Excel Column Headings
- 3. To complete Chart Column D, multiple Column B value within Row x Column C value within Row
- 4. To complete Chart Column E value, subtract Column D value within Row from Column C within Row
- 5. Total TSS Removal = Sum All Values in Column D

	Location:	CB#1 to CDS Unit	(full treatment)		
	А	B TSS Removal	C Starting TSS	D Amount	E Remaining
	BMP ¹	Rate ¹	Load*	Removed (B*C)	Load (C-D)
neet	Deep Sump & Hooded CBs	0.25	1.00	0.25	0.75
moval Worksheet	CDS Unit	0.74	0.75	0.55	0.20
()					
TSS Ro Calculation					
Calc					
			[SS Removal =	80%	Separate Form Needs to be Completed for Each Outlet or BMP Train
	Prepared By:	<u>15 Liberty Way</u> <u>APH</u> 1/25/23		*Equals remaining load from which enters the BMP	n previous BMP (E)

INSTRUCTIONS:

1. Sheet is nonautomated. Print sheet and complete using hand calculations. Column A and B: See MassDEP Structural BMP Table

- 2. The calculations must be completed using the Column Headings specified in Chart and Not the Excel Column Headings
- 3. To complete Chart Column D, multiple Column B value within Row x Column C value within Row
- 4. To complete Chart Column E value, subtract Column D value within Row from Column C within Row
- 5. Total TSS Removal = Sum All Values in Column D





Illicit Discharge Statement

Illicit Discharge Statement

Under the Massachusetts Wetlands Protection Act – Stormwater Management Standards

For 15 Liberty Way Franklin, MA

All illicit discharges to the Stormwater Management System are prohibited. The Stormwater Management System is the system for conveying, treating, and infiltrating stormwater. Illicit discharges to Stormwater Management Systems are discharges that are not entirely comprised of stormwater, but do not include discharges from the following activities or facilities:

- Firefighting
- Water Line Flushing
- Potable Water Sources
- Landscape Irrigation
- Potable Water Sources
- Uncontaminated Groundwater
- Air-conditioning Condensation

- Dechlorinated Water from Swimming Pools
- Water used for street washing
- Water used for clean residential buildings without detergents
- Foundation Drains

The site will be operated and maintained in accordance with the Operation and Maintenance Plan dated January 13, 2023 prepared by Level Design Group, LLC.

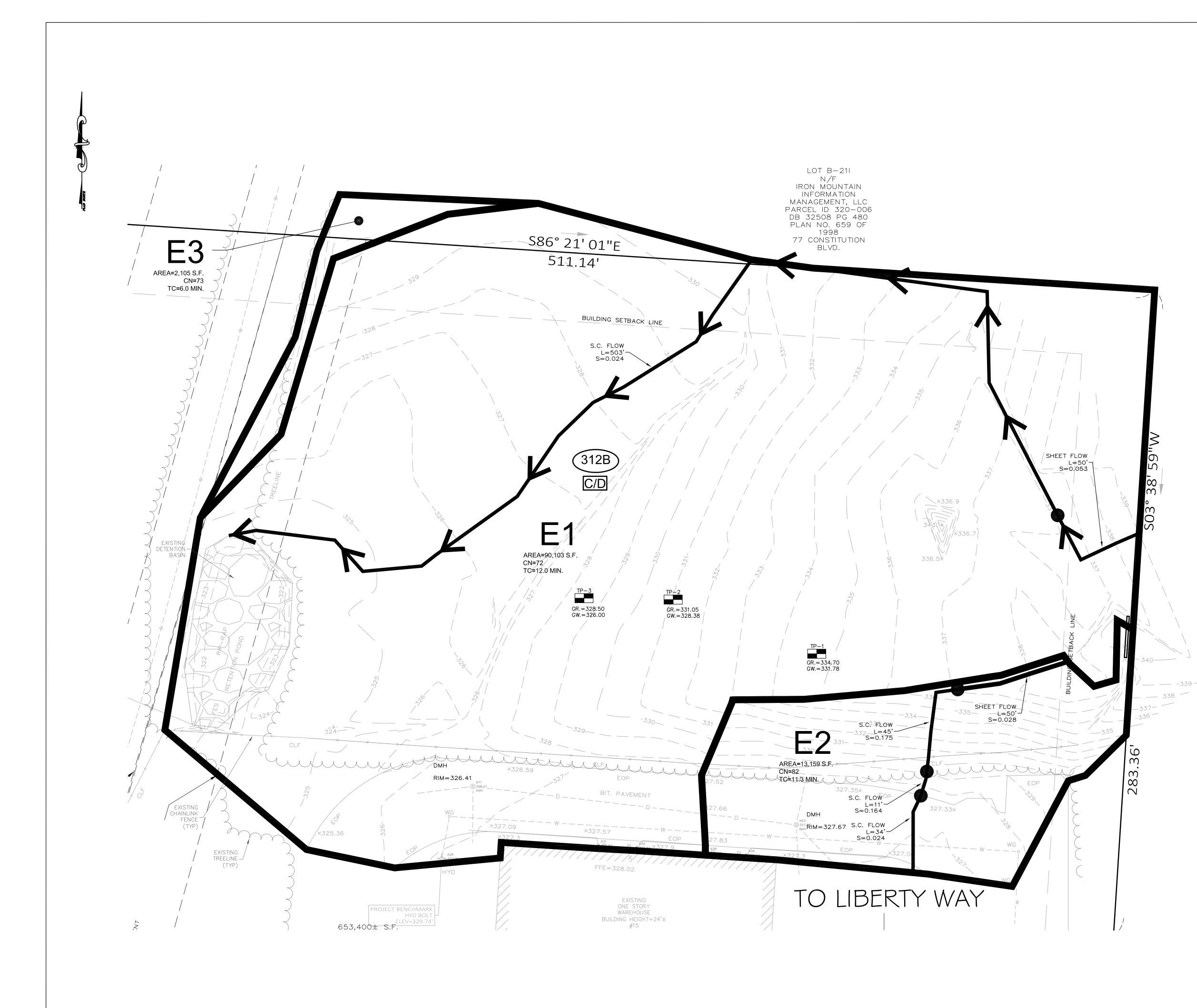
I, *(Applicant)* do hereby agree to comply with requirements set forth within the Illicit Discharge Statement and will not knowingly discharge illicit materials to the stormwater management system once it is brought online **upon** completion of construction.

Signature:_____

Date:



Existing Drainage Plan & HydroCAD Diagram



WATERSHED LEGEND



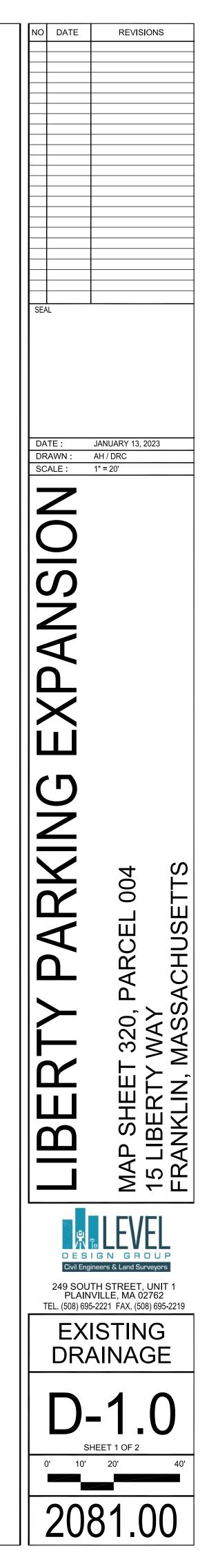
SOIL TYPE

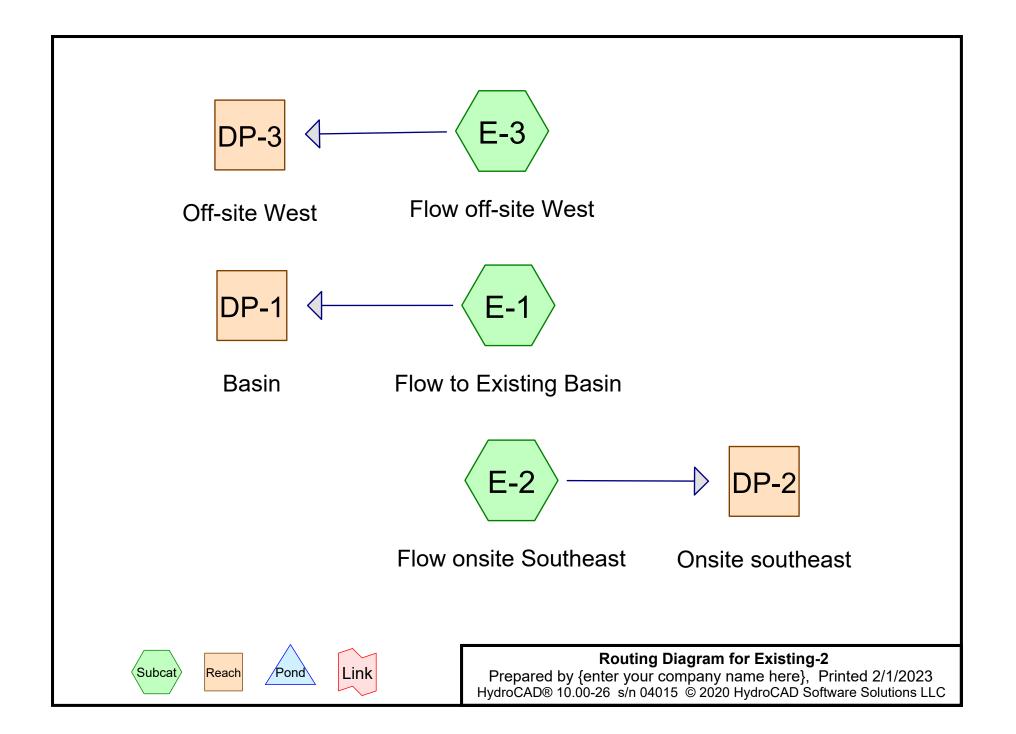
HYDROLOGIC SOIL GROUP

SOILS



WOODBRIDGE FINE SANDY LOAM







HydroCAD Analysis

Existing Conditions – 2 Year Storm

Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
9,896	79	50-75% Grass cover, Fair, HSG C (E-1, E-2, E-3)
10,479	98	Paved parking (E-1, E-2)
84,992	70	Woods, Good, HSG C (E-1, E-2, E-3)
105,367	74	TOTAL AREA

Existing-2 Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC

Printed 2/1/2023 Page 2

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
0	HSG B	
94,888	HSG C	E-1, E-2, E-3
0	HSG D	
10,479	Other	E-1, E-2
105.367		TOTAL AREA

Existing-2
Prepared by {enter your company name here}
HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC

Printed 2/1/2023 Page 3

			Grou	und Covers	(all nodes)		
HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(sq-ft) 0	(sq-ft) 0	(sq-ft) 9,896	(sq-ft) 0	(sq-ft) 0	(sq-ft) 9,896	Cover 50-75% Grass cover, Fair	Numbers
0	0	9,090 0	0	10,479	10,479	Paved parking	E-1, E-2
0	0	84,992	0	0	84,992	Woods, Good	E-1, E-2, E-3
0	0	94,888	0	10,479	105,367	TOTAL AREA	
i sting-2 pared by {ente			rel			Type III	24-hr 2-Year Rainfall=3.05" Printed 2/1/2023
droCAD® 10.00-2	6 s/n 04015 ©	2020 HydroCA	D Software Solu	tions LLC			Printed 2/1/2023 Page 4
		Ru	ime span=0.00 noff by SCS TF j by Stor-Ind+T	R-20 method,	JH=SCS, W		
bcatchmentE-1	I: Flow to Exi	sting Basin					Impervious Runoff Depth=0.84" CN=72 Runoff=1.49 cfs 6,292 cf
bcatchment E-2	2: Flow onsite	Southeast			Run Flow L	off Area=13,159 sf 38.16% ength=140' Tc=11.3 min (Impervious Runoff Depth=1.42" CN=82 Runoff=0.42 cfs 1,555 cf
bcatchment E-3	3: Flow off-sit	e West			R	unoff Area=2,105 sf 0.00% Tc=6.0 min	Impervious Runoff Depth=0.89" CN=73 Runoff=0.05 cfs 156 cf
ach DP-1: Basiı	n						Inflow=1.49 cfs 6,292 cf Outflow=1.49 cfs 6,292 cf
ach DP-2: Onsi	te southeast						Inflow=0.42 cfs 1,555 cf Outflow=0.42 cfs 1,555 cf
each DP-3: Off-s	ite West						Inflow=0.05 cfs 156 cf Outflow=0.05 cfs 156 cf
			Total Run	off Area = 105	5,367 sf Ru 90.05%	unoff Volume = 8,003 cf % Pervious = 94,888 sf	Average Runoff Depth = 0.91" 9.95% Impervious = 10,479 sf

-hr 2-Ye 5,457 5,777 7,869 0,103 4,646 5,457 .ength (feet) 50	ear Rair <u>CN E</u> 98 F 70 V 79 5 72 V 5 6 Slope (ft/ft) 0.0530 0.0240	nfall=3.05" Paved par Noods, Gi 50-75% G Weighted . 33.94% Pe 5.06% Imp Velocity (ft/sec) 0.10	" rking iood, HS irass cov Average ervious / pervious y Capa) ((SG C ver, Fa e Area s Area acity (cfs)	air, HSG C Description Sheet Flow, Woods: Ligh Shallow Cou Unpaved K	Sheet Flo t underbrus ncentrated	w sh n= 0.4							
5,457 5,777 7,869 0,103 4,646 5,457 .ength (feet) 50 503	98 F 70 V 79 5 72 V 56 6 Slope (ft/ft) 0.0530 0.0240	Paved par Noods, G 50-75% G Weighted J 33.94% Pe 5.06% Imp Velocity (ft/sec) 0.10	rking lood, HS Average ervious A pervious y Capa) (r	ver, Fa e Area s Area acity (cfs)	Description Sheet Flow, Woods: Ligh Shallow Col	t underbrus ncentrated	sh n= 0.4 i Flow,	.00 P2= :	3.05"					
5,777 7,869 0,103 4,646 5,457 .ength (feet) 50 503	70 V 79 5 72 V 50 6 50 6 50 6 6 50 6 6 50 6 6 72 8 50 72 V 5 6 6 72 8 72 V 5 72 8 72 V 5 72 8 72 V 5 72 V 7 72 V 5 72 V 5 72 V 7 72 V 7 72 V 7 72 V 7 72 V 7 72 V 7 72 V 7 72 V 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	Noods, G 50-75% G Weighted J 33.94% Pe 5.06% Imp Velocity (ft/sec) 0.10	iood, HS Grass cov Average ervious / pervious y Capa) ((ver, Fa e Area s Area acity (cfs)	Description Sheet Flow, Woods: Ligh Shallow Col	t underbrus ncentrated	sh n= 0.4 i Flow,	00 P2= :	3.05"					
7,869 0,103 4,646 5,457 .ength (feet) 50 503	79 5 72 V 56 Slope (ft/ft) 0.0530 0.0240	50-75% G Weighted J 33.94% Pe 5.06% Imp Velocity (ft/sec) 0.10	Grass cov Average ervious A pervious y Capa) (1)	ver, Fa e Area s Area acity (cfs)	Description Sheet Flow, Woods: Ligh Shallow Col	t underbrus ncentrated	sh n= 0.4 i Flow,	00 P2= :	3.05"					
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(feet) 50 503	(ft/ft) 0.0530 0.0240	(ft/sec) 0.10) (e D	(cfs)	Sheet Flow, Woods: Ligh Shallow Cor	t underbrus ncentrated	sh n= 0.4 i Flow,	.00 P2= :	3.05"					
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553	Total				Unpaved K	<u>v= 16.1 tps</u>								
				OCAD	Software Solu					Туре	e III 24-hr		Printed 2/1/2	
				Su			low to E	xisting I	Basin					
	1.4	9 cfs					Rı	unoff off V Run	r Ra Are /olur off E ow L	iinfal ea=90 me=6 Depth _engt	l=3.0),103 5,292 h=0.8 th=5 2.0 m)5" sf cf }4" 53' nin	Runof]
k	y {ent	y {enter your 10.00-26 s/n 04	y {enter your compan	y (enter your company name 10.00-26 s/n 04015 © 2020 Hydr	y {enter your company name here} 10.00-26 s/n 04015 © 2020 HydroCAD Su	y {enter your company name here} 10.00-26 s/n 04015 © 2020 HydroCAD Software Solu Subcatchme Hyc	y {enter your company name here} 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Subcatchment E-1: F Hydrograph	py {enter your company name here} 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Subcatchment E-1: Flow to E Hydrograph 1.49 cfs 2 Ru	enter your company name here} 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Subcatchment E-1: Flow to Existing Hydrograph 1.49 cfs 2-Yea Runoff Runoff Runoff Run	py {enter your company name here} 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph 1.49 cfs T 2-Year Ra Runoff Are Runoff Volu Runoff I Flow L	by {enter your company name here} 10.00-26 s/n 04015 @ 2020 HydroCAD Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph 1.49 cfs Type I 2-Year Rainfal Runoff Area=90 Runoff Volume=6 Runoff DeptI Flow Leng Tc=1	by {enter your company name here} 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24 2-Year Rainfall=3.0 Runoff Area=90,103 Runoff Volume=6,292 Runoff Depth=0.6 Flow Length=5 Tc=12.0 n CN=	y {enter your company name here} 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph	Printed 2/1/2 Particle 2020 HydroCAD Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 2-Year Rainfall=3.05" Runoff Area=90,103 sf Runoff Depth=0.84" Flow Length=553' Tc=12.0 min CN=72

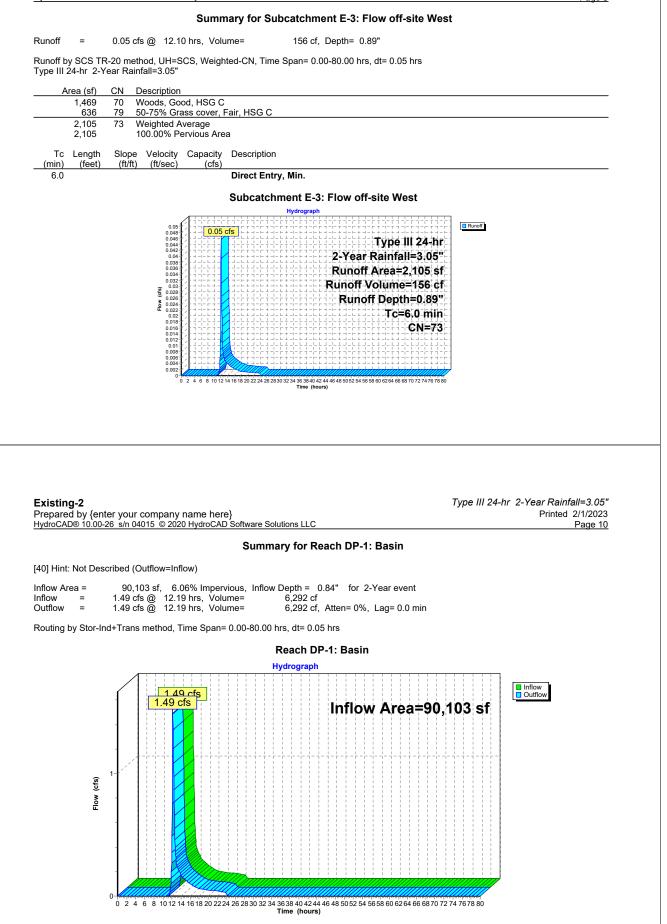
Summary for Subcatchment E-2: Flow onsite Southeast

Runoff = 0.42 cfs @ 12.16 hrs, Volume= 1,555 cf, Depth= 1.42"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs Type III 24-hr 2-Year Rainfall=3.05"

A	rea (sf)	CN	Description		
*	5,022	98	Paved park	ina	
	6.746			od, HSG C	
	1.391				Fair, HSG C
	13.159		Weighted A	,	
	8.137			rvious Area	
5,022 38.16% Impervious Are					ca
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)		(cfs)	2000,1210,1
11.0	50	0.0280			Sheet Flow.
					Woods: Light underbrush n= 0.400 P2= 3.05"
0.1	45	0.1750	6.74		Shallow Concentrated Flow,
0		000	0		Unpaved Kv= 16.1 fps
0.0	11	0.1640	6.52		Shallow Concentrated Flow,
0.0		00	0.02		Unpaved Kv= 16.1 fps
0.2	34	0.0240	3.14		Shallow Concentrated Flow,
0.2	01	0.0210	0.11		Paved Kv= 20.3 fps
11.3	140	Total			

Type III 24-hr 2-Year Rainfall=3.05" Existing-2 Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Printed 2/1/2023 Page 8 Subcatchment E-2: Flow onsite Southeast Hydrograph 0.46 Runoff 0.44 0.42 cfs 0.42 Type III 24-hr 0.4 0.38 2-Year Rainfall=3.05" 0.36 0.34 Runoff Area=13,159 sf 0.32 0.3 Runoff Volume=1,555 cf 0.28 () 0.26 0.24 Runoff Depth=1.42" 0.24 0.22 0.2 Flow Length=140' 0.2 0.18 Tc=11.3 min 0.16 0.14 CN=82 0.12 0.1 0.08 0.06 0.04 0.02 0 0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 74 76 78 80 Time (hours)



Summary for Reach DP-2: Onsite southeast

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	13,159 sf, 38.16% Impervious, Inflow Depth = 1.42" for 2-Year event	
Inflow	=	0.42 cfs @ 12.16 hrs, Volume= 1,555 cf	
Outflow	=	0.42 cfs @ 12.16 hrs, Volume= 1,555 cf, Atten= 0%, Lag= 0.0 n	nin

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs Reach DP-2: Onsite southeast Hydrograph Inflow
Outflow 0.42 cfs 0.42 cfs 0.46 0.44 Inflow Area=13,159 sf 0.42 0.4 0.38 0.36 0.34 0.3 (cfs) 0.26 0.24 0.22 Flov 0.18 0.16 0.14 0.12 0.1 0.08 0.06 0.04 0.02 0-0 2 4 6 8 1012 1416 18 2022 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 74 76 78 80 Time (hours)

Existing-2

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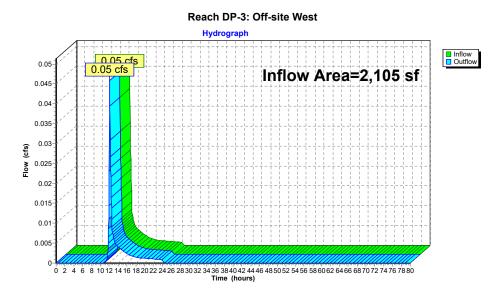
Type III 24-hr 2-Year Rainfall=3.05" Printed 2/1/2023 Page 12

Summary for Reach DP-3: Off-site West

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2,105 sf,	0.00% Impervious,	Inflow Depth = 0.89"	for 2-Year event
Inflow	=	0.05 cfs @	12.10 hrs, Volume=	156 cf	
Outflow	=	0.05 cfs @	12.10 hrs, Volume=	156 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs





HydroCAD Analysis

Existing Conditions – 10 Year Storm

249 SOUTH STREET UNIT 1 PLAINVILLE MA 02762 TEL508 695 2221 FAX508 695 2219 CONTACT@LEVELDG.COM LEVELDG.COM

Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
9,896	79	50-75% Grass cover, Fair, HSG C (E-1, E-2, E-3)
10,479	98	Paved parking (E-1, E-2)
84,992	70	Woods, Good, HSG C (E-1, E-2, E-3)
105,367	74	TOTAL AREA

Existing-2 Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC

Printed 2/1/2023 Page 2

Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
0	HSG B	
94,888	HSG C	E-1, E-2, E-3
0	HSG D	
10,479	Other	E-1, E-2
105.367		TOTAL AREA

Existing-2	
Prepared by {enter	your company name here}
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	Giou	nu covers (all nodes)		
HSG-A HSG-B HSG-C (sq-ft) (sq-ft) (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Subcatchment Numbers
0 0 9,896	0	0	9,896	50-75% Grass cover, Fair	
0 0 0	0	10,479	10,479	Paved parking	E-1, E-2
0 0 84,992 0 0 94,888	0 0	0 10,479	84,992 105,367	Woods, Good TOTAL AREA	E-1, E-2, E-3
epared by {enter your company name here		ions LLC		Type III 2	14-hr 10-Year Rainfall=5.15" Printed 2/1/2023 Page 4
epared by {enter your company name here troCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Tir Run) <u>Software Soluti</u> me span=0.00- off by SCS TR	80.00 hrs, dt= -20 method, l	JH=SCS, W	601 points	Printed 2/1/2023
epared by {enter your company name here droCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Tir Run Reach routing) <u>Software Soluti</u> me span=0.00- off by SCS TR	80.00 hrs, dt= -20 method, l	JH=SCS, W Pond routi Ru	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06%	Printed 2/1/2023
Run) <u>Software Soluti</u> me span=0.00- off by SCS TR	80.00 hrs, dt= -20 method, l	JH=SCS, W Pond routi Ru Flow Let Run	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06% ngth=553' Tc=12.0 min Cl off Area=13,159 sf 38.16%	Printed 2/1/2023 Page 4
epared by {enter your company name here troCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Tir Run Reach routing I bcatchment E-1: Flow to Existing Basin bcatchment E-2: Flow onsite Southeast) <u>Software Soluti</u> me span=0.00- off by SCS TR	80.00 hrs, dt= -20 method, l	JH=SCS, W Pond routi Ru Flow Lei Run Flow Li	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06% ngth=553' Tc=12.0 min Ci off Area=13,159 sf 38.16% ength=140' Tc=11.3 min (unoff Area=2,105 sf 0.00%	Printed 2/1/2023 Page 4 Impervious Runoff Depth=2.31" N=72 Runoff=4.53 cfs 17,375 cf Impervious Runoff Depth=3.21"
epared by {enter your company name here droCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Tir Run Reach routing bcatchment E-1: Flow to Existing Basin bcatchment E-2: Flow onsite Southeast bcatchment E-3: Flow off-site West) <u>Software Soluti</u> me span=0.00- off by SCS TR	80.00 hrs, dt= -20 method, l	JH=SCS, W Pond routi Ru Flow Lei Run Flow Li	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06% ngth=553' Tc=12.0 min Ci off Area=13,159 sf 38.16% ength=140' Tc=11.3 min (unoff Area=2,105 sf 0.00%	Printed 2/1/2023 Page 4 Impervious Runoff Depth=2.31" N=72 Runoff=4.53 cfs 17,375 cf Impervious Runoff Depth=3.21" CN=82 Runoff=0.95 cfs 3,524 cf Impervious Runoff Depth=2.40"
epared by {enter your company name here droCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Tir Run Reach routing I bcatchmentE-1: Flow to Existing Basin) <u>Software Soluti</u> me span=0.00- off by SCS TR	80.00 hrs, dt= -20 method, l	JH=SCS, W Pond routi Ru Flow Lei Run Flow Li	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06% ngth=553' Tc=12.0 min Ci off Area=13,159 sf 38.16% ength=140' Tc=11.3 min (unoff Area=2,105 sf 0.00%	Printed 2/1/2023 Page 4 Impervious Runoff Depth=2.31" N=72 Runoff=4.53 cfs 17,375 cf Impervious Runoff Depth=3.21" CN=82 Runoff=0.95 cfs 3,524 cf Impervious Runoff Depth=2.40" CN=73 Runoff=0.13 cfs 421 cf Inflow=4.53 cfs 17,375 cf

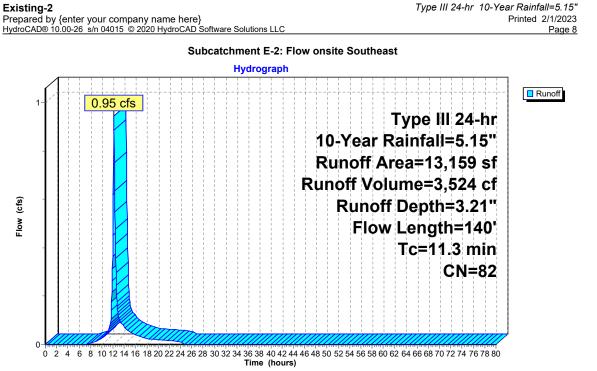
			thod, UH=S ainfall=5.15		
	Area (sf)		Description		
	5,457 76,777	98	Paved park		<u></u>
	7,869	79	50-75% Gra	ass cover, I	, Fair, HSG C
	90,103 84,646 5,457			rvious Area ervious Area	
Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
8.6	50	0.0530		(0.0)	Sheet Flow, Sheet Flow Woods: Light underbrush n= 0.400 P2= 3.05"
3.4	503	0.0240	2.49		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
12.0	553	Total			onpaved Kv- to. hps
Existir Prepare HydroCA	ed by {en	ter you 26 s/n 0	company 4015 © 202		Type III 24-hr 10-Year Rainfall=5.15" re} D Software Solutions LLC Page 6 Subcatchment E-1: Flow to Existing Basin
Prepare	ed by {en	ter your 26 s/n 0	company 4015 © 202	0 HydroCAE	re} Printed 2/1/2023 D Software Solutions LLC Page 6
Prepare	ed by {en	ter your	company 4015 © 202	0 HydroCAE	re} Printed 2/1/2023 D Software Solutions LLC Page 6 Subcatchment E-1: Flow to Existing Basin Hydrograph
Prepare	ed by {en	- <u>26 s/n 0</u>	company 4015 © 202	0 HydroCAE	re} Printed 2/1/2023 D Software Solutions LLC Page 6 Subcatchment E-1: Flow to Existing Basin Hydrograph Runoff Runoff
Prepare	ed by {en	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} Printed 2/1/2023 D Software Solutions LLC Page 6 Subcatchment E-1: Flow to Existing Basin Hydrograph Runoff Runoff
Prepare HydroCA	ed by {en \D® 10.00	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} Printed 2/1/2023 D Software Solutions LLC Page 6 Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr
Prepare HydroCA	ed by {en	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} D Software Solutions LLC Page 6 Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 10-Year Rainfall=5.15"
Prepare HydroCA	ed by {en \D® 10.00	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} D Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 10-Year Rainfall=5.15" Runoff Area=90,103 sf
Prepare HydroCA	5- 4-	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} D Software Solutions LLC Page 6 Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 10-Year Rainfall=5.15"
Prepare HydroCA	ed by {en \D® 10.00	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} D Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 10-Year Rainfall=5.15" Runoff Area=90,103 sf Runoff Volume=17,375 cf
Prepare HydroCA	5- 4-	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} D Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 10-Year Rainfall=5.15" Runoff Area=90,103 sf Runoff Volume=17,375 cf Runoff Depth=2.31"
Prepare HydroCA	5- 4-	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} D Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 10-Year Rainfall=5.15" Runoff Area=90,103 sf Runoff Volume=17,375 cf Runoff Depth=2.31" Flow Length=553'
Prepare HydroCA	5- 4-	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} D Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 10-Year Rainfall=5.15" Runoff Area=90,103 sf Runoff Volume=17,375 cf Runoff Depth=2.31"
Prepare HydroCA	5- 	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} D Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 10-Year Rainfall=5.15" Runoff Area=90,103 sf Runoff Volume=17,375 cf Runoff Depth=2.31" Flow Length=553' Tc=12.0 min
Prepare HydroCA	5- 	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} D Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 10-Year Rainfall=5.15" Runoff Area=90,103 sf Runoff Volume=17,375 cf Runoff Depth=2.31" Flow Length=553'
Prepare HydroCA	5- 	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} D Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 10-Year Rainfall=5.15" Runoff Area=90,103 sf Runoff Volume=17,375 cf Runoff Depth=2.31" Flow Length=553' Tc=12.0 min
Prepare HydroCA	5- 	- <u>26 s/n 0</u>	4015 © 202	0 HydroCAE	re} D Software Solutions LLC Subcatchment E-1: Flow to Existing Basin Hydrograph Type III 24-hr 10-Year Rainfall=5.15" Runoff Area=90,103 sf Runoff Volume=17,375 cf Runoff Depth=2.31" Flow Length=553' Tc=12.0 min

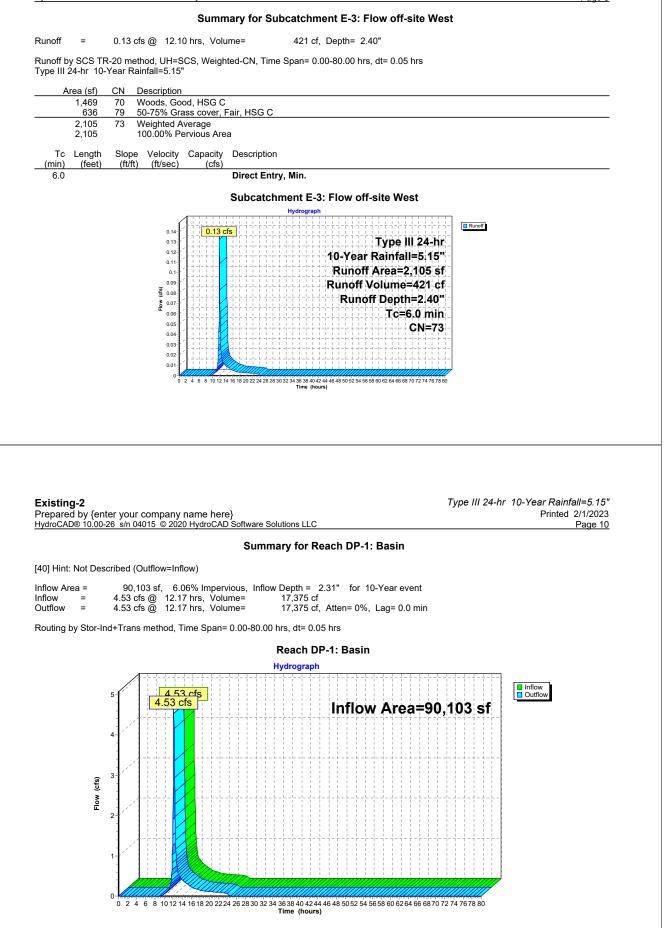
Summary for Subcatchment E-2: Flow onsite Southeast

Runoff 0.95 cfs @ 12.16 hrs, Volume= 3,524 cf, Depth= 3.21" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs Type III 24-hr 10-Year Rainfall=5.15"

^	rea (sf)	CN	Description		
P					
*	5,022	98	Paved park	ing	
	6.746	70	Woods, Go	od, HSG C	
	1,391				Fair, HSG C
	13,159		Weighted A	,	
	8.137			rvious Area	
	5,022		38.16% IM	pervious Are	
_				.	
Tc		Slope		Capacity	Description
(min)	(feet)	(ft/ft	(ft/sec)	(cfs)	
11.0	50	0.0280	0.08		Sheet Flow.
					Woods: Light underbrush n= 0.400 P2= 3.05"
0.1	45	0.1750	6.74		Shallow Concentrated Flow,
0.1	45	0.1750	0.74		
					Unpaved Kv= 16.1 fps
0.0	11	0.1640	6.52		Shallow Concentrated Flow,
					Unpaved Kv= 16.1 fps
0.2	34	0.0240	3.14		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
11.3	140	Total			
11.5	140	rolai			



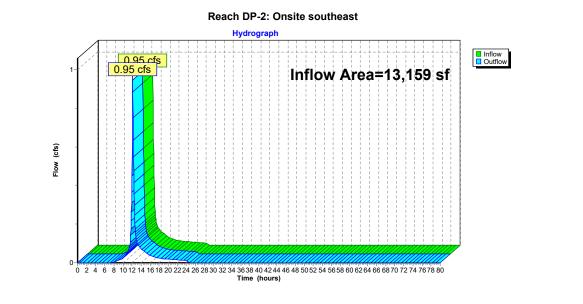


Summary for Reach DP-2: Onsite southeast

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	13,159 sf, 38.16% Impervious, Inflow Depth = 3.21" for 10-Year event	ent
Inflow	=	0.95 cfs @ 12.16 hrs, Volume= 3,524 cf	
Outflow	=	0.95 cfs @ 12.16 hrs, Volume= 3,524 cf, Atten= 0%, Lag= 0.0 min	min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs



Existing-2

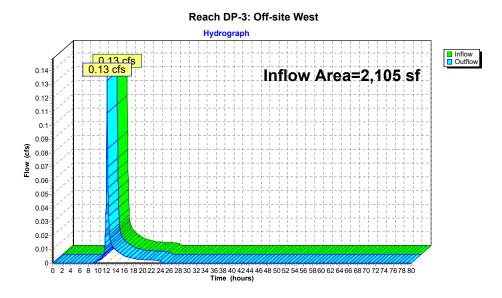
Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Type III 24-hr 10-Year Rainfall=5.15" Printed 2/1/2023 Page 12

Summary for Reach DP-3: Off-site West

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2,105 sf,	0.00% Impervious,	Inflow Depth = 2.40"	for 10-Year event
Inflow	=	0.13 cfs @ 1	12.10 hrs, Volume=	421 cf	
Outflow	=	0.13 cfs @ 1	12.10 hrs, Volume=	421 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs





HydroCAD Analysis

Existing Conditions - 25 Year Storm

Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
9,896	79	50-75% Grass cover, Fair, HSG C (E-1, E-2, E-3)
10,479	98	Paved parking (E-1, E-2)
84,992	70	Woods, Good, HSG C (E-1, E-2, E-3)
105,367	74	TOTAL AREA

Existing-2 Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC

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Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
0	HSG B	
94,888	HSG C	E-1, E-2, E-3
0	HSG D	
10,479	Other	E-1, E-2
105.367		TOTAL AREA

Existing-2
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(so ff)	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(sq-ft) 0	(sq-ft) 0	(sq-ft) 9,896	(sq-ft) 0	(sq-ft) 0	(sq-ft) 9,896	Cover 50-75% Grass cover, Fair	Numbers
0	0	9,090 0	0	10,479	10,479	Paved parking	E-1, E-2, E-3
0	0	84,992	0	0	84,992	Woods, Good	E-1, E-2, E-3
0	0	94,888	0	10,479	105,367	TOTAL AREA	
pared by {enter	er your comp 26 s/n 04015 ©	any name he 2020 HydroCA	re} D Software Solu	itions LLC		Type III 2	24-hr 25-Year Rainfall=6.35" Printed 2/1/2023 Page 4
pared by {enter	er your comp 26 s/n 04015 ©	2020 HydroCA T Ru	D Software Solu ime span=0.00 noff by SCS TF	0-80.00 hrs, dt= R-20 method, l	JH=SCS, W	601 points	Printed 2/1/2023
isting-2 pared by {ente lroCAD® 10.00-2	26 s/n 04015 ©	<u>∂ 2020 HydroCA</u> T Ru Reach routing	D Software Solu ime span=0.00 noff by SCS TF	0-80.00 hrs, dt= R-20 method, l	JH=SCS, W Pond rout Ru	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06%	Printed 2/1/2023
pared by {enter roCAD® 10.00-2	26 s/n 04015 ⓒ 1: Flow to Exis	⊇ 2020 HydroCA T Ru Reach routing sting Basin	D Software Solu ime span=0.00 noff by SCS TF	0-80.00 hrs, dt= R-20 method, l	JH=SCS, W Pond routi Ru Flow Le Run	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06% ngth=553' Tc=12.0 min C off Area=13,159 sf 38.16%	Printed 2/1/2023 Page 4
pared by {ente roCAD® 10.00-2	26 s/n 04015 1: Flow to Exi 2: Flow onsite	⊇ 2020 HydroCA T Ru Reach routing sting Basin Southeast	D Software Solu ime span=0.00 noff by SCS TF	0-80.00 hrs, dt= R-20 method, l	JH=SCS, W Pond rout Ru Flow Le Run Flow L	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06% ngth=553' Tc=12.0 min C off Area=13,159 sf 38.16% ength=140' Tc=11.3 min (unoff Area=2,105 sf 0.00%	Printed 2/1/2023 Page 4 Impervious Runoff Depth=3.28" N=72 Runoff=6.49 cfs 24,642 cf Impervious Runoff Depth=4.31"
pared by {ente roCAD® 10.00-2 pocatchmentE-1 pocatchmentE-2	26 s/n 04015 @ 1: Flow to Exis 2: Flow onsite 3: Flow off-sit	⊇ 2020 HydroCA T Ru Reach routing sting Basin Southeast	D Software Solu ime span=0.00 noff by SCS TF	0-80.00 hrs, dt= R-20 method, l	JH=SCS, W Pond rout Ru Flow Le Run Flow L	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06% ngth=553' Tc=12.0 min C off Area=13,159 sf 38.16% ength=140' Tc=11.3 min (unoff Area=2,105 sf 0.00%	Printed 2/1/2023 Page 4 Impervious Runoff Depth=3.28" N=72 Runoff=6.49 cfs 24,642 cf Impervious Runoff Depth=4.31" CN=82 Runoff=1.26 cfs 4,727 cf
pared by {ente roCAD® 10.00-2 pocatchmentE-1 pocatchmentE-2	26 s/n 04015 ⓒ 1: Flow to Exi 2: Flow onsite 3: Flow off-sit n	⊇ 2020 HydroCA T Ru Reach routing sting Basin Southeast	D Software Solu ime span=0.00 noff by SCS TF	0-80.00 hrs, dt= R-20 method, l	JH=SCS, W Pond rout Ru Flow Le Run Flow L	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06% ngth=553' Tc=12.0 min C off Area=13,159 sf 38.16% ength=140' Tc=11.3 min (unoff Area=2,105 sf 0.00%	Printed 2/1/2023 Page 4 Memory Page 4 Memory

Runoff b Type III	24-hr 25-)5 hrs		
<u></u>	Area (sf) 5,457		Description							
	76,777	70 \		od, HSG C						
	7,869 90,103	72 \	Neighted A	verage	Fair, HSG C					
	84,646 5,457			rvious Area ervious Are						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
8.6	50	0.0530	0.10	(013)	Sheet Flow, S		- 0 400 D2-	2.05"		
3.4	503	0.0240	2.49		Woods: Light Shallow Cond Unpaved Kv=	centrated Flov		3.05		
12.0	553	Total								
Prepare	ed by {en	ter your 26 s/n 0/	company L015 © 202		D Software Solution		to Existing		vpe III 24-hr 2	5-Year Rainfall=6.35" Printed 2/1/2023 Page 6
Existin Prepare HydroCA	ed by {en	ter your 26 s/n 04	company 1015 © 202	0 HydroCAI	D Software Solution		to Existing		/pe III 24-hr 2	Printed 2/1/2023
Prepare HydroCA	7- 6-	<u>26 s/n 04</u>	company 4015 © 202	0 HydroCAI	D Software Solution	t E-1: Flow	25-Ye	^{Basin} Typ ar Rain	/pe III 24-hr 2 e III 24-l fall=6.3{	Printed 2/1/2023 Page 6
Prepare HydroCA	ed by {en \D® 10.00-	<u>26 s/n 04</u>	4015 © 202	0 HydroCAI	D Software Solution	t E-1: Flow ograph	25-Ye Runot	^{Basin} Typ ar Rain if Area=	e 24- fall=6.3{	Printed 2/1/2023 Page 6
Prepare <u>HydroCA</u>	7- 6-	<u>26 s/n 04</u>	4015 © 202	0 HydroCAI	D Software Solution	t E-1: Flow ograph	25-Ye Runo unoff V	^{Basin} Typ ar Rain if Area= olume=	e III 24-I fall=6.3 90,103 24,642	Printed 2/1/2023 Page 6
Prepare <u>HydroCA</u>	7- 6-	<u>26 s/n 04</u>	4015 © 202	0 HydroCAI	D Software Solution	t E-1: Flow ograph	25-Ye Runo unoff V Ru	^{Basin} Typ ar Rain ff Area= olume= noff De	e III 24-I fall=6.3 90,103 24,642 pth=3.28	Printed 2/1/2023 Page 6
Prepare HydroCA	7- 6-	<u>26 s/n 04</u>	4015 © 202	0 HydroCAI	D Software Solution	t E-1: Flow ograph	25-Ye Runo unoff V Ru	Basin Typ ar Rain if Area= olume= noff De Iow Lei	e 24- fall=6.3 90,103 24,642 pth=3.28 ngth=55	Printed 2/1/2023 Page 6
Prepare HydroCA	2d by {en D® 10.00-	<u>26 s/n 04</u>	4015 © 202	0 HydroCAI	D Software Solution	t E-1: Flow ograph	25-Ye Runo unoff V Ru	Basin Typ ar Rain if Area= olume= noff De Iow Lei	e III 24-I fall=6.3 90,103 24,642 pth=3.28 ngth=55 =12.0 m	Printed 2/1/2023 Page 6
Prepare HydroCA Elow (cts)	2d by {en D® 10.00-	<u>26 s/n 04</u>	4015 © 202	0 HydroCAI	D Software Solution	t E-1: Flow ograph	25-Ye Runo unoff V Ru	Basin Typ ar Rain if Area= olume= noff De Iow Lei	e 24- fall=6.3 90,103 24,642 pth=3.28 ngth=55	Printed 2/1/2023 Page 6
Prepare HydroCA Elow (cts)	7- 6- 5- 4-	<u>26 s/n 04</u>	4015 © 202	0 HydroCAI	D Software Solution	t E-1: Flow ograph	25-Ye Runo unoff V Ru	Basin Typ ar Rain if Area= olume= noff De Iow Lei	e III 24-I fall=6.3 90,103 24,642 pth=3.28 ngth=55 =12.0 m	Printed 2/1/2023 Page 6

Summary for Subcatchment E-2: Flow onsite Southeast

Runoff 1.26 cfs @ 12.16 hrs, Volume= 4,727 cf, Depth= 4.31" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs Type III 24-hr 25-Year Rainfall=6.35"

	А	rea (sf)	CN	Description		
1		5,022	98	Paved park	ina	
		6.746			od, HSG C	
		1.391				Fair, HSG C
-		13.159		Weighted A	,	
		8.137			vious Area	
		5,022			pervious Are	
		0,022		00.1070 111		
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)		(cfs)	2000.000
-	11.0	50	0.0280		()	Sheet Flow.
			0.0200	0.00		Woods: Light underbrush n= 0.400 P2= 3.05"
	0.1	45	0.1750	6.74		Shallow Concentrated Flow,
	0.1	10	0.1700	0.71		Unpaved Kv= 16.1 fps
	0.0	11	0.1640	6.52		Shallow Concentrated Flow,
	0.0		0.1010	0.02		Unpaved Kv= 16.1 fps
	0.2	34	0.0240	3.14		Shallow Concentrated Flow,
	0.2	04	0.0240	0.14		Paved Kv= 20.3 fps
-	11.3	140	Total			

Existing-2

Type III 24-hr 25-Year Rainfall=6.35" Printed 2/1/2023 Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 8 Subcatchment E-2: Flow onsite Southeast Hydrograph Runoff 1.26 cfs Type III 24-hr 25-Year Rainfall=6.35" Runoff Area=13,159 sf 1 Runoff Volume=4,727 cf Runoff Depth=4.31" Flow (cfs) Flow Length=140' Tc=11.3 min **CN=82** 0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 74 76 78 80 Time (hours)

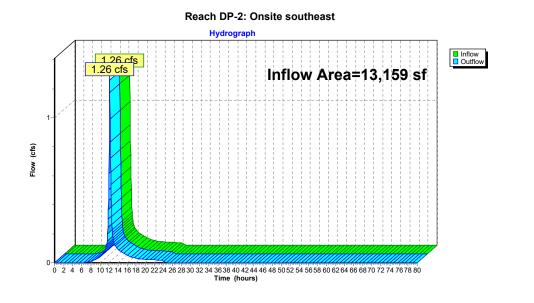
	=	0.19 cf	s@ 12.0	9 hrs, Vo	ume=		593 c	f, Dept	h= 3.38	3"									
	/ SCS TR- 24-hr 25-Y				hted-CN	, Time S	Span= (0.00-80.	00 hrs,	dt= 0.0	5 hrs								
Ar			escription																
	1,469 636	79 5	0-75% Gr	od, HSG (ass cover,		SG C													
	2,105 2,105		/eighted / 00.00% P	verage ervious Ar	ea														
	Length			Capacity		iption													
<u>(min)</u> 6.0	(feet)	(ft/ft)	(ft/sec)	(cfs)		t Entry,	Min.												
					Subc	atchm	ent E	3: Flo	w off-s	site W	est								
			021 022 0.19 0.19 0.16 0.17 0.16 0.13 0.13 0.13 0.13 0.13 0.13 0.12 0.14 0.14 0.00 0.01 0.00 0.00 0.00 0.00						Year F Inoff / Ioff Vi	Area= olume Dept Tc=	=6, 2,10 ≥=59; h=3, 6,0 h CN	35" 5 sf 3 cf 38" 73		noff					
				nome bo								T	ype l	II 24-I	hr 2			nfall=6.3	
Prepared	g-2 d by {ente D® 10.00-26				D Softwa					Desir		7	ype l	II 24-I	hr 2			n <i>fall=6.3</i> d 2/1/20 Page	23
Prepared HydroCAL	d by {ente D® 10.00-26	<u>ð s/n 04</u>	015 © 202	0 HydroCA	D Softwa	are Soluti			DP-1:	Basin		7	ype l	II 24-ł	hr 2			d 2/1/20	23
Prepared HydroCAE [40] Hint: Inflow Are Inflow	d by {ente <u>D® 10.00-26</u> Not Descr ea =	<u>s s/n 04</u> ibed (C 90,1	0 <u>15 © 202</u> utflow=Int 03 sf, 6.	<u>0 HydroCA</u> low) 06% Impe	D Softwa Su	immar nflow D	y for F epth =	Reach 3.28"	for 25	-Year e	event		ype I	II 24-I	hr 2			d 2/1/20	23
[40] Hint: Inflow Are Inflow Outflow	d by {ente <u>D® 10.00-26</u> Not Descr ea =	ibed (C 90,1 6.49 cf 6.49 cf	015 © 202 utflow=Inf 03 sf, 6. s @ 12.1 s @ 12.1	0 HydroCA 1ow) 06% Impe 7 hrs, Vo 7 hrs, Vo	<u>D Softwa</u> Su rvious, 1 lume= lume=	immar Inflow D 2 2	y for F epth = 4,642 c 4,642 c	3.28" f f, Atter		-Year e	event		ype I	II 24-I	hr 2			d 2/1/20	23
Prepared HydroCAE [40] Hint: Inflow Are Inflow Outflow	d by {ente <u>0</u> ® 10.00-26 Not Descr ea = = = =	ibed (C 90,1 6.49 cf 6.49 cf	015 © 202 utflow=Inf 03 sf, 6. s @ 12.1 s @ 12.1	0 HydroCA 1ow) 06% Impe 7 hrs, Vo 7 hrs, Vo	<u>D Softwa</u> Su rvious, 1 lume= lume=	Immar Inflow D 2 2 0.00 hrs	y for F epth = 4,642 c 4,642 c	Reach 3.28" f f, Atter 05 hrs	for 25 = 0%,	-Year e	event		ype I	24-	hr 2			d 2/1/20	23
Prepared HydroCAE [40] Hint: Inflow Are Inflow Outflow	d by {ente <u>0</u> ® 10.00-26 Not Descr ea = = = =	ibed (C 90,1 6.49 cf 6.49 cf	015 © 202 utflow=Inf 03 sf, 6. s @ 12.1 s @ 12.1	0 HydroCA 1ow) 06% Impe 7 hrs, Vo 7 hrs, Vo	<u>D Softwa</u> Su rvious, 1 lume= lume=	immar inflow D 2 2 0.00 hrs R i	y for F epth = 4,642 c 4,642 c s, dt= 0.	3.28" f f, Atter 05 hrs DP-1: I	for 25 = 0%,	-Year e	event		ype I	11 24-1	hr 2			d 2/1/20	23
Prepared HydroCAE [40] Hint: Inflow Are Inflow Outflow	d by {ente <u>0</u> ® 10.00-26 Not Descr ea = = = =	ibed (C 90,1 6.49 cf 6.49 cf +Trans	015 © 202 utflow=Inf 03 sf, 6. s @ 12.1 s @ 12.1	<u>0 HydroCA</u> low) 06% Impe 7 hrs, Vo 7 hrs, Vo 7 ime Span	<u>D Softwa</u> Su rvious, 1 lume= lume=	immar inflow D 2 2 0.00 hrs R i	y for F epth = 4,642 c 4,642 c s, dt= 0. each E	3.28" f f, Atter 05 hrs)P-1: I h	for 25 = 0%,	-Year 6 Lag= 0	event .0 min						Printed v	d 2/1/20	23
Prepared HydroCAE [40] Hint: Inflow Are Inflow Outflow	d by {ente ≫ 10.00-20 Not Desci ea = = = ; ; ;	ibed (C 90,1 6,49 cf 6,49 cf 6,49 cf +Trans	utflow=Ini 03 sf, 6. s @ 12.1 s @ 12.1 method, ⁻	<u>0 HydroCA</u> low) 06% Impe 7 hrs, Vo 7 hrs, Vo 7 ime Span	<u>D Softwa</u> Su rvious, 1 lume= lume=	immar inflow D 2 2 0.00 hrs R i	y for F epth = 4,642 c 4,642 c s, dt= 0. each E	3.28" f f, Atter 05 hrs)P-1: I h	for 25 = 0%, Basin	-Year 6 Lag= 0	event .0 min					Inflov	Printed v	d 2/1/20	23

Summary for Reach DP-2: Onsite southeast

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =	13,159 sf, 38.16% Impervious,	Inflow Depth = 4.31" for 25-Year event
Inflow =	1.26 cfs @ 12.16 hrs, Volume=	4,727 cf
Outflow =	1.26 cfs @ 12.16 hrs, Volume=	4,727 cf, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs



Existing-2

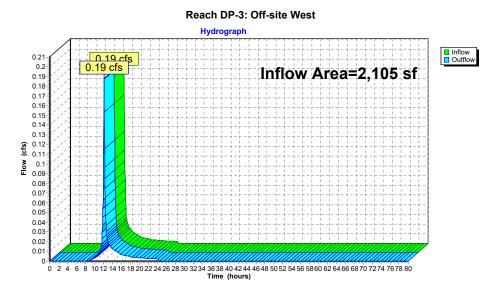
Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Type III 24-hr 25-Year Rainfall=6.35" Printed 2/1/2023 Page 12

Summary for Reach DP-3: Off-site West

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	2,105 sf,	0.00% Impervious,	Inflow Depth = 3.38	for 25-Year event
Inflow	=	0.19 cfs @	12.09 hrs, Volume=	593 cf	
Outflow	=	0.19 cfs @	12.09 hrs, Volume=	593 cf, Att	en= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs





HydroCAD Analysis

Existing Conditions - 100 Year Storm

Area Listing (all nodes)

Area (sq-ft)	CN	Description (subcatchment-numbers)
9,896	79	50-75% Grass cover, Fair, HSG C (E-1, E-2, E-3)
10,479	98	Paved parking (E-1, E-2)
84,992	70	Woods, Good, HSG C (E-1, E-2, E-3)
105,367	74	TOTAL AREA

Existing-2 Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC

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Soil Listing (all nodes)

Area (sq-ft)	Soil Group	Subcatchment Numbers
0	HSG A	
0	HSG B	
94,888	HSG C	E-1, E-2, E-3
0	HSG D	
10,479	Other	E-1, E-2
105.367		TOTAL AREA

Existing-2	
Prepared by {enter your company name here}	
HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC	

Printed 2/1/2023 Page 3

1	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover	Numbers
0 0	0 0	9,896 0	0 0	0 10,479	9,896 10,479	50-75% Grass cover, Fair Paved parking	E-1, E-2, E-3 E-1, E-2
0	0	84,992	0	0	84,992	Woods, Good	E-1, E-2, E-3
0	0	94,888	0	10,479	105,367	TOTAL AREA	
isting-2 epared by {ente troCAD® 10.00-2	er your comp 8 s/n 04015 ©	any name he	re} D Software Solu	tions LLC		Type III 24	<i>I-hr 100-Year Rainfall=8.16"</i> Printed 2/1/2023 Page 4
epared by {enter	er your comp 26 s/n 04015 ©	2020 HydroCA T Ru	D Software Solu ime span=0.00 noff by SCS TF	-80.00 hrs, dt= R-20 method, l	JH=SCS, W	601 points	
epared by {ente	26 s/n 04015 ©	<u>2020 HydroCA</u> T Ru Reach routing	D Software Solu ime span=0.00 noff by SCS TF	-80.00 hrs, dt= R-20 method, l	JH=SCS, W Pond rout Ru	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06%	Printed 2/1/2023
epared by {ente troCAD® 10.00-2	26 ˈs/n 04015 ⓒ 1: Flow to Exis	∂ 2020 HydroCA T Ru Reach routing sting Basin	D Software Solu ime span=0.00 noff by SCS TF	-80.00 hrs, dt= R-20 method, l	JH=SCS, W Pond rout Ru Flow Le Run	601 points /eighted-CN ing by Stor-Ind method noff Area=90,103 sf 6.06% ngth=553' Tc=12.0 min C off Area=13,159 sf 38.16%	Printed 2/1/2023 Page 4
epared by {ente troCAD® 10.00-2 bcatchmentE-1 bcatchmentE-2	26 s/n 04015 @ 1: Flow to Exis 2: Flow onsite	≥ 2020 HydroCA T Ru Reach routing sting Basin Southeast	D Software Solu ime span=0.00 noff by SCS TF	-80.00 hrs, dt= R-20 method, l	JH=SCS, W Pond rout Ru Flow Le Run Flow L	601 points /eighted-CN ing by Stor-Ind method ngth=553' Tc=12.0 min C off Area=13,159 sf 38.16% ength=140' Tc=11.3 min (unoff Area=2,105 sf 0.00%	Printed 2/1/2023 Page 4 Impervious Runoff Depth=4.84" N=72 Runoff=9.57 cfs 36,305 cf Impervious Runoff Depth=6.01"
epared by {ente lroCAD® 10.00-2 bocatchmentE-1 bocatchmentE-2 bocatchmentE-3	26 s/n 04015 @ 1: Flow to Exis 2: Flow onsite 3: Flow off-sit	≥ 2020 HydroCA T Ru Reach routing sting Basin Southeast	<u>D Software Solu</u> ime span=0.00 noff by SCS TF	-80.00 hrs, dt= R-20 method, l	JH=SCS, W Pond rout Ru Flow Le Run Flow L	601 points /eighted-CN ing by Stor-Ind method ngth=553' Tc=12.0 min C off Area=13,159 sf 38.16% ength=140' Tc=11.3 min (unoff Area=2,105 sf 0.00%	Printed 2/1/2023 Page 4 Impervious Runoff Depth=4.84" N=72 Runoff=9.57 cfs 36,305 cf Impervious Runoff Depth=6.01" CN=82 Runoff=1.74 cfs 6,592 cf Impervious Runoff Depth=4.95"
epared by {ente troCAD® 10.00-2	1: Flow to Exi 1: Flow to Exi 2: Flow onsite 3: Flow off-sit n	≥ 2020 HydroCA T Ru Reach routing sting Basin Southeast	<u>D Software Solu</u> ime span=0.00 noff by SCS TF	-80.00 hrs, dt= R-20 method, l	JH=SCS, W Pond rout Ru Flow Le Run Flow L	601 points /eighted-CN ing by Stor-Ind method ngth=553' Tc=12.0 min C off Area=13,159 sf 38.16% ength=140' Tc=11.3 min (unoff Area=2,105 sf 0.00%	Printed 2/1/2023 Page 4 Impervious Runoff Depth=4.84" N=72 Runoff=9.57 cfs 36,305 cf Impervious Runoff Depth=6.01" CN=82 Runoff=1.74 cfs 6,592 cf Impervious Runoff Depth=4.95" CN=73 Runoff=0.27 cfs 869 cf Inflow=9.57 cfs 36,305 cf

Гуре III		0-rearr	Rainfall=8.1		ieu-Civ,	nine opa	an= 0.00-	30.00 hr	s, dt= 0	.05 hrs					
	Area (sf)		Description												
e .	5,457 76,777	70		od, HSG C											
	7,869 90,103	72	Weighted A			i C									
	84,646 5,457			rvious Area ervious Are											
Tc (min)		(ft/ft)	(ft/sec)	Capacity (cfs)											
8.6 3.4		0.0530 0.0240			Woods: Shallov	Light un v Conce	eet Flow derbrush ntrated F		00 P2	= 3.05"					
12.0	553	Total			Unpave	d Kv=1	16.1 fps								
	ed by {er			name her							Туре	III 24-hr	100-Ye	<i>ar Rainfall=</i> 8 Printed 2/1/2	2023
Prepare	ed by {er			20 HydroCAI	Software			w to Fi	vietino	Basi		III 24-hr	100-Ye	Printed 2/1/2	
Prepare	ed by {er			20 HydroCAI	Software		E-1: Flo	w to E	kisting	g Basi		III 24-hr	100-Yea	Printed 2/1/2	2023
Prepare	ed by {er			20 HydroCAI	Software	hment	E-1: Flo	w to E	cisting	g Basi		24-hr	100-Ye	Printed 2/1/2 Pa	2023 ge 6
Prepare	ed by {er AD® 10.00	-26 s/n 0		20 HydroCAI	Software	hment	E-1: Flo	w to E	cisting	g Basi	n			Printed 2/1/2	2023 ge 6
⊃repare HydroC/ 1	ed by {er AD® 10.00	-26 s/n 0	4015 © 202	20 HydroCAI	Software	hment	E-1: Flo				n Type	111 24	-hr	Printed 2/1/2 Pa	2023 ge 6
Prepara HydroCA	ed by {er AD® 10.00	-26 s/n 0	4015 © 202	20 HydroCAI	Software	hment	E-1: Flo	10	0-Ye	ar l	n Type Rainfa	24 =8.	-hr 16"	Printed 2/1/2 Pa	2023 ge 6
Preparu <u>HydroC</u> A	ed by {er	-26 s/n 0	4015 © 202	20 HydroCAI	Software	hment	E-1: Flo	10 R	0-Ye	ear I	n Type Rainfa rea=9	24 =8. 0,103	-hr 16'' 3 sf	Printed 2/1/2 Pa	2023 ge 6
Preparu <u>HydroC</u> A	ed by {er AD® 10.00	-26 s/n 0	4015 © 202	20 HydroCAI	Software	hment	E-1: Flo	10 R	0-Ye uno off \	ear l off A /olu	⊓ Type Rainfa rea=9 ime=3	24 =8. 0,103 6,305	-hr 16" 3 sf 5 cf	Printed 2/1/2 Pa	2023 ge 6
Prepare HydroCA	ed by {er	-26 s/n 0	4015 © 202	20 HydroCAI	Software	hment	E-1: Flo	10 R	0-Ye uno off \	ear l off A /olu	n Type Rainfa rea=9	24 =8. 0,103 6,305	-hr 16" 3 sf 5 cf	Printed 2/1/2 Pa	2023 ge 6
Prepare HydroCA	ed by {er	-26 s/n 0	4015 © 202	20 HydroCAI	Software	hment	E-1: Flo	10 R	0-Ye uno off \ Ru	ear l ff A /olu inof	n Type Rainfa rea=9 ime=3 f Dept	III 24 II=8. 0,103 6,305 h=4.8	-hr 16" 3 sf 5 cf 84"	Printed 2/1/2 Pa	2023 ge 6
Prepare HydroC/ 1	ed by {er AD® 10.00	-26 s/n 0	4015 © 202	20 HydroCAI	Software	hment	E-1: Flo	10 R	0-Ye uno off \ Ru	ear l ff A /olu inof	⊓ Rainfa rea=9 ime=3 f Dept / Leng	III 24 II=8. 0,103 6,305 h=4.1 jth=5	-hr 16" 3 sf 5 cf 84"	Printed 2/1/2 Pa	2023 ge 6
Prepare HydroC/ 1	ed by {er AD® 10.00 9 8 7 6 5 4	-26 s/n 0	4015 © 202	20 HydroCAI	Software	hment	E-1: Flo	10 R	0-Ye uno off \ Ru	ear l ff A /olu inof	⊓ Rainfa rea=9 ime=3 f Dept / Leng	III 24 II=8. 0,103 6,305 h=4.1 jth=5 2.0 r	-hr 16" 3 sf 5 cf 84" 53' nin	Printed 2/1/2 Pa	2023 ge 6
Prepare HydroC/ 1	ed by {er AD® 10.00	-26 s/n 0	4015 © 202	20 HydroCAI	Software	hment	E-1: Flo	10 R	0-Ye uno off \ Ru	ear l off A /olu inof	⊓ Rainfa rea=9 ime=3 f Dept / Leng	III 24 II=8. 0,103 6,305 h=4.1 jth=5	-hr 16" 3 sf 5 cf 84" 53' nin	Printed 2/1/2 Pa	2023 ge 6
Prepare HydroC/ 1 I	ed by {er AD® 10.00 9 8 7 6 5 4	-26 s/n 0	4015 © 202	20 HydroCAI	Software	hment	E-1: Flo	10 R	0-Ye uno off \ Ru	ear l off A /olu inof	⊓ Rainfa rea=9 ime=3 f Dept / Leng	III 24 II=8. 0,103 6,305 h=4.1 jth=5 2.0 r	-hr 16" 3 sf 5 cf 84" 53' nin	Printed 2/1/2 Pa	2023 ge 6

Summary for Subcatchment E-2: Flow onsite Southeast

Runoff = 1.74 cfs @ 12.16 hrs, Volume= 6,592 cf, Depth= 6.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs Type III 24-hr 100-Year Rainfall=8.16" $\,$

	A	rea (sf)	CN	Description				
	*	5.022	98	Paved parking				
		6.746		Woods, Good, HSG C				
		1,391				Fair, HSG C		
-		13,159		Weighted A				
		8.137			vious Area			
		5,022			pervious Are			
		-,						
	Тс	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)		(cfs)			
-	11.0	50	0.0280	0.08		Sheet Flow.		
						Woods: Light underbrush n= 0.400 P2= 3.05"		
	0.1	45	0.1750	6.74		Shallow Concentrated Flow,		
						Unpaved Kv= 16.1 fps		
	0.0	11	0.1640	6.52		Shallow Concentrated Flow,		
						Unpaved Kv= 16.1 fps		
	0.2	34	0.0240	3.14		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps		
	11.3	140	Total					

Type III 24-hr 100-Year Rainfall=8.16" Existing-2 Printed 2/1/2023 Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 8 Subcatchment E-2: Flow onsite Southeast Hydrograph Runoff 1.74 cfs Type III 24-hr 100-Year Rainfall=8.16" Runoff Area=13,159 sf Runoff Volume=6,592 cf Runoff Depth=6.01" Flow (cfs) Flow Length=140' Tc=11.3 min **CN=82** 0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 74 76 78 80 Time (hours)

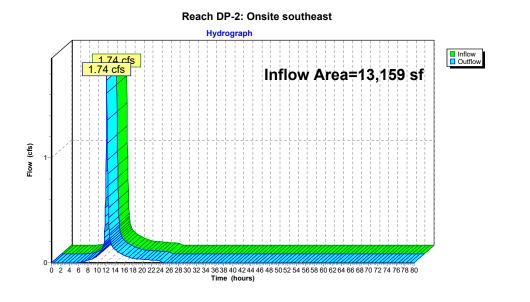
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Inflow Area = 90,103 sf, 6.06% Impervious, Inflow Depth = 4.84" for 100-Year event Inflow = 9.57 cfs @ 12.17 hrs, Volume= 36,305 cf Outflow = 9.57 cfs @ 12.17 hrs, Volume= 36,305 cf, Atten= 0%, Lag= 0.0 min Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs Reach DP-1: Basin Hydrograph	<pre>ifiow Area = 90,103 sf, 6.06% Impervious, Inflow Depth = 4.84" for 100-Year event</pre>	nflow Area = 90,103 sf, 6.06% Impervious, Inflow Depth = 4.84" for 100-Year event nflow = 9.57 cfs @ 12.17 hrs, Volume= 36,305 cf Dutflow = 9.57 cfs @ 12.17 hrs, Volume= 36,305 cf, Atten= 0%, Lag= 0.0 min Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs Reach DP-1: Basin Hydrograph 1000 100 100 100 100 100 100 100 100 10
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Outflow = 9.57 cfs @ 12.17 hrs, Volume= 36,305 cf, Atten= 0%, Lag= 0.0 min Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs Reach DP-1: Basin Hydrograph	utflow = 9.57 cfs @ 12.17 hrs, Volume= 36,305 cf, Atten= 0%, Lag= 0.0 min outing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs Reach DP-1: Basin Hydrograph 0 0.57 cfs 0 0.57 cfs	Dutflow = 9.57 cfs @ 12.17 hrs, Volume= 36,305 cf, Atten= 0%, Lag= 0.0 min Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs Reach DP-1: Basin Hydrograph 0 0 57 cfs 0 0 57 cfs 0 0 57 cfs 0 0 0 57 cfs 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
Reach DP-1: Basin Hydrograph	Reach DP-1: Basin Hydrograph	Reach DP-1: Basin Hydrograph
Hydrograph	Hydrograph	Hydrograph (9) of graph (9)
	(y) MP (y)	(9) Mg 4 3 (9) Mg 4 3 (9) Mg 4 3 (9) Mg 4 3 (9) Mg 4 3 (9) Mg 4 3 (9) Mg 4 3 (9) Mg (9) Mg 4 3 (9) Mg (9) Mg
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1 ZI J J <mark>9 57 CIS</mark> I F F F F F F F F F F F F F F F F F F	9.57 CTS 9 8 7 (y) Mg 5 4 3 10 10 10 10 10 10 10 10 10 10	(g) (g) (g) (g) (g) (g) (g) (g)
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	(i) wolf i i i i i i i i i i i i i i i i i i i	NOL STREET STREE
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9 8 7	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
9 8 7		
9 8 7		
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		9 9 8 7 7 6 6 4 3 7 7 7 7 7 7 7 7 7 7 7 7 7
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	(i) wolf i i i i i i i i i i i i i i i i i i i	7 6 6 5 4 3
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	(i) wolf i i i i i i i i i i i i i i i i i i i	NOL STREET STREE
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⁹	(j) MOIE 4 3	(§) MOL 4 3
⁹	4	4 3 3 3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5
⁹	4	4 3 3 3 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5
	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
9 8 7	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
9 8 7	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
9 8 7	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
9 8 7	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
9 8 7	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
9 8 7	4	4 3 3 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
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(s;) more 4		
(s) (s) (s) (s) (s) (s) (s) (s)		

Summary for Reach DP-2: Onsite southeast

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	13,159 sf, 38.16% Impervious, Inflow Depth = 6.01" for 100-Year event	
Inflow	=	1.74 cfs @ 12.16 hrs, Volume= 6,592 cf	
Outflow	=	1.74 cfs @ 12.16 hrs, Volume= 6,592 cf, Atten= 0%, Lag= 0.0 min	

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs



Existing-2

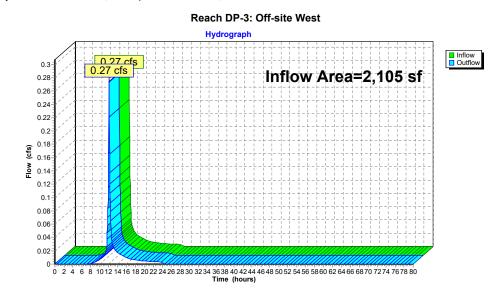
Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Type III 24-hr 100-Year Rainfall=8.16" Printed 2/1/2023 Page 12

Summary for Reach DP-3: Off-site West

[40] Hint: Not Described (Outflow=Inflow)

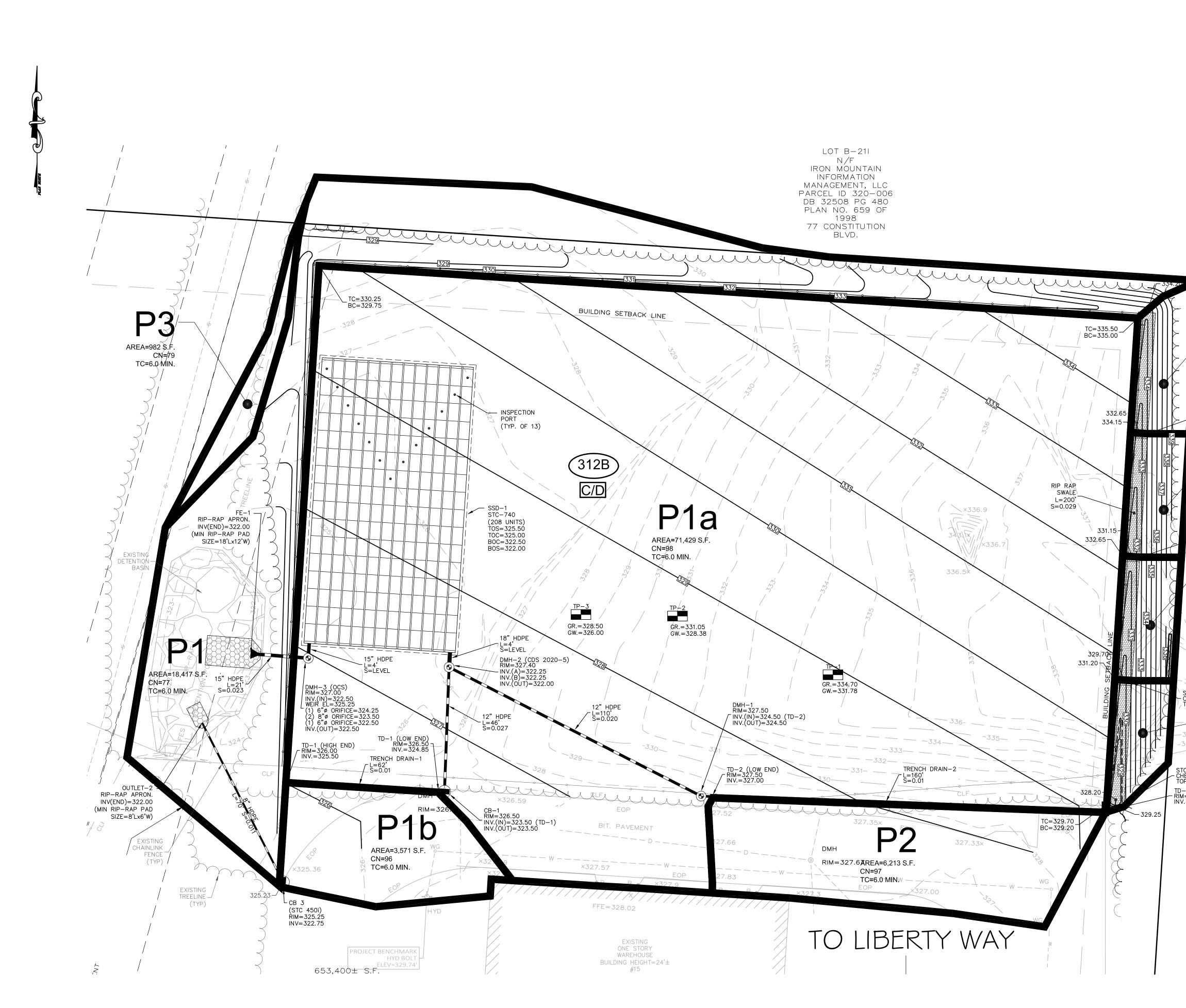
Inflow Area	a =	2,105 sf,	0.00% Impervious,	Inflow Depth = 4.95"	for 100-Year event
Inflow	=	0.27 cfs @ 1	12.09 hrs, Volume=	869 cf	
Outflow	=	0.27 cfs @ 1	12.09 hrs, Volume=	869 cf, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs





Proposed Drainage Plan & HydroCAD Diagram



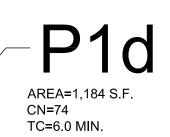
WATERSHED LEGEND



SOIL TYPE [X]

TC=6.0 MIN.

TONE GABION HECK DAM DP EL.=334.05



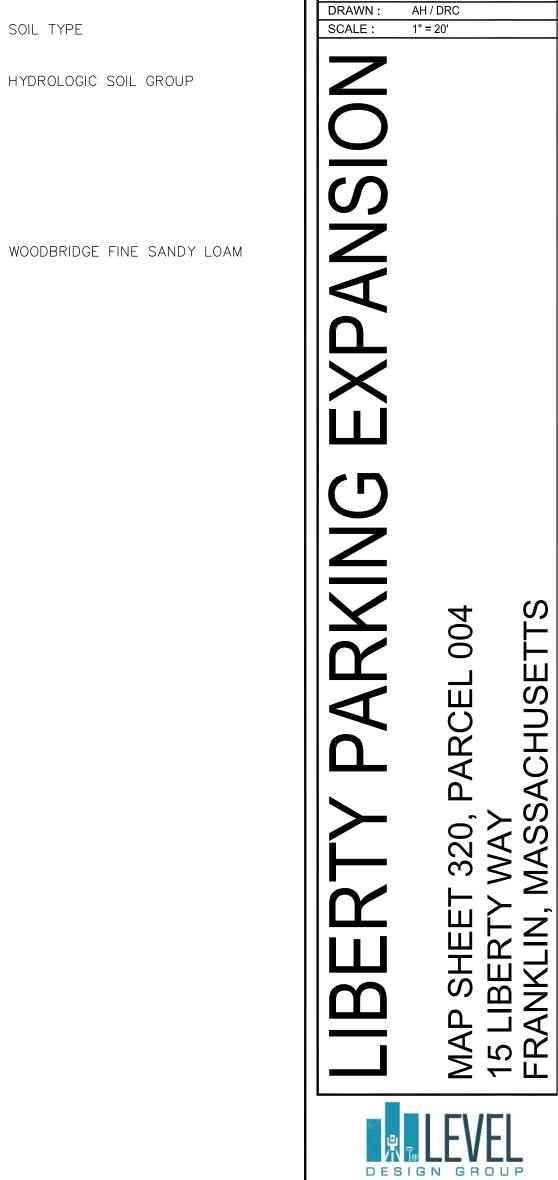
STONE GABION - CHECK DAM TOP EL.=332.50

P1e AREA=1,184 S.F. QN=75

TC=6.0 MIN.



STONE GABION CHECK DAM TOP EL.=329.50 TD-2 (HIGH END) - RIM=329.15 INV.=328.60



ACHUSETTS

NO DATE

SEAL

DATE :

JANUARY 13, 2023

REVISIONS

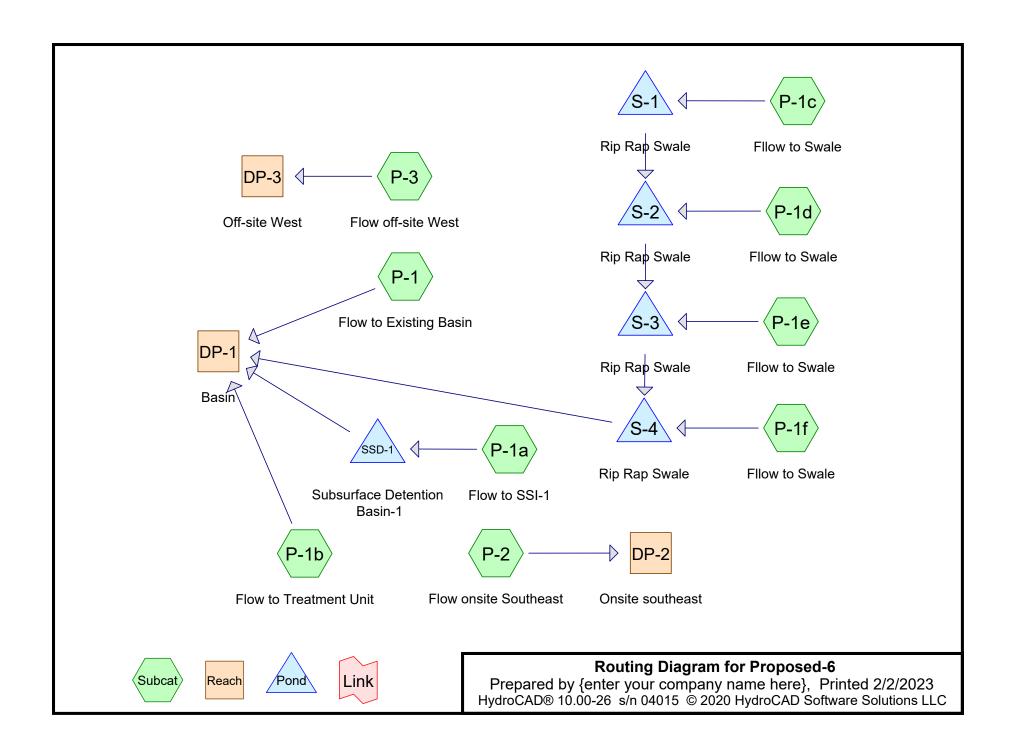




HYDROLOGIC SOIL GROUP

SOILS

(312B)





HydroCAD Analysis

Proposed Conditions - 2 Year Storm

Proposed-6
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	Page 1

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
18,555	79	50-75% Grass cover, Fair, HSG C (P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3)
80,078	98	Paved parking (P-1a, P-1b, P-2)
1,396	66	Rip Rap Swale (P-1c, P-1d, P-1e, P-1f)
5,338	70	Woods, Good, HSG C (P-1, P-1c, P-1d, P-1e, P-1f, P-3)
105,367	93	TOTAL AREA

Area Listing (all nodes)

Soil Listing (all nodes)

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Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
0	HSG A	
0	HSG B	
23,893	HSG C	P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3
0	HSG D	
81,474	Other	P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2
105,367		TOTAL AREA

Proposed-6
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			Gro	und Covers	(all nodes))	
 HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Subcatchment Numbers
 0	0	18,555	0	0	18,555	50-75% Grass cover, Fair	P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3
0	0	0	0	80,078	80,078	Paved parking	P-1a, P-1b, P-2
0	0	0	0	1,396	1,396	Rip Rap Swale	P-1c, P-1d, P-1e, P-1f
0	0	5,338	0	0	5,338	Woods, Good	P-1, P-1c, P-1d, P-1e, P-1f, P-3
0	0	23,893	0	81,474	105,367	TOTAL AREA	

Proposed-6 Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC

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					Pip	e Listin	g (all nodes	5)		
Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)	
1	SSD-1	322.50	322.00	21.3	0.0235	0.013	15.0	0.0	0.0	

Time span=0.00-80.00 hrs, dt=0.05 hrs, 1601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment P-1: Flow to Existing Basin	Runoff Area=18,417 sf 0.00% Impervious Runoff Depth=1.11" Flow Length=461' Tc=10.0 min CN=77 Runoff=0.46 cfs 1,697 cf
Subcatchment P-1a: Flow to SSI-1	Runoff Area=71,428 sf 99.59% Impervious Runoff Depth=2.82" Tc=6.0 min CN=98 Runoff=4.74 cfs 16,774 cf
Subcatchment P-1b: Flow to Treatment Unit	Runoff Area=3,571 sf 87.17% Impervious Runoff Depth=2.60" Tc=6.0 min CN=96 Runoff=0.23 cfs 774 cf
SubcatchmentP-1c: Fllow to Swale	Runoff Area=1,317 sf 0.00% Impervious Runoff Depth=0.84" Tc=6.0 min CN=72 Runoff=0.03 cfs 92 cf
SubcatchmentP-1d: Fllow to Swale	Runoff Area=1,184 sf 0.00% Impervious Runoff Depth=0.94" Tc=6.0 min CN=74 Runoff=0.03 cfs 93 cf
SubcatchmentP-1e: Fllow to Swale	Runoff Area=1,184 sf 0.00% Impervious Runoff Depth=0.99" Tc=6.0 min CN=75 Runoff=0.03 cfs 98 cf
Subcatchment P-1f: Fllow to Swale	Runoff Area=1,071 sf 0.00% Impervious Runoff Depth=0.94" Tc=6.0 min CN=74 Runoff=0.03 cfs 84 cf
Subcatchment P-2: Flow onsite Southeast	Runoff Area=6,213 sf 93.87% Impervious Runoff Depth=2.71" Tc=6.0 min CN=97 Runoff=0.41 cfs 1,402 cf
Subcatchment P-3: Flow off-site West	Runoff Area=982 sf 0.00% Impervious Runoff Depth=1.23" Tc=6.0 min CN=79 Runoff=0.03 cfs 100 cf
Reach DP-1: Basin	Inflow=1.40 cfs 17,732 cf Outflow=1.40 cfs 17,732 cf
Reach DP-2: Onsite southeast	Inflow=0.41 cfs 1,402 cf Outflow=0.41 cfs 1,402 cf

Proposed-6 Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC	Type III 24-hr 2-Year Rainfall=3.05" Printed 2/2/2023 Page 6
Reach DP-3: Off-site West	Inflow=0.03 cfs 100 cf Outflow=0.03 cfs 100 cf
Pond S-1: Rip Rap Swale	Peak Elev=333.17' Storage=92 cf Inflow=0.03 cfs 92 cf Outflow=0.00 cfs 0 cf
Pond S-2: Rip Rap Swale	Peak Elev=331.73' Storage=93 cf Inflow=0.03 cfs 93 cf Outflow=0.00 cfs 0 cf
Pond S-3: Rip Rap Swale	Peak Elev=330.29' Storage=98 cf Inflow=0.03 cfs 98 cf Outflow=0.00 cfs 0 cf
Pond S-4: Rip Rap Swale	Peak Elev=328.70' Storage=84 cf Inflow=0.03 cfs 84 cf Outflow=0.00 cfs 0 cf
Pond SSD-1: Subsurface Detention Basin-1	Peak Elev=323.63' Storage=8,316 cf Inflow=4.74 cfs 16,774 cf Outflow=1.01 cfs 15,261 cf

Total Runoff Area = 105,367 sf Runoff Volume = 21,114 cf Average Runoff Depth = 2.40" 24.00% Pervious = 25,289 sf 76.00% Impervious = 80,078 sf

		R-20 me			ume= 1,697 cf, Depth= 1.11" nted-CN, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs	
			nfall=3.05"			
A	Area (sf) 4,454	70	Description Woods, Go	od, HSG C		
	<u>13,963</u> 18,417		50-75% Gra Weighted A		Fair, HSG C	
	18,417		100.00% P			
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description	
7.3	50	0.0800	· · · · · · · · · · · · · · · · · · ·		Sheet Flow, Sheet Flow Woods: Light underbrush n= 0.400 P2= 3.05"	
2.7	411	0.0240) 2.49		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
10.0	461	Total			especta contrapo	•
					Type III 24-hr 2-Year Rainfall=3.05"	
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Prepare	ed by {en	1ter you -26 s/n (r company 14015 © 202	0 HydroCAE	e} Printed 2/2/2023 D Software Solutions LLC Page 8	6
Prepare HydroCA	ed by {en	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} Printed 2/2/2023 D Software Solutions LLC Page 8 Subcatchment P-1: Flow to Existing Basin	6
Prepare HydroCA	ed by {en AD® 10.00	<u>-26 s/n (</u>	r company 14015 © 202 .46 cfs	0 HydroCAE	e} Printed 2/2/2023 Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph	6
Prepare HydroCA 0 0 0	ed by {en AD® 10.00	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} Printed 2/2/2023 D Software Solutions LLC Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph	6
Prepare HydroCA 0 0 0 0	ed by {en AD® 10.00	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} Printed 2/2/2023 D Software Solutions LLC Printed 2/2/2023 Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph Runoff Type III 24-hr 2-Year Rainfall=3.05"	6
Prepare HydroCA 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ed by {en 10.5 1	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} Printed 2/2/2023 D Software Solutions LLC Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph	6
Prepare HydroC# 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.5	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} Printed 2/2/2023 Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph Type III 24-hr 2-Year Rainfall=3.05" Runoff Area=18,417 sf	6
Prepare HydroC# 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.5 .48 .44 .44 .44 .44 .38 .34 .34 .34 .34 .34 .34 .34 .34	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} Printed 2/2/2023 Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph Printed 2/2/2023 Page 8 Contemporation State	6
Prepare HydroCA 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} Printed 2/2/2023 Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph	6
Flow (cfs) 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ed by {en D® 10.00 0.5 .48 .46 .44 .44 .44 .38 .36 .34 .34 .22 .22 .24 .24 .24 .24 .24 .2	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} Printed 2/2/2023 Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph Type III 24-hr 2-Year Rainfall=3.05" Runoff Area=18,417 sf Runoff Volume=1,697 cf Runoff Depth=1.11" Flow Length=461	6
Prepare HydroCA 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.5	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} Printed 2/2/2023 Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph	6
Prepare HydroCA 1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ad by (en <u>D</u> ® 10.00 0.5 48 46 	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} D Software Solutions LLC Subcatchment P-1: Flow to Existing Basin Hydrograph Runoff Runoff Area=18,417 sf Runoff Volume=1,697 cf Runoff Depth=1.11" Flow Length=461' Tc=10.0 min	6
Prepare <u>HydroC</u> <u>HydroC</u> 0 0 0 0 0 0 0 0 0 0 0 0 0	ad by (en <u>D</u> ® 10.00 0.5 48 46 44 44 44 - - 44 - - - - - - - - - - - - -	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} Printed 2/2/2023 Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph Type III 24-hr 2-Year Rainfall=3.05" Runoff Area=18,417 sf Runoff Volume=1,697 cf Runoff Depth=1.11" Flow Length=461	6
Prepare <u>HydroCA</u> 0 0 0 0 0 0 0 0 0 0 0 0 0	ed by {en D® 10.00 0.5 .4 .4 .4 .4 .4 .4 .4 .4 .4 .4	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} D Software Solutions LLC Subcatchment P-1: Flow to Existing Basin Hydrograph Runoff Runoff Area=18,417 sf Runoff Volume=1,697 cf Runoff Depth=1.11" Flow Length=461' Tc=10.0 min	6
Prepare <u>HydroC</u> 0 0 0 0 0 0 0 0 0 0 0 0 0	ed by {en <u>D</u> ⊕ 10.00 0.5	<u>-26 s/n (</u>	04015 © 202	0 HydroCAE	e} D Software Solutions LLC Subcatchment P-1: Flow to Existing Basin Hydrograph Runoff Runoff Area=18,417 sf Runoff Volume=1,697 cf Runoff Depth=1.11" Flow Length=461' Tc=10.0 min	6

	-	r for Subcatchment P-1a: Flow to SSI-1
unoff =	4.74 cfs @ 12.09 hrs, Volume=	16,774 cf, Depth= 2.82"
noff by SCS TR	20 method, UH=SCS, Weighted-CN ar Rainfall=3.05"	N, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs
Area (sf)	CN Description	
71,133 295	98 Paved parking79 50-75% Grass cover, Fair, HS	SG C
71,428 295	98 Weighted Average 0.41% Pervious Area	
71,133	99.59% Impervious Area	
Tc Length (min) (feet)	Slope Velocity Capacity Desc (ft/ft) (ft/sec) (cfs)	ription
6.0		ct Entry, Min.
roposed-6	r your company name here}	Type III 24-hr 2-Year Rainfall=3. Printed 2/2/20
epared by {enter	r your company name here} § s/n 04015 © 2020 HydroCAD Softwa	Printed 2/2/20
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epared by {enter	<u>š š/n 04015 © 2020 HydroCAD Softwa</u> Su	are Solutions LLC Printed 2/2/20 Page bcatchment P-1a: Flow to SSI-1
epared by {enter	S s/n 04015 © 2020 HydroCAD Softwa	Printed 2/2/20 Page bcatchment P-1a: Flow to SSI-1 Hydrograph
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separed by {entor droCAD® 10.00-2	<u>š š/n 04015 © 2020 HydroCAD Softwa</u> Su	Printed 2/2/20 Page bcatchment P-1a: Flow to SSI-1 Hydrograph Type III 24-hr 2-Year Rainfall=3.05" Runoff Area=71,428 sf
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<u>A</u>	Area (sf) 339		Descripti Rip Rap													
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	1,317 1,317 1,317		Weighted 100.00%	l Avera	ge											
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Inoff						•		ment P-1d: Flle	ow to Swale		
	=		ofs @ 12					Depth= 0.94"			
	by SCS TF 24-hr 2-Y				S, Weigl	nted-CN, T	lime Span= 0.	00-80.00 hrs, dt=	0.05 hrs		
A	Area (sf)		Descripti								
	350 216		Rip Rap Woods, (Good	, HSG C						
	<u>618</u> 1,184		50-75% Weighte			Fair, HSG	C				
	1,184		100.00%	Perv	ious Are	ea					
Tc (min)	Length (feet)	Slope (ft/ft			apacity (cfs)	Descript	tion				
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epare	ed by {en	ter you 26 s/n (r compa 4015 © 2	ny na 2020 F	ime her	D Software	Solutions LLC	.1d: Ellow to S		pe III 24-hr 2-Y	
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	=		-		hrs, V					Depth= 0.9		5 h m						
	24-hr 2-1				,5, vve	igntea-	-CN, I	ime Sp	ban= 0.0	0-80.00 hrs	, at= 0.0	5 nrs						
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Runoff by SCS 7	0.03 cfs @ 12.10 hrs, Volume= 100 cf, Depth= 1.23"
	TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs -Year Rainfall=3.05"
Area (sf)	
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	Subcatchment P-3: Flow off-site West
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Proposed-6	Type III 24-hr 2-Year Rainfall=3.05"
Prepared by {e	Type III 24-hr 2-Year Rainfall=3.05" enter your company name here} 00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 24
Prepared by {e	enter your company name here} Printed 2/2/2023
Prepared by {e lydroCAD® 10.0	enter your company name here} 2/2/2023 20-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 24
Prepared by {e HydroCAD® 10.0 40] Hint: Not De nflow Area =	enter your company name here} <u>D-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC</u> Summary for Reach DP-1: Basin escribed (Outflow=Inflow) 98,172 sf, 75.63% Impervious, Inflow Depth > 2.17" for 2-Year event
Prepared by {e <u>lydroCAD® 10.0</u> 40] Hint: Not De	enter your company name here} 2/2/2023 20-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Summary for Reach DP-1: Basin escribed (Outflow=Inflow)
Prepared by {e HydroCAD® 10.0 40] Hint: Not De nflow Area = nflow = Dutflow =	enter your company name here} <u>Printed 2/2/2023</u> <u>Page 24</u> <u>Summary for Reach DP-1: Basin</u> escribed (Outflow=Inflow) <u>98,172 sf, 75.63% Impervious, Inflow Depth > 2.17" for 2-Year event</u> <u>1.40 cfs @ 12.15 hrs, Volume= 17,732 cf</u>
Prepared by {e HydroCAD® 10.0 40] Hint: Not De nflow Area = nflow = Dutflow =	enter your company name here} Printed 2/2/2023 00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 24 Summary for Reach DP-1: Basin escribed (Outflow=Inflow) 98,172 sf, 75.63% Impervious, Inflow Depth > 2.17" for 2-Year event 1.40 cfs @ 12.15 hrs, Volume= 17,732 cf 1.40 cfs @ 12.15 hrs, Volume= 17,732 cf, Atten= 0%, Lag= 0.0 min

Summary for Reach DP-2: Onsite southeast

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	6,213 sf, 93.87% Impervious, Inflow Depth = 2.71" for 2-Year ever	nt
Inflow	=	0.41 cfs @ 12.09 hrs, Volume= 1,402 cf	
Outflow	=	0.41 cfs @ 12.09 hrs, Volume= 1,402 cf, Atten= 0%, Lag= 0.0	min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs Reach DP-2: Onsite southeast Hydrograph Inflow
Outflow 0.41 cfs 0.41 cfs Inflow Area=6,213 sf 0.42 0.4 0.38 0.34 0.3 0.28 (j) 0.26 0.24 0.22 Flov 0.2 0.18 0.16 0.14 0.12 0.1 0.08 0.06 0.04 0.02 0-0 2 4 6 8 1012 1416 18 2022 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 74 76 78 80 Time (hours)

Proposed-6

Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC

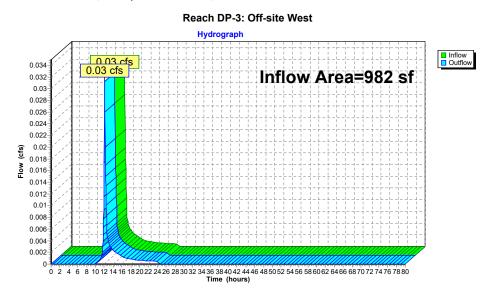
Type III 24-hr 2-Year Rainfall=3.05" Printed 2/2/2023 Page 26

Summary for Reach DP-3: Off-site West

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	982 sf,	0.00% Impervious,	Inflow Depth = 1	.23" for 2-Year event
Inflow	=	0.03 cfs @	12.10 hrs, Volume=	100 cf	
Outflow	=	0.03 cfs @	12.10 hrs, Volume=	100 cf,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs



Summary for Pond S-1: Rip Rap Swale

Inflow Area =	1,317 sf, 0.00% Impervious,	Inflow Depth = 0.84" for 2-Year event
Inflow =	0.03 cfs @ 12.10 hrs, Volume=	92 cf
Outflow =	0.00 cfs @ 0.00 hrs, Volume=	0 cf, Atten= 100%, Lag= 0.0 min
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 333.17' @ 24.40 hrs Surf.Area= 200 sf Storage= 92 cf

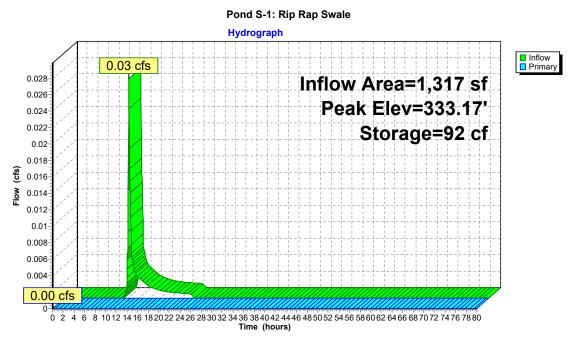
Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no outflow)

/olume	Invert	Avail.Storage	Storage Description	า			
#1	331.15'	120 cf	Custom Stage Dat 414 cf Overall - 114			ds	
#2	332.65'	114 cf	Custom Stage Dat	a (Irregular)Listed	below (Recalc) I	nside #1	
		234 cf	Total Available Sto				
Elevation (feet)	Surf.A	area Perim. q-ft) (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
331.15	(0 0.0	0	0	0		
331.50		21 27.2	2	2	59		
332.50		171 99.3	84	86	788		
332.65		200 104.4	28	114	872		
334.15		200 104.4	300	414	1,028		
Elevation	Surf.A	rea Perim.	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sc	q-ft) (feet)	(cubic-feet)	(cubic-feet)	(sq-ft)		
332.65		0 0.0	0	0	0		
333.00		21 27.2	2	2	59		
334.00		171 99.3	84	86	788		
334.15		200 104.4	28	114	872		

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Type III 24-hr 2-Year Rainfall=3.05" Printed 2/2/2023 Page 28

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=331.15' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)



Summary for Pond S-2: Rip Rap Swale

Inflow Area =	2,501 sf, 0.00% Impervious,	Inflow Depth = 0.45" for 2-Year event
Inflow =	0.03 cfs @ 12.10 hrs, Volume=	93 cf
Outflow =	0.00 cfs @ 0.00 hrs, Volume=	0 cf, Atten= 100%, Lag= 0.0 min
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 331.73' @ 24.40 hrs Surf.Area= 200 sf Storage= 93 cf

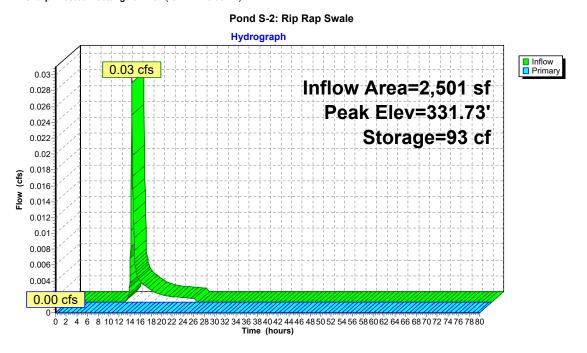
Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no outflow)

#1	329.65'	120 cf	Custom Stage Dat	ta (Irregular) isted	helow (Recalc)		
"	020.00	120 01	407 cf Overall - 107				
#2	331.15'	107 cf		istom Stage Data (Irregular)Listed below (Recalc) Inside #1			
		227 cf	Total Available Sto		· · · · ·		
Elevation	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft) (feet)	(cubic-feet)	(cubic-feet)	(sq-ft)		
329.65	(0.0	0	0	0		
330.50	74	61.1	21	21	298		
331.15	200		86	107	873		
332.65	200) 104.5	300	407	1,029		
Elevation	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft) (feet)	(cubic-feet)	(cubic-feet)	(sq-ft)		
331.15	(0.0	0	0	0		
332.00	74		21	21	298		
332.65	200) 104.5	86	107	873		
	a stration of	Inviorit Out	et Devices				
	5				a = 1 a 1		
#1 Pr	imary 33	32.50' 7.0'	long Sharp-Crested	d Rectangular Wei	r 2 End Contrac		

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Type III 24-hr 2-Year Rainfall=3.05" Printed 2/2/2023 Page 30

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=329.65' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)



Summary for Pond S-3: Rip Rap Swale

Inflow Area =	3,685 sf, 0.00% Impervious,	Inflow Depth = 0.32" for 2-Year event
Inflow =	0.03 cfs @ 12.10 hrs, Volume=	98 cf
Outflow =	0.00 cfs @ 0.00 hrs, Volume=	0 cf, Atten= 100%, Lag= 0.0 min
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 330.29' @ 24.40 hrs Surf.Area= 200 sf Storage= 98 cf

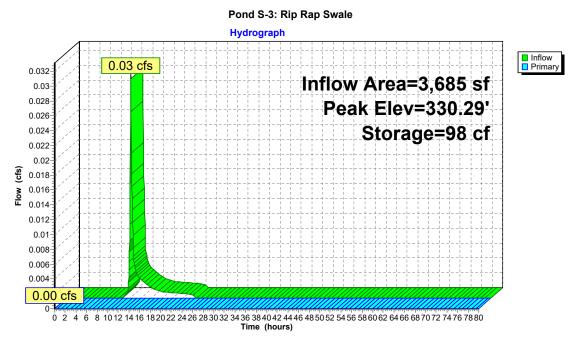
Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no outflow)

#1 #2	329.70' 112 cf Custom Stage Data (Irregular)Listed below (Recalc) 328.20' 120 cf Custom Stage Data (Irregular)Listed below (Recalc) 412 cf Overall - 112 cf Embedded = 300 cf x 40.0% Voltage					
		232 cf	Total Available Stor	rage		
Elevation	Surf.Area		Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)		(cubic-feet)	(cubic-feet)	(sq-ft)	
329.70	0	0.0	0	0	0	
330.00	16	23.0	2	2	42	
331.00	158		75	76	722	
331.20	200	104.5	36	112	873	
Elevation	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
328.20	0	0.0	0	0	0	
328.50	16	23.0	2	2	42	
329.50	158	95.1	75	76	722	
329.70	200	104.5	36	112	873	
331.20	200	104.5	300	412	1,030	
Device R	outing I	nvert Outl	et Devices			

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Type III 24-hr 2-Year Rainfall=3.05" Printed 2/2/2023 Page 32

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=328.20' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)



Summary for Pond S-4: Rip Rap Swale

Inflow Area =	4,756 sf, 0.00% Impervious,	Inflow Depth = 0.21" for 2-Year event
Inflow =	0.03 cfs @ 12.10 hrs, Volume=	84 cf
Outflow =	0.00 cfs @ 0.00 hrs, Volume=	0 cf, Atten= 100%, Lag= 0.0 min
Primary =	0.00 cfs @ 0.00 hrs, Volume=	0 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 328.70' @ 24.40 hrs Surf.Area= 188 sf Storage= 84 cf

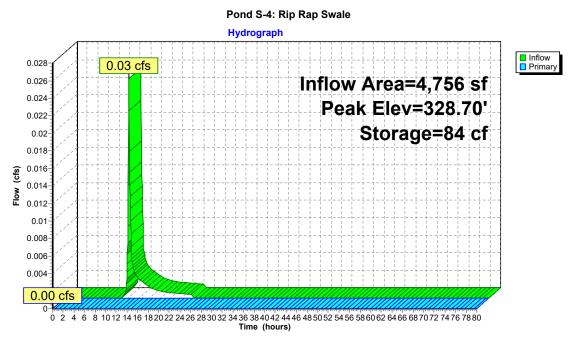
Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no outflow)

#1 #2	328.20' 326.70'	108 cf 113 cf	Custom Stage Dat Custom Stage Dat 390 cf Overall - 108	ta (Irregular)Listed	below (Recalc)		
		221 cf	Total Available Stor	rage			
Elevation (feet)	Surf.Area (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
328.20 329.00	0 71		0 19	0 19	0 259		
329.25 329.70	112 188	76.4	23 67	42 108	466 832		
Elevation (feet)	Surf.Area (sq-ft)	Perim.	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
326.70	0		0	0	0		
327.50 327.75	71 112		19 23	19 42	259 466		
328.20 329.70	188 188		67 282	108 390	832 985		
Device Ro	outing I	nvert Outl	et Devices				
#1 Pr	imary 32	9.25' 7.0'	long Sharp-Crested	l Rectangular Wei	r 2 End Contracti	on(s)	

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Type III 24-hr 2-Year Rainfall=3.05" Printed 2/2/2023 Page 34

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=326.70' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Controls 0.00 cfs)



Summary for Pond SSD-1: Subsurface Detention Basin-1

Inflow Area =	71,428 sf, 99.59% Impervious,	Inflow Depth = 2.82" for 2-Year event
Inflow =	4.74 cfs @ 12.09 hrs, Volume=	16,774 cf
Outflow =	1.01 cfs @ 12.50 hrs, Volume=	15,261 cf, Atten= 79%, Lag= 24.8 min
Primary =	1.01 cfs @ 12.50 hrs, Volume=	15,261 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 9 Peak Elev= 323.63' @ 12.50 hrs Surf.Area= 7,434 sf Storage= 8,316 cf

Plug-Flow detention time= 234.6 min calculated for 15,261 cf (91% of inflow) Center-of-Mass det. time= 188.6 min (946.0 - 757.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	322.00'	6,586 cf	63.25'W x 117.54'L x 3.50'H Field A
			26,020 cf Overall - 9,556 cf Embedded = 16,464 cf x 40.0% Voids
#2A	322.50'	9,556 cf	ADS_StormTech SC-740 +Cap x 208 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			208 Chambers in 13 Rows
		16,141 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	322.50'	15.0" Round Culvert L= 21.3' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 322.50' / 322.00' S= 0.0235 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf
#2	Device 1	325.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Device 1	324.25'	6.0" Vert. Orifice/Grate C= 0.600
#4	Device 1	323.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
#5	Device 1	322.50'	6.0" Vert. Orifice/Grate C= 0.600

Proposed-6

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Type III 24-hr 2-Year Rainfall=3.05" Printed 2/2/2023 Page 36

Primary OutFlow Max=1.01 cfs @ 12.50 hrs HW=323.63' (Free Discharge) 1=Culvert (Passes 1.01 cfs of 4.23 cfs potential flow) 2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs) 3=Orifice/Grate (Controls 0.00 cfs) 4=Orifice/Grate (Orifice Controls 0.12 cfs @ 1.23 fps) 5=Orifice/Grate (Orifice Controls 0.89 cfs @ 4.52 fps)

Type III 24-hr 2-Year Rainfall=3.05"

Pond SSD-1: Subsurface Detention Basin-1 - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length) Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

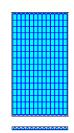
16 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 115.54' Row Length +12.0" End Stone x 2 = 117.54' Base Length 13 Rows x 51.0" Wide + 6.0" Spacing x 12 + 12.0" Side Stone x 2 = 63.25' Base Width 6.0" Base + 30.0" Chamber Height + 6.0" Cover = 3.50' Field Height

208 Chambers x 45.9 cf = 9,555.5 cf Chamber Storage

26,019.7 cf Field - 9,555.5 cf Chambers = 16,464.2 cf Stone x 40.0% Voids = 6,585.7 cf Stone Storage

Chamber Storage + Stone Storage = 16,141.2 cf = 0.371 af Overall Storage Efficiency = 62.0% Overall System Size = 117.54' x 63.25' x 3.50'

208 Chambers 963.7 cy Field 609.8 cy Stone



Proposed-6

Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Printed 2/2/2023 Page 38 Pond SSD-1: Subsurface Detention Basin-1 Hydrograph Inflow 4.74 cfs 5 Inflow Area=71,428 sf Peak Elev=323.63' 4 Storage=8,316 cf 3 Flow (cfs) 2 1.01 cfs 0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 74 76 78 80 Time (hours)



HydroCAD Analysis

Proposed Conditions - 10 Year Storm

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	Page 1

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
18,555	79	50-75% Grass cover, Fair, HSG C (P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3)
80,078	98	Paved parking (P-1a, P-1b, P-2)
1,396	66	Rip Rap Swale (P-1c, P-1d, P-1e, P-1f)
5,338	70	Woods, Good, HSG C (P-1, P-1c, P-1d, P-1e, P-1f, P-3)
105,367	93	TOTAL AREA

Area Listing (all nodes)

Soil Listing (all nodes)

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Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
0	HSG A	
0	HSG B	
23,893	HSG C	P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3
0	HSG D	
81,474	Other	P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2
105,367		TOTAL AREA

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			Gro	und Covers	(all nodes))	
 HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Subcatchment Numbers
 0	0	18,555	0	0	18,555	50-75% Grass cover, Fair	P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3
0	0	0	0	80,078	80,078	Paved parking	P-1a, P-1b, P-2
0	0	0	0	1,396	1,396	Rip Rap Swale	P-1c, P-1d, P-1e, P-1f
0	0	5,338	0	0	5,338	Woods, Good	P-1, P-1c, P-1d, P-1e, P-1f, P-3
0	0	23,893	0	81,474	105,367	TOTAL AREA	

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					Pip	e Listin	g (all nodes	5)		
Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)	
1	SSD-1	322.50	322.00	21.3	0.0235	0.013	15.0	0.0	0.0	

Time span=0.00-80.00 hrs, dt=0.05 hrs, 1601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment P-1: Flow to Existing Basin	Runoff Area=18,417 sf 0.00% Impervious Runoff Depth=2.75" Flow Length=461' Tc=10.0 min CN=77 Runoff=1.18 cfs 4,219 cf
Subcatchment P-1a: Flow to SSI-1	Runoff Area=71,428 sf 99.59% Impervious Runoff Depth=4.91" Tc=6.0 min CN=98 Runoff=8.07 cfs 29,243 cf
Subcatchment P-1b: Flow to Treatment Unit	Runoff Area=3,571 sf 87.17% Impervious Runoff Depth=4.68" Tc=6.0 min CN=96 Runoff=0.40 cfs 1,393 cf
Subcatchment P-1c: Fllow to Swale	Runoff Area=1,317 sf 0.00% Impervious Runoff Depth=2.31" Tc=6.0 min CN=72 Runoff=0.08 cfs 254 cf
Subcatchment P-1d: Fllow to Swale	Runoff Area=1,184 sf 0.00% Impervious Runoff Depth=2.48" Tc=6.0 min CN=74 Runoff=0.08 cfs 245 cf
Subcatchment P-1e: Fllow to Swale	Runoff Area=1,184 sf 0.00% Impervious Runoff Depth=2.57" Tc=6.0 min CN=75 Runoff=0.08 cfs 254 cf
Subcatchment P-1f: Fllow to Swale	Runoff Area=1,071 sf 0.00% Impervious Runoff Depth=2.48" Tc=6.0 min CN=74 Runoff=0.07 cfs 222 cf
Subcatchment P-2: Flow onsite Southeast	Runoff Area=6,213 sf 93.87% Impervious Runoff Depth=4.80" Tc=6.0 min CN=97 Runoff=0.70 cfs 2,483 cf
Subcatchment P-3: Flow off-site West	Runoff Area=982 sf 0.00% Impervious Runoff Depth=2.93" Tc=6.0 min CN=79 Runoff=0.08 cfs 240 cf
Reach DP-1: Basin	Inflow=4.29 cfs 33,560 cf Outflow=4.29 cfs 33,560 cf
Reach DP-2: Onsite southeast	Inflow=0.70 cfs 2,483 cf Outflow=0.70 cfs 2,483 cf

Proposed-6 Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC	Type III 24-hr 10-Year Rainfall=5.15" Printed 2/2/2023 Page 6
Reach DP-3: Off-site West	Inflow=0.08 cfs 240 cf Outflow=0.08 cfs 240 cf
Pond S-1: Rip Rap Swale	Peak Elev=334.05' Storage=215 cf Inflow=0.08 cfs 254 cf Outflow=0.00 cfs 39 cf
Pond S-2: Rip Rap Swale	Peak Elev=332.50' Storage=198 cf Inflow=0.08 cfs 284 cf Outflow=0.00 cfs 86 cf
Pond S-3: Rip Rap Swale	Peak Elev=331.00' Storage=195 cf Inflow=0.08 cfs 340 cf Outflow=0.01 cfs 145 cf
Pond S-4: Rip Rap Swale	Peak Elev=329.25' Storage=148 cf Inflow=0.07 cfs 367 cf Outflow=0.01 cfs 219 cf
Pond SSD-1: Subsurface Detention Basin-1	Peak Elev=324.20' Storage=11,384 cf Inflow=8.07 cfs 29,243 cf Outflow=3.18 cfs 27,728 cf

Total Runoff Area = 105,367 sf Runoff Volume = 38,553 cf Average Runoff Depth = 4.39" 24.00% Pervious = 25,289 sf 76.00% Impervious = 80,078 sf

Area (sf) Noods, Good, HSG C 13.863 7 13.87 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 18.17 7 19.0 0.090 0.11 Woods, Light underthrush ne 0.400 10.0 461 10.0 461 10.0 461 10.0 461 10.0 461 10.0 10.022. 10.0 10.022. 10.0 10.022. 10.0 10.022. 10.0 10.022. 10.0 10.022. 10.0 10.022. 10.0 10.022. <			R-20 met	-		Ime= 4,219 cf, Depth= 2.75" ted-CN, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs
4.454 70 Woods, Good, HSG C 13.863 77 Weighted Average 18.417 70 Weighted Average 18.417 71 Weighted Average 18.417 100.0040 2.49 Shallow Concentrated Flow, Unpaved Kv=16.1 tps 10.0 461 Total	4					
18.417 7 Weighted Average 100.00% Periods Xea To Leight Slope Velocity Capacity Description 7.3 500.0000 0.11 Sheet Flow, Sheet Flow, Woods: Light underbrush n= 0.400 P2=3.05" 2.7 411 0.0240 2.49 Shallow Concentrated Flow, Unpaved Kv=16.1 (ps 10.0 461 Total	,	4,454	70 V	Voods, Go	od, HSG C	
To Length Slope Velocity Capacity Description 7.3 50 0.0000 0.11 Sheet Flow, Sheet Flow, Woods: Upth underbrush n= 0.400 P2=3.05" 2.7 411 0.0240 2.49 Shallow Concentrated Flow, Impaved Kv=16.1 fps 10.0 461 Total Total Total Type III 24-hr 10-Year Rainfall=5.15" Proposed-6 Type III 24-hr 10-Year Rainfall=5.15" Printed 2/2/2023 Printed 2/2/2023 10.0 461 Total Type III 24-hr 10-Year Rainfall=5.15" Proposed-6 Subcatchment P-1: Flow to Existing Basin Printed 2/2/2023 10.0 1.1 Existing Capacity Type III 24-hr 10.1 1.1 Existing Basin Type III 24-hr		18,417	77 V	Veighted A	verage	
Child (refer) (fth) (ft) (ft) Sheet Flow, Sheet Flow Woods: Upth underbrush n= 0.400 P2= 3.05" 2.7 411 0.240 2.49 Shallow Concentrated Flow, Unpaved Kv=16.11ps 10.0 461 Total Total						
2.7 411 0.0240 2.49 Woods: Light underbrush n=0.400 P2=3.05" 10.0 461 Total Total Total Total Proposed-6 Proposed-6 Type III 24-hr 10-Year Rainfall=5.15" Proposed-6 Type III 24-hr 10-Year Rainfall=5.15" Proposed-6 Type III 24-hr 10-Year Rainfall=5.15" Proposed-6 Subcatchment P-1: Flow to Existing Basin Hydrograph Hydrograph 1 1.18 cfs 1 1 1						Description
Type III 24-hr 10-Year Rainfall=5.15" Proposed-6 Prepared by (enter your company name here) Prepared by (enter your company name here) Prepared by (enter your company name here) Prepared by (enter your company name here) Printed 21/2023 Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph 10-Year Rainfall=5.15" Runoff Area=18,417 sf Runoff Volume=4,219 cf	7.3	50	0.0800	0.11		
200 461 Total Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-6 Proposed-7 Pro	2.7	411	0.0240	2.49		
Pripared by {enter your company name here} hydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph 1.18 cfs 10-Year Rainfall=5.15" Runoff Area=18,417 sf Runoff Volume=4,219 cf	10.0	461	Total			
Pripared by {enter your company name here} hydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph 1.18 cfs 10-Year Rainfall=5.15" Runoff Area=18,417 sf Runoff Volume=4,219 cf						
Pripared by {enter your company name here} hydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph 1.18 cfs 10-Year Rainfall=5.15" Runoff Area=18,417 sf Runoff Volume=4,219 cf						
Pripared by {enter your company name here} hydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph 1.18 cfs 10-Year Rainfall=5.15" Runoff Area=18,417 sf Runoff Volume=4,219 cf						
Pripared by {enter your company name here} hydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph 1.18 cfs 10-Year Rainfall=5.15" Runoff Area=18,417 sf Runoff Volume=4,219 cf						
Pripared by {enter your company name here} hydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph 1.18 cfs 10-Year Rainfall=5.15" Runoff Area=18,417 sf Runoff Volume=4,219 cf						
Pripared by {enter your company name here} hydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Page 8 Subcatchment P-1: Flow to Existing Basin Hydrograph 1.18 cfs 10-Year Rainfall=5.15" Runoff Area=18,417 sf Runoff Volume=4,219 cf						
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	Flow (cfs)					Flow Length=461' Tc=10.0 min

	nmary for Subcatchment P-1a: Flow to SSI-1
noff = 8.07 cfs @ 12.09 hrs, Volur	
noff by SCS TR-20 method, UH=SCS, Weight pe III 24-hr 10-Year Rainfall=5.15"	tted-CN, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs
Area (sf) CN Description	
71,133 98 Paved parking 295 79 50-75% Grass cover, Fa	Fair, HSG C
71,428 98 Weighted Average 295 0.41% Pervious Area	
71,133 99.59% Impervious Are	
Tc Length Slope Velocity Capacity (min) (feet) (ft/ft) (ft/sec) (cfs)	•
6.0	Direct Entry, Min.
	e} Type III 24-hr 10-Year Rainfall=5.1 Printed 2/2/20
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unoff	=	0.40 cf	s@ 12	2.09 hrs,	Volume=		1,393 cf,	Depth= 4.68"	
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	ea (sf) 3,113		escription						
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					-	Subcatchment P-1e: Fllow to Swale	
Inoff	=		-	09 hrs, Vol		254 cf, Depth= 2.57"	
inoff by	y SCS TF 24-hr 10-	R-20 me Year Ra	thod, UH= ainfall=5.15	SCS, Weigl 5"	hted-CN, Tim	ne Span= 0.00-80.00 hrs, dt= 0.05 hrs	
A	rea (sf)		Descriptior				
	350 58	70		ood, HSG C			
	<u>776</u> 1,184	75	Weighted /	Average	Fair, HSG C		—
	1,184			Pervious Are			
(min)	Length (feet)	Slope (ft/ft		Capacity (cfs)			
6.0					Direct Ent	try, Min.	
	sed-6					Type III 24-hr 10-Year Rainfall=5.	5"
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	by SCS TF 24-hr 10-				CS, W	eighte	ed-CN, ⁻	Time Sp	an= 0.00-80.0	00 hrs, dt= 0	0.05 hrs			
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	1,071	/ 4	100.00	% Pe	rvious	Area								
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Runoff	=	0.70	cfs @	12.09	9 hrs,	Volun	ne=		2,483	cf, De	pth= 4	.80"										
Runoff	by SCS TH	R-20 m	ethod, l	JH=S	CS, W	eighte	ed-CN	, Time	Span=	0.00-8	0.00 h	rs, dt= 0	.05 h	nrs								
	24-hr 10-																					
A	Area (sf) 5,832	<u>CN</u> 98	Descri Paved		na																	-
	381	79	50-75	% Gra	ISS CO		air, HS	SG C														-
	6,213 381	97	Weigh 6.13%	Perv	ious A	rea																
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Prepare lydroCA 0	.75 .75 .75 .65 .55 .55 .55	26 s/n	04015	© 2020	name) Hydro	CAD	Softwa	chme	nt P-2	: Flov		-Ye Runo	ar off Vo	T Ra Ar	/pe inf ea:	> a =6	24 =5 21 48	4-h 15 3 s)F)f 	Printed	2/2/2023 Page 22	3
Prepare lydroCA 0	75 75 0.7 65 0.6 55 0.5 45	26 s/n	04015	© 2020	name) Hydro	CAD	Softwa	chme	nt P-2	: Flov		-Ye Rung	ar off Vo	T Ra Ar	/pe inf ea: ne:	> a =6 =2	24 =5 21 48 =4	4-h 15 3 s 3 c	۲	Printed	2/2/2023 Page 22	3
Prepare HydroCA 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	.75 .75 .75 .65 .55 .55 .45 .45 .45	26 s/n	04015	© 2020	name D Hydro	CAD	Softwa	chme	nt P-2	: Flov		-Ye Rung	ar off Vo	T Ra Ar	/pe inf ea: ne:	= a =6 =2 oth =6	24 =5 21 48 =4	4-h 15 3 s 3 c 80 mi)r >f n	Printed	2/2/2023 Page 22	3
Prepare lydroCA 0 0 0 0 0 0 0 0 0 0 0 0 0	ed by (en D® 10.00 .75 0.7 .65 0.6 .55 0.5 .45 0.4 .35 0.4 .35 0.3 .35	26 s/n	04015	© 2020	name	CAD	Softwa	chme	nt P-2	: Flov		-Ye Rung	ar off Vo	T Ra Ar	/pe inf ea: ne:	= a =6 =2 oth =6	24 =5 21 48 =4	4-h 15 3 s 3 c 80 mi)r >f n	Printed	2/2/2023 Page 22	3
Prepare <u>lydroCA</u> 0 0 0 L 0 0 0 0 0 0 0 0 0 0 0 0 0	ad by (en <u>D® 10.00</u> .75 0.7 .65 0.6 .55 0.6 .55 0.5 0.5 0.5 0.4 .55 0.4 .45 0.4 .25 .25 .25 .25 .25 .25 .25 .25	26 s/n	04015	© 2020	name) Hydro	CAD	Softwa	chme	nt P-2	: Flov		-Ye Rung	ar off Vo	T Ra Ar	/pe inf ea: ne:	= a =6 =2 oth =6	24 =5 21 48 =4	4-h 15 3 s 3 c 80 mi)r >f n	Printed	2/2/2023 Page 22	3
Prepare <u>lydroC</u> A 0 0 0 0 0 0 0 0 0 0 0 0 0	A by (en D® 10.00 .75 .75 .65 .65 .65 .65 .65 .65 .65 .6	26 s/n	04015	© 2020	name) Hydro	CAD	Softwa	chme	nt P-2	: Flov		-Ye Rung	ar off Vo	T Ra Ar	/pe inf ea: ne:	= a =6 =2 oth =6	24 =5 21 48 =4	4-h 15 3 s 3 c 80 mi)r >f n	Printed	2/2/2023 Page 22	3
Prepare <u>lydroCA</u> 0 0 0 0 0 0 0 0 0 0 0 0 0	A by {en D 0 10.00 .75 .75 .75 .75 .75 .75 .75 .75	26 s/n	04015	© 2020	name) Hydro	CAD	Softwa	chme	nt P-2	: Flov		-Ye Rung	ar off Vo	T Ra Ar	/pe inf ea: ne:	= a =6 =2 oth =6	24 =5 21 48 =4	4-h 15 3 s 3 c 80 mi)r >f n	Printed	2/2/2023 Page 22	3
Prepare <u>lydroCA</u> 0 0 0 0 0 0 0 0 0 0 0 0 0	A by (en D® 10.00 .75 .75 .65 .65 .65 .65 .65 .65 .65 .6	26 s/n	04015	© 2020	name) Hydro	CAD	Softwa	chme	nt P-2	: Flov		-Ye Rung	ar off Vo	T Ra Ar	/pe inf ea: ne:	= a =6 =2 oth =6	24 =5 21 48 =4	4-h 15 3 s 3 c 80 mi)r >f n	Printed	2/2/2023 Page 22	3

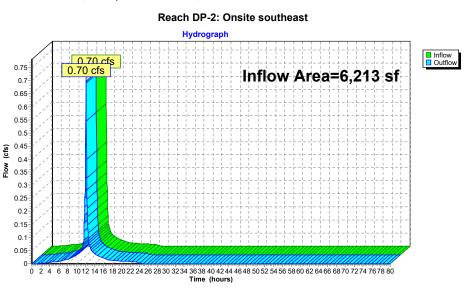
Runoff =	0.08	cfs @ 12.0	9 hrs, Vol	ume=		240 cf,	Depth	= 2.93"	,						
Runoff by SCS ⁻ Type III 24-hr 1				nted-CN,	Time Sp	pan= 0.0	0.08-00	0 hrs, di	t= 0.05	hrs					
Area (sf)		Descriptior													
10 972	70 79	Woods, Go 50-75% Gr			ЭC										
982 982		Weighted A 100.00% P		ea											
Tc Lengtl		e Velocity		Descrip	otion										
<u>(min)</u> (feet 6.0) (ft/f	t) (ft/sec)	(cfs)	Direct	Entry, I	Min.									
				Subca	tchme	ent P-3	: Flow	off-si	te Wes	st					
		0.085 0.085 0.075 0.075 0.065 0.055 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.045 0.025 0.025 0.025 0.025 0.025 0.025 0.025 0.025 0.0450					Runc Runc Ru	ear Ra unoff off Vo unoff I		=5.1(=982 =240 =2.93 5.0 mi CN=7	5"	Runoff			
Proposed-6											Тур	pe III 24	1-hr		Rainfall=5.15"
Prepared by {e				D Softwar							Тур	be III 24	1-hr		Rainfall=5.15" nted 2/2/2023 Page 24
Prepared by {e HydroCAD® 10.0	0-26 s/n	04015 © 202	20 HydroCA	D Softwar	e Solutio nmary		each D)P-1: B	Basin		Туן	oe III 24	1-hr		nted 2/2/2023
Proposed-6 Prepared by {e HydroCAD® 10.0 [40] Hint: Not De Inflow Area = Inflow = Outflow =	<u>0-26 s/n</u> escribed 98 4.29	04015 © 202	2 <u>0 HydroCA</u> flow) 63% Imper 20 hrs, Vol	<u>D Softwar</u> Sur vious, In ume=	nmary flow De 33	pth = _4	l.10" f	for 10-Y			Ty	oe III 24	t-hr		nted 2/2/2023
Prepared by {e <u>HydroCAD® 10.0</u> [40] Hint: Not De Inflow Area = Inflow =	0-26 s/n escribed 98 4.29 4.29	04015 © 202 (Outflow=In ,172 sf, 75. cfs @ 12.2 cfs @ 12.2	20 HydroCA flow) 63% Imper 20 hrs, Vol 20 hrs, Vol	D Softwar Sur vious, In ume= ume=	nmary flow De 33 33	pth = _4 ,560 cf ,560 cf,	I.10" f Atten=	for 10-Y	Year eve		Тур	be III 24	1-hr		nted 2/2/2023
Prepared by {e <u>HydroCAD® 10.0</u> [40] Hint: Not Do Inflow Area = Inflow = Outflow =	0-26 s/n escribed 98 4.29 4.29	04015 © 202 (Outflow=In ,172 sf, 75. cfs @ 12.2 cfs @ 12.2	20 HydroCA flow) 63% Imper 20 hrs, Vol 20 hrs, Vol	D Softwar Sur vious, In ume= ume=	nmary flow De 33 33 .00 hrs, Re a	t for Re pth = _2 ,560 cf, ,560 cf, dt= 0.0 ach DI	l.10" f Atten= 5 hrs	for 10-Υ :0%, La	Year eve		Тур	oe III 24	1-hr		nted 2/2/2023
Prepared by {e <u>HydroCAD® 10.0</u> [40] Hint: Not Do Inflow Area = Inflow = Outflow =	0-26 s/n escribed 98 4.29 4.29	04015 © 202 (Outflow=In ,172 sf, 75. cfs @ 12.2 cfs @ 12.2	20 HydroCA flow) 63% Imper 20 hrs, Vol 20 hrs, Vol	D Softwar Sur vious, In ume= ume=	nmary flow De 33 33 .00 hrs, Re a	pth = 2 ,560 cf ,560 cf, dt= 0.0	l.10" f Atten= 5 hrs	for 10-Υ :0%, La	Year eve		<i>Ty</i>	be III 24	1-hr	Pri	nted 2/2/2023
Prepared by {e <u>HydroCAD® 10.0</u> [40] Hint: Not Do Inflow Area = Inflow = Outflow =	0-26 s/n escribed 98 4.29 4.29	(Outflow=In ,172 sf, 75. cfs @ 12.2 cfs @ 12.2 ns method,	flow) flow) 63% Imper 20 hrs, Vol 20 hrs, Vol Time Span	D Softwar Sur vious, In ume= ume=	nmary flow De 33 33 .00 hrs, Re a	t for Re pth = _2 ,560 cf, ,560 cf, dt= 0.0 ach DI	I.10" f Atten= 5 hrs P-1: Ba	for 10-1	Year eve ag= 0.0	min					nted 2/2/2023
Prepared by {e <u>HydroCAD® 10.0</u> [40] Hint: Not Do Inflow Area = Inflow = Outflow =	0-26 s/n escribed 98 4.29 4.29	(Outflow=In ,172 sf, 75. cfs @ 12.2 cfs @ 12.2 ns method,	flow) flow) 63% Imper 20 hrs, Vol 20 hrs, Vol Time Span	D Softwar Sur vious, In ume= ume=	nmary flow De 33 33 .00 hrs, Re a	t for Re pth = _2 ,560 cf, ,560 cf, dt= 0.0 ach DI	I.10" f Atten= 5 hrs P-1: Ba	for 10-1	Year eve ag= 0.0	min		oe III 24		Pri	nted 2/2/2023
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Prepared by {e HydroCAD® 10.0 [40] Hint: Not Do Inflow Area = Inflow = Outflow =	0-26 s/n escribed 98 4.29 4.29 4.29 -Ind+Tra	(Outflow=In ,172 sf, 75. cfs @ 12.2 cfs @ 12.2 ns method,	flow) flow) 63% Imper 20 hrs, Vol 20 hrs, Vol Time Span	D Softwar Sur vious, In ume= ume=	nmary flow De 33 33 .00 hrs, Re a	t for Re pth = _2 ,560 cf, ,560 cf, dt= 0.0 ach DI	I.10" f Atten= 5 hrs P-1: Ba	for 10-1	Year eve ag= 0.0	min				Pri	nted 2/2/2023
Prepared by {e <u>HydroCAD® 10.0</u> [40] Hint: Not De Inflow Area = Inflow = Outflow = Routing by Stor-	0-26 s/n escribed 98 4.29 4.29 4.29 -Ind+Tra	(Outflow=In ,172 sf, 75. cfs @ 12.2 cfs @ 12.2 ns method,	flow) flow) 63% Imper 20 hrs, Vol 20 hrs, Vol Time Span	D Softwar Sur vious, In ume= ume=	nmary flow De 33 33 .00 hrs, Re a	t for Re pth = _2 ,560 cf, ,560 cf, dt= 0.0 ach DI	I.10" f Atten= 5 hrs P-1: Ba	for 10-1	Year eve ag= 0.0	min				Pri	nted 2/2/2023
Prepared by {e HydroCAD® 10.0 [40] Hint: Not Do Inflow Area = Inflow = Outflow =	0-26 s/n escribed 98 4.29 4.29 4.29 -Ind+Tra	(Outflow=In ,172 sf, 75. cfs @ 12.2 cfs @ 12.2 ns method,	flow) flow) 63% Imper 20 hrs, Vol 20 hrs, Vol Time Span	D Softwar Sur vious, In ume= ume=	nmary flow De 33 33 .00 hrs, Re a	t for Re pth = _2 ,560 cf, ,560 cf, dt= 0.0 ach DI	I.10" f Atten= 5 hrs P-1: Ba	for 10-1	Year eve ag= 0.0	min				Pri	nted 2/2/2023
Prepared by {e <u>HydroCAD® 10.0</u> [40] Hint: Not De Inflow Area = Inflow = Outflow = Routing by Stor-	0-26 s/n escribed 98 4.29 4.29 4.29 -Ind+Tra	(Outflow=In ,172 sf, 75. cfs @ 12.2 cfs @ 12.2 ns method,	flow) flow) 63% Imper 20 hrs, Vol 20 hrs, Vol Time Span	D Softwar Sur vious, In ume= ume=	nmary flow De 33 33 .00 hrs, Re a	t for Re pth = 2 ,560 cf, ,560 cf, dt= 0.0 ach DI	I.10" f Atten= 5 hrs P-1: Ba	for 10-1	Year eve ag= 0.0	min				Pri	nted 2/2/2023
Prepared by {e <u>HydroCAD® 10.0</u> [40] Hint: Not De Inflow Area = Inflow = Outflow = Routing by Stor-	0-26 s/n escribed 98 4.29 4.29 4.29 -Ind+Tra	(Outflow=In ,172 sf, 75. cfs @ 12.2 cfs @ 12.2 ns method,	flow) flow) 63% Imper 20 hrs, Vol 20 hrs, Vol Time Span	D Softwar Sur vious, In ume= ume=	nmary flow De 33 33 .00 hrs, Re a	t for Re pth = 2 ,560 cf, ,560 cf, dt= 0.0 ach DI	I.10" f Atten= 5 hrs P-1: Ba	for 10-1	Year eve ag= 0.0	min				Pri	nted 2/2/2023
Prepared by {e <u>HydroCAD® 10.0</u> [40] Hint: Not De Inflow Area = Inflow = Outflow = Routing by Stor-	0-26 s/n escribed 98 4.29 4.29 4.29 -Ind+Tra	(Outflow=In ,172 sf, 75. cfs @ 12.2 cfs @ 12.2 ns method,	flow) flow) 63% Imper 20 hrs, Vol 20 hrs, Vol Time Span	D Softwar Sur vious, In ume= ume=	nmary flow De 33 33 .00 hrs, Re a	t for Re pth = 2 ,560 cf, ,560 cf, dt= 0.0 ach DI	I.10" f Atten= 5 hrs P-1: Ba	for 10-1	Year eve ag= 0.0	min				Pri	nted 2/2/2023

Summary for Reach DP-2: Onsite southeast

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	ea =	6,213 sf, 93.87% Impervious, Inflow Dept	h= 4.80"	for 10-Year event
Inflow	=	0.70 cfs @ 12.09 hrs, Volume= 2,4	83 cf	
Outflow	=	0.70 cfs @ 12.09 hrs, Volume= 2,4	83 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs



Proposed-6

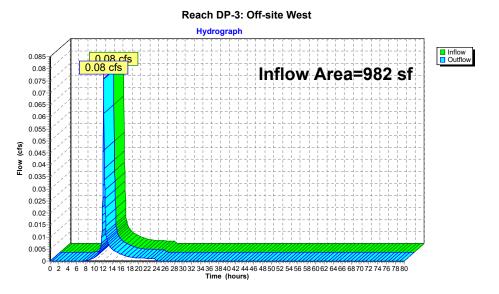
Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC Type III 24-hr 10-Year Rainfall=5.15" Printed 2/2/2023 Page 26

Summary for Reach DP-3: Off-site West

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	982 sf,	0.00% Impervious,	Inflow Depth = 2.93"	for 10-Year event
Inflow	=	0.08 cfs @ 1	12.09 hrs, Volume=	240 cf	
Outflow	=	0.08 cfs @ 1	12.09 hrs, Volume=	240 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs



Summary for Pond S-1: Rip Rap Swale

Inflow Area =	1,317 sf, 0.00% Impervious,	Inflow Depth = 2.31" for 10-Year event
Inflow =	0.08 cfs @ 12.10 hrs, Volume=	254 cf
Outflow =	0.00 cfs @ 17.20 hrs, Volume=	39 cf, Atten= 97%, Lag= 306.3 min
Primary =	0.00 cfs @ 17.20 hrs, Volume=	39 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 334.05' @ 17.20 hrs Surf.Area= 200 sf Storage= 215 cf

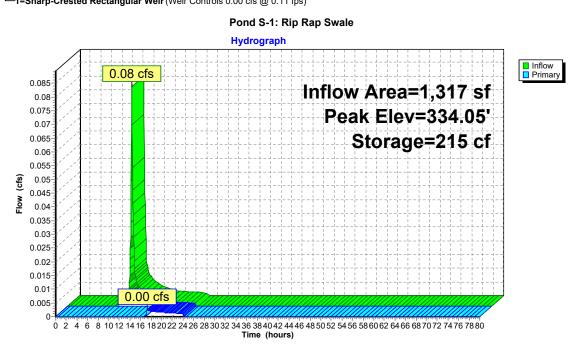
Plug-Flow detention time= 517.4 min calculated for 39 cf (15% of inflow) Center-of-Mass det. time= 369.3 min (1,210.3 - 841.0)

#1	331.15'	120 cf	Custom Stage Date 414 cf Overall - 114			side	
#2	332.65'	114 cf	Custom Stage Dat				
		234 cf	Total Available Stor				
Elevation (feet)	Surf.Area (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
331.15		0.0	0	0	0		
331.50	21	27.2	2	2	59		
332.50	171	99.3	84	86	788		
332.65	200) 104.4	28	114	872		
334.15	200) 104.4	300	414	1,028		
Elevation	Surf.Area	a Perim.	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft) (feet)	(cubic-feet)	(cubic-feet)	(sq-ft)		
332.65	C	0.0	0	0	0		
333.00	21	27.2	2	2	59		
334.00	171	I 99.3	84	86	788		
334.15	200) 104.4	28	114	872		

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Type III 24-hr 10-Year Rainfall=5.15" Printed 2/2/2023 Page 28

Primary OutFlow Max=0.00 cfs @ 17.20 hrs HW=334.05' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.00 cfs @ 0.11 fps)



Summary for Pond S-2: Rip Rap Swale

Inflow Area =	2,501 sf, 0.00% Impervious,	Inflow Depth = 1.36" for 10-Year event
Inflow =	0.08 cfs @ 12.09 hrs, Volume=	284 cf
Outflow =	0.00 cfs @ 17.23 hrs, Volume=	86 cf, Atten= 94%, Lag= 308.2 min
Primary =	0.00 cfs @ 17.23 hrs, Volume=	86 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 332.50' @ 17.23 hrs Surf.Area= 200 sf Storage= 198 cf

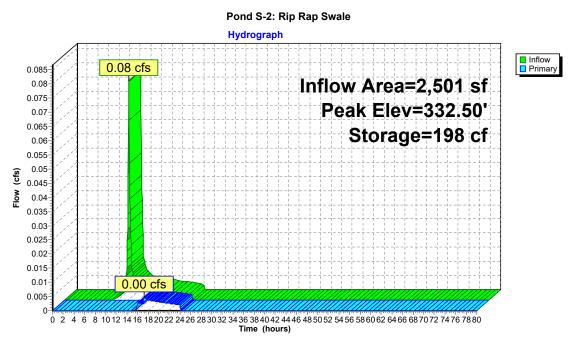
Plug-Flow detention time= 475.9 min calculated for 86 cf (30% of inflow) Center-of-Mass det. time= 296.3 min (1,183.8 - 887.5)

#1	329.65'	120 cf	Custom Stage Da	ta (Irregular)Listed	below (Recalc)	
			407 cf Overall - 10			
#2	331.15'	107 cf	Custom Stage Da	ta (Irregular)Listed	below (Recalc)	Inside #1
		227 cf	Total Available Sto	rage		
Elevation	Surf.Ar	ea Perim.	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq		(cubic-feet)	(cubic-feet)	(sq-ft)	
329.65		0 0.0	0	0	0	
330.50		74 61.1	21	21	298	
331.15		00 104.5	86	107	873	
332.65	2	00 104.5	300	407	1,029	
Elevation	Surf.Ar	ea Perim.	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq·	-ft) (feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
331.15		0 0.0	0	0	0	
332.00		74 61.1	21	21	298	
332.65	2	00 104.5	86	107	873	
Device Ro	outing	Invert Out	et Devices			
#1 Pr	imary	332.50' 7.0'	long Sharp-Crested	d Rectangular Wei	r 2 End Contract	tion(s)
	,			J		()

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Type III 24-hr 10-Year Rainfall=5.15" Printed 2/2/2023 Page 30

Primary OutFlow Max=0.00 cfs @ 17.23 hrs HW=332.50' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.00 cfs @ 0.11 fps)



Summary for Pond S-3: Rip Rap Swale

Inflow Area =	3,685 sf, 0.00% Impervious,	Inflow Depth = 1.11" for 10-Year event
Inflow =	0.08 cfs @ 12.09 hrs, Volume=	340 cf
Outflow =	0.01 cfs @ 17.26 hrs, Volume=	145 cf, Atten= 91%, Lag= 309.9 min
Primary =	0.01 cfs @ 17.26 hrs, Volume=	145 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 331.00' @ 17.26 hrs Surf.Area= 200 sf Storage= 195 cf

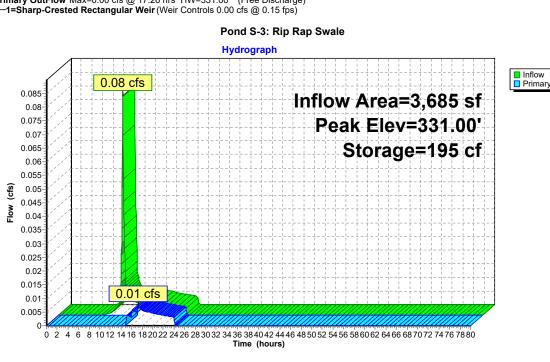
Plug-Flow detention time= 438.1 min calculated for 145 cf (43% of inflow) Center-of-Mass det. time= 235.3 min (1,157.5 - 922.2)

Volume #1 #2	Invert 329.70' 328.20'	Avail.Storage 112 c 120 c	Custom Stage Da	ta (Irregular)Listed		Inside #2
			412 cf Overall - 112		00 cf x 40.0% Vo	oids
		232 c	Total Available Sto	rage		
Elevation	Surf.A	rea Perim		Cum.Store	Wet.Area	
(feet)	(so	q-ft) (feet) (cubic-feet)	(cubic-feet)	(sq-ft)	
329.70		0 0.	0 0	0	0	
330.00		16 23.) 2	2	42	
331.00		158 95.	1 75	76	722	
331.20	2	200 104.	5 36	112	873	
Elevation	Surf.A	rea Perim	. Inc.Store	Cum.Store	Wet.Area	
(feet)	(so	q-ft) (feet) (cubic-feet)	(cubic-feet)	(sq-ft)	
328.20		0 0.	0 0	0	0	
328.50		16 23.	2	2	42	
329.50		158 95.	1 75	76	722	
329.70		200 104.	5 36	112	873	
331.20	1	200 104.	5 300	412	1,030	
	outing	Invert Or	tlet Devices			
Device Ro #1 Pr	outing	Invert Ou	' long Sharp-Crested			

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Type III 24-hr 10-Year Rainfall=5.15" Printed 2/2/2023 Page 32

Primary OutFlow Max=0.00 cfs @ 17.26 hrs HW=331.00' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.00 cfs @ 0.15 fps)



Summary for Pond S-4: Rip Rap Swale

Inflow Area =	4,756 sf, 0.00% Impervious,	Inflow Depth = 0.93" for 10-Year event
Inflow =	0.07 cfs @ 12.09 hrs, Volume=	367 cf
Outflow =	0.01 cfs @ 17.27 hrs, Volume=	219 cf, Atten= 87%, Lag= 310.3 min
Primary =	0.01 cfs @ 17.27 hrs, Volume=	219 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 329.25' @ 17.27 hrs Surf.Area= 188 sf Storage= 148 cf

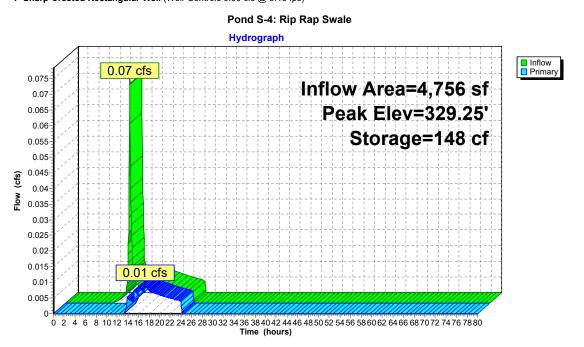
Plug-Flow detention time= 320.0 min calculated for 219 cf (60% of inflow) Center-of-Mass det. time= 155.6 min (1,118.7 - 963.1)

#1 #2	328.20' 326.70'	108 cf 113 cf	Custom Stage Dat Custom Stage Dat 390 cf Overall - 108	ta (Irregular)Listed	below (Recalc)	Inside #2 bids
		221 cf	Total Available Sto	rage		
Elevation (feet)	Surf.Area (sq-ft)		Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft <u>)</u>	
328.20	0		0	0	0	
329.00	71		19	19	259	
329.25	112		23	42	466	
329.70	188	3 102.0	67	108	832	
Elevation	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)) (feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
326.70	0	0.0	0	0	0	
327.50	71	56.9	19	19	259	
327.75	112	2 76.4	23	42	466	
328.20	188	3 102.0	67	108	832	
329.70	188	3 102.0	282	390	985	

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Type III 24-hr 10-Year Rainfall=5.15" Printed 2/2/2023 Page 34

Primary OutFlow Max=0.00 cfs @ 17.27 hrs HW=329.25' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.00 cfs @ 0.15 fps)



Summary for Pond SSD-1: Subsurface Detention Basin-1

Inflow Area =	71,428 sf, 99.59% Impervious,	Inflow Depth = 4.91" for 10-Year event
Inflow =	8.07 cfs @ 12.09 hrs, Volume=	29,243 cf
Outflow =	3.18 cfs @ 12.31 hrs, Volume=	27,728 cf, Atten= 61%, Lag= 13.5 min
Primary =	3.18 cfs @ 12.31 hrs, Volume=	27,728 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 9 Peak Elev= 324.20' @ 12.31 hrs Surf.Area= 7,434 sf Storage= 11,384 cf

Plug-Flow detention time= 174.8 min calculated for 27,728 cf (95% of inflow) Center-of-Mass det. time= 144.7 min (892.2 - 747.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	322.00'	6,586 cf	63.25'W x 117.54'L x 3.50'H Field A
			26,020 cf Overall - 9,556 cf Embedded = 16,464 cf x 40.0% Voids
#2A	322.50'	9,556 cf	ADS_StormTech SC-740 +Cap x 208 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			208 Chambers in 13 Rows
		16,141 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	322.50'	15.0" Round Culvert L= 21.3' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 322.50' / 322.00' S= 0.0235 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf
#2	Device 1	325.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Device 1	324.25'	6.0" Vert. Orifice/Grate C= 0.600
#4	Device 1	323.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
#5	Device 1	322.50'	6.0" Vert. Orifice/Grate C= 0.600

Proposed-6

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Type III 24-hr 10-Year Rainfall=5.15" Printed 2/2/2023 Page 36

Primary OutFlow Max=3.17 cfs @ 12.31 hrs HW=324.20' (Free Discharge) 1=Culvert (Passes 3.17 cfs of 6.13 cfs potential flow) 2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs) 3=Orifice/Grate (Controls 0.00 cfs) 4=Orifice/Grate (Orifice Controls 2.03 cfs @ 2.91 fps) 5=Orifice/Grate (Orifice Controls 1.14 cfs @ 5.80 fps)

Pond SSD-1: Subsurface Detention Basin-1 - Chamber Wizard Field A

Chamber Model = ADS_StormTech SC-740 +Cap (ADS StormTech® SC-740 with cap length) Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

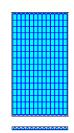
16 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 115.54' Row Length +12.0" End Stone x 2 = 117.54' Base Length 13 Rows x 51.0" Wide + 6.0" Spacing x 12 + 12.0" Side Stone x 2 = 63.25' Base Width 6.0" Base + 30.0" Chamber Height + 6.0" Cover = 3.50' Field Height

208 Chambers x 45.9 cf = 9,555.5 cf Chamber Storage

26,019.7 cf Field - 9,555.5 cf Chambers = 16,464.2 cf Stone x 40.0% Voids = 6,585.7 cf Stone Storage

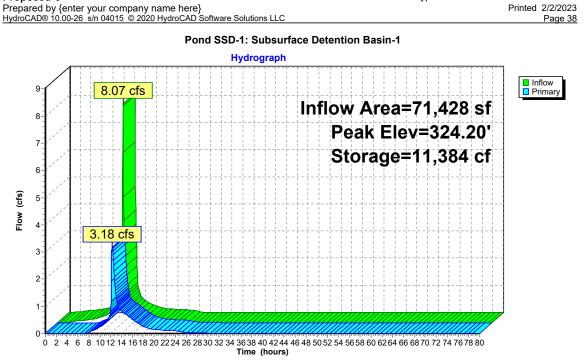
Chamber Storage + Stone Storage = 16,141.2 cf = 0.371 af Overall Storage Efficiency = 62.0%Overall System Size = 117.54' x 63.25' x 3.50'

208 Chambers 963.7 cy Field 609.8 cy Stone



Proposed-6

Type III 24-hr 10-Year Rainfall=5.15" Printed 2/2/2023





HydroCAD Analysis

Proposed Conditions - 25 Year Storm

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	Page 1

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
18,555	79	50-75% Grass cover, Fair, HSG C (P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3)
80,078	98	Paved parking (P-1a, P-1b, P-2)
1,396	66	Rip Rap Swale (P-1c, P-1d, P-1e, P-1f)
5,338	70	Woods, Good, HSG C (P-1, P-1c, P-1d, P-1e, P-1f, P-3)
105,367	93	TOTAL AREA

Area Listing (all nodes)

Soil Listing (all nodes)

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Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
0	HSG A	
0	HSG B	
23,893	HSG C	P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3
0	HSG D	
81,474	Other	P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2
105,367		TOTAL AREA

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Ground Covers (all nodes)							
 HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Subcatchment Numbers
 0	0	18,555	0	0	18,555	50-75% Grass cover, Fair	P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3
0	0	0	0	80,078	80,078	Paved parking	P-1a, P-1b, P-2
0	0	0	0	1,396	1,396	Rip Rap Swale	P-1c, P-1d, P-1e, P-1f
0	0	5,338	0	0	5,338	Woods, Good	P-1, P-1c, P-1d, P-1e, P-1f, P-3
0	0	23,893	0	81,474	105,367	TOTAL AREA	

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				Pipe Listing (all nodes)						
Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)	
1	SSD-1	322.50	322.00	21.3	0.0235	0.013	15.0	0.0	0.0	

Time span=0.00-80.00 hrs, dt=0.05 hrs, 1601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentP-1: Flow to Existing Basin	Runoff Area=18,417 sf 0.00% Impervious Runoff Depth=3.79" Flow Length=461' Tc=10.0 min CN=77 Runoff=1.62 cfs 5,811 cf
SubcatchmentP-1a: Flow to SSI-1	Runoff Area=71,428 sf 99.59% Impervious Runoff Depth=6.11" Tc=6.0 min CN=98 Runoff=9.97 cfs 36,378 cf
SubcatchmentP-1b: Flow to Treatment Unit	Runoff Area=3,571 sf 87.17% Impervious Runoff Depth=5.88" Tc=6.0 min CN=96 Runoff=0.49 cfs 1,749 cf
SubcatchmentP-1c: Filow to Swale	Runoff Area=1,317 sf 0.00% Impervious Runoff Depth=3.28" Tc=6.0 min CN=72 Runoff=0.11 cfs 360 cf
SubcatchmentP-1d: Filow to Swale	Runoff Area=1,184 sf 0.00% Impervious Runoff Depth=3.48" Tc=6.0 min CN=74 Runoff=0.11 cfs 343 cf
SubcatchmentP-1e: Fllow to Swale	Runoff Area=1,184 sf 0.00% Impervious Runoff Depth=3.58" Tc=6.0 min CN=75 Runoff=0.11 cfs 353 cf
SubcatchmentP-1f: Fllow to Swale	Runoff Area=1,071 sf 0.00% Impervious Runoff Depth=3.48" Tc=6.0 min CN=74 Runoff=0.10 cfs 311 cf
SubcatchmentP-2: Flow onsite Southeast	Runoff Area=6,213 sf 93.87% Impervious Runoff Depth=5.99" Tc=6.0 min CN=97 Runoff=0.86 cfs 3,103 cf
SubcatchmentP-3: Flow off-site West	Runoff Area=982 sf 0.00% Impervious Runoff Depth=3.99" Tc=6.0 min CN=79 Runoff=0.10 cfs 327 cf
Reach DP-1: Basin	Inflow=5.91 cfs 43,035 cf Outflow=5.91 cfs 43,035 cf
Reach DP-2: Onsite southeast	Inflow=0.86 cfs 3,103 cf Outflow=0.86 cfs 3,103 cf

Proposed-6 Prepared by {enter your company name here} HydroCAD® 10.00-26 s/n 04015 © 2020 HydroCAD Software Solutions LLC	Type III 24-hr 25-Year Rainfall=6.35" Printed 2/2/2023 Page 6		
Reach DP-3: Off-site West	Inflow=0.10 cfs 327 cf		
	Outflow=0.10 cfs 327 cf		
Pond S-1: Rip Rap Swale	Peak Elev=334.06' Storage=216 cf Inflow=0.11 cfs 360 cf		
	Outflow=0.01 cfs 145 cf		
Pond S-2: Rip Rap Swale	Peak Elev=332.51' Storage=199 cf Inflow=0.11 cfs 489 cf		
	Outflow=0.02 cfs 291 cf		
Pond S-3: Rip Rap Swale	Peak Elev=331.01' Storage=196 cf Inflow=0.11 cfs 644 cf		
	Outflow=0.03 cfs 450 cf		
Pond S-4: Rip Rap Swale	Peak Elev=329.26' Storage=149 cf Inflow=0.10 cfs 761 cf		
	Outflow=0.04 cfs 615 cf		
Pond SSD-1: Subsurface Detention Basin-1	Peak Elev=324.55' Storage=13,040 cf Inflow=9.97 cfs 36,378 cf		
	Outflow=4.35 cfs 34,861 cf		

Total Runoff Area = 105,367 sf Runoff Volume = 48,735 cf Average Runoff Depth = 5.55" 24.00% Pervious = 25,289 sf 76.00% Impervious = 80,078 sf

			thod, UH=S iinfall=6.35'		nted-CN, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs	
	Area (sf)		Description			
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(min)		(ft/ft)	(ft/sec)	Capacity (cfs)		
7.3 2.7		0.0800 0.0240			Sheet Flow, Sheet Flow Woods: Light underbrush n= 0.400 P2= 3.05" Shallow Concentrated Flow,	
10.0	461	Total			Unpaved Kv= 16.1 fps	—
		tor your			Type III 24-hr 25-Year Rainfall=6.3	
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	oy SCS T 24-hr 25					/eighte	ed-CN,	, Time	Span=	0.00-8	0.00 hrs	s, dt= 0).05 h	rs							
	Area (sf)	CN	Descr																		
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	<u>458</u> 3,571	79 96	50-75 Weigh	nted Av	verage	e	air, HS	GC													-
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<u>(min)</u> 6.0	(feet)	(ft/1	t) (ft	/sec)	(cfs)	Direct	Entry	, Min.												-
	sed-6														Type	III 24	-hr 2:	5-Yea	ar Rain	fall=6.35	
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Prepared by {en <u>HydroCAD® 10.00</u> 40] Hint: Not Des nflow Area = nflow = Dutflow = Routing by Stor-In	-26 s/n 0401 scribed (Out 98,172 5.91 cfs (5.91 cfs (nd+Trans m	15 © 2020 HydroCA tflow=Inflow) 2 sf, 75.63% Impe @ 12.19 hrs, Vo @ 12.19 hrs, Vo @ 12.19 hrs, Vo nethod, Time Span	AD Software Solutions Summary f ervious, Inflow Dept plume= 43,0 plume= 43,0 n= 0.00-80.00 hrs, d Read	for Reach DP-1 th = 5.26" for 2 335 cf 335 cf, Atten= 0%, it= 0.05 hrs ch DP-1: Basin graph	5-Year event Lag= 0.0 mi	n		Printe	d 2/2/2023
Prepared by {en HydroCAD® 10.00 40] Hint: Not Des nflow Area = nflow = Dutflow =	-26 s/n 0401 scribed (Out 98,172 5.91 cfs (5.91 cfs (nd+Trans m	15 © 2020 HydroCA tflow=Inflow) 2 sf, 75.63% Impe @ 12.19 hrs, Vo @ 12.19 hrs, Vo @ 12.19 hrs, Vo nethod, Time Span	AD Software Solutions Summary f ervious, Inflow Dept plume= 43,0 plume= 43,0 n= 0.00-80.00 hrs, d Read	for Reach DP-1 th = 5.26" for 2 335 cf 335 cf, Atten= 0%, it= 0.05 hrs ch DP-1: Basin graph	5-Year event Lag= 0.0 mi	n		Printe	d 2/2/2023

Type III 24-hr 25-Year Rainfall=6.35"

Printed 2/2/2023

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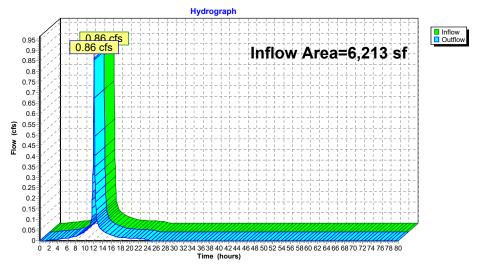
Summary for Reach DP-2: Onsite southeast

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	6,213 sf	, 93.87% Impervious,	Inflow Depth = 5.9	9" for 25-Year event
Inflow	=	0.86 cfs @	12.09 hrs, Volume=	3,103 cf	
Outflow	=	0.86 cfs @	12.09 hrs, Volume=	3,103 cf, A	tten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs

Reach DP-2: Onsite southeast



Proposed-6

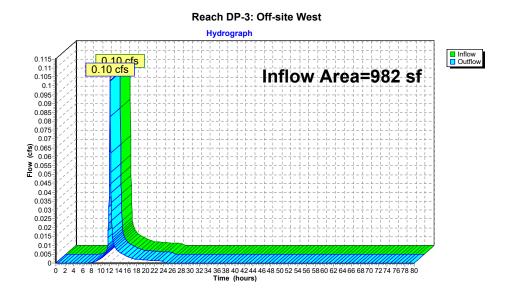
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Summary for Reach DP-3: Off-site West

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	982 sf,	0.00% Impervious,	Inflow Depth = 3.99"	for 25-Year event
Inflow	=	0.10 cfs @ 1	12.09 hrs, Volume=	327 cf	
Outflow	=	0.10 cfs @ 1	12.09 hrs, Volume=	327 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs



Summary for Pond S-1: Rip Rap Swale

Inflow Area =	1,317 sf, 0.00% Impervious,	Inflow Depth = 3.28" for 25-Year event
Inflow =	0.11 cfs @ 12.09 hrs, Volume=	360 cf
Outflow =	0.01 cfs @ 13.04 hrs, Volume=	145 cf, Atten= 90%, Lag= 56.7 min
Primary =	0.01 cfs @ 13.04 hrs, Volume=	145 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 334.06' @ 13.04 hrs Surf.Area= 200 sf Storage= 216 cf

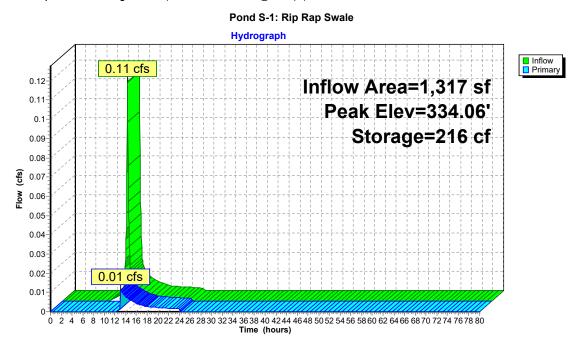
Plug-Flow detention time= 288.6 min calculated for 145 cf (40% of inflow) Center-of-Mass det. time= 164.1 min (994.9 - 830.9)

#4	004 451		Storage	Storage Description		h - l (D l -)	
#1	331.15'		120 cf	Custom Stage Dat			
40	000.051		444-5	414 cf Overall - 114			
#2	332.65'		114 cf	Custom Stage Dat		below (Recalc)	Inside #1
			234 cf	Total Available Stor	age		
			. .		0 01		
Elevation		Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(feet)	((sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
331.15		0	0.0	0	0	0	
331.50		21	27.2	2	2	59	
332.50		171	99.3	84	86	788	
332.65		200	104.4	28	114	872	
334.15		200	104.4	300	414	1,028	
Elevation	Surf	Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(feet)	((sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
332.65		0	0.0	0	0	0	
333.00		21	27.2	2	2	59	
334.00		171	99.3	84	86	788	
334.15		200	104.4	28	114	872	
Device Re	outing	Inv	ert Outle	et Devices			
#1 Pi	imary	334.0		long Sharp-Crested		- 2 End Control	tion(a)

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Type III 24-hr 25-Year Rainfall=6.35" Printed 2/2/2023 Page 28

Primary OutFlow Max=0.01 cfs @ 13.04 hrs HW=334.05' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.01 cfs @ 0.23 fps)



Summary for Pond S-2: Rip Rap Swale

Inflow Area =	2,501 sf, 0.00% Impervious,	Inflow Depth = 2.35" for 25-Year event
Inflow =	0.11 cfs @ 12.09 hrs, Volume=	489 cf
Outflow =	0.02 cfs @ 13.06 hrs, Volume=	291 cf, Atten= 80%, Lag= 57.8 min
Primary =	0.02 cfs @ 13.06 hrs, Volume=	291 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 332.51' @ 13.06 hrs Surf.Area= 200 sf Storage= 199 cf

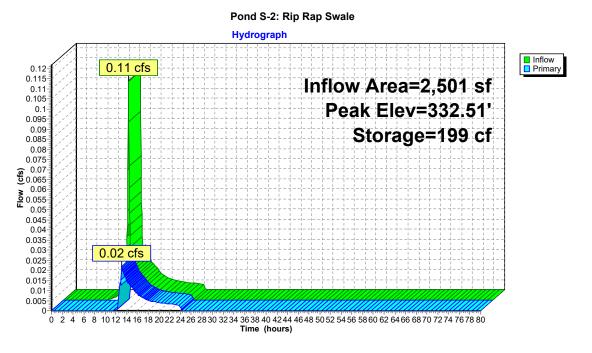
Plug-Flow detention time= 242.6 min calculated for 291 cf (59% of inflow) Center-of-Mass det. time= 110.2 min (986.7 - 876.4)

#1	329.65'	120 c		ata (Irregular)Listed			
#2	331.15'	107 c		07 cf Embedded = 30 ata (Irregular)Listed			
	001110	227 c					
Elevation (feet)	Surf.A	Area Perim q-ft) (fee		Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
329.65	(0	0 0.	/ / /	0	0		
330.50		74 61.	1 21	21	298		
331.15		200 104.	5 86	107	873		
332.65		200 104.	5 300	407	1,029		
Elevation	Surf.A	rea Perim	. Inc.Store	Cum.Store	Wet.Area		
(feet)	(s	q-ft) (feet	t) (cubic-feet)	(cubic-feet)	(sq-ft)		
331.15		0 0.	0 0	0	0		
332.00		74 61.	1 21	21	298		
332.65		200 104.	5 86	107	873		
Device Ro	outing	Invert Ou	Itlet Devices				
#1 Pr	imary	332.50' 7.0)' long Sharp-Creste	d Rectangular Wei	r 2 End Contrac	ction(s)	

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Type III 24-hr 25-Year Rainfall=6.35" Printed 2/2/2023 Page 30

Primary OutFlow Max=0.01 cfs @ 13.06 hrs HW=332.51' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.01 cfs @ 0.24 fps)



Summary for Pond S-3: Rip Rap Swale

Inflow Area =	3,685 sf, 0.00% Impervious,	Inflow Depth = 2.10" for 25-Year event
Inflow =	0.11 cfs @ 12.09 hrs, Volume=	644 cf
Outflow =	0.03 cfs @ 13.06 hrs, Volume=	450 cf, Atten= 71%, Lag= 58.2 min
Primary =	0.03 cfs @ 13.06 hrs, Volume=	450 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 331.01' @ 13.06 hrs Surf.Area= 200 sf Storage= 196 cf

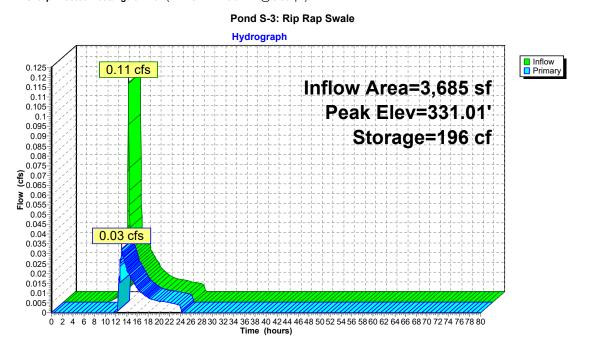
Plug-Flow detention time= 192.8 min calculated for 450 cf (70% of inflow) Center-of-Mass det. time= 81.0 min (978.3 - 897.4)

Volume	Invert	Avail.	Storage	Storage Description	า		
#1 #2	329.70' 328.20'		112 cf 120 cf	Custom Stage Dat Custom Stage Dat 412 cf Overall - 112	a (Irregular)Listed	below (Recalc)	
			232 cf	Total Available Stor	rage		
Elevation (feet)		f.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
329.70		0	0.0	0	0	0	
330.00		16	23.0	2	2	42	
331.00		158	95.1	75	76	722	
331.20		200	104.5	36	112	873	
Elevation	Surf	f.Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(feet)		(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
328.20		0	0.0	0	0	0	
328.50		16	23.0	2	2	42	
329.50		158	95.1	75	76	722	
329.70		200	104.5	36	112	873	
331.20		200	104.5	300	412	1,030	
	outing rimary	Inv 331.0		et Devices long Sharp-Crested	l Rectangular Wei	r 2 End Contrac	ction(s)

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Type III 24-hr 25-Year Rainfall=6.35" Printed 2/2/2023 Page 32

Primary OutFlow Max=0.02 cfs @ 13.06 hrs HW=331.01' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.02 cfs @ 0.33 fps)



Summary for Pond S-4: Rip Rap Swale

Inflow Area =	4,756 sf, 0.00% Impervious,	Inflow Depth = 1.92" for 25-Year event
Inflow =	0.10 cfs @ 12.09 hrs, Volume=	761 cf
Outflow =	0.04 cfs @ 13.06 hrs, Volume=	615 cf, Atten= 57%, Lag= 58.2 min
Primary =	0.04 cfs @ 13.06 hrs, Volume=	615 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 329.26' @ 13.06 hrs Surf.Area= 188 sf Storage= 149 cf

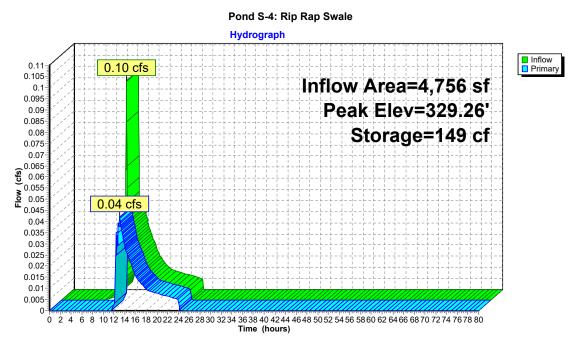
Plug-Flow detention time= 131.6 min calculated for 615 cf (81% of inflow) Center-of-Mass det. time= 50.0 min (966.2 - 916.2)

/olume #1 #2	Invert Av 328.20' 326.70'	<u>ail.Storage/</u> 108 cf 113 cf	Storage Description Custom Stage Date Custom Stage Date	ta (Irregular)Listed		nside #2
			390 cf Overall - 108			ds
		221 cf	Total Available Sto	rage		
Elevation	Surf.Area		Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft	:) (feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
328.20	(0.0	0	0	0	
329.00	7	1 56.9	19	19	259	
329.25	11:	2 76.4	23	42	466	
329.70	188	B 102.0	67	108	832	
Elevation	Surf.Area	a Perim.	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft) (feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
326.70	(0.0	0	0	0	
327.50	7.	1 56.9	19	19	259	
327.75	112	2 76.4	23	42	466	
328.20	188	8 102.0	67	108	832	
329.70	188	B 102.0	282	390	985	

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Type III 24-hr 25-Year Rainfall=6.35" Printed 2/2/2023 Page 34

Primary OutFlow Max=0.02 cfs @ 13.06 hrs HW=329.26' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.02 cfs @ 0.34 fps)



Summary for Pond SSD-1: Subsurface Detention Basin-1

Inflow Area =	71,428 sf, 99.59% Impervious,	Inflow Depth = 6.11" for 25-Year event
Inflow =	9.97 cfs @ 12.09 hrs, Volume=	36,378 cf
Outflow =	4.35 cfs @ 12.27 hrs, Volume=	34,861 cf, Atten= 56%, Lag= 11.3 min
Primary =	4.35 cfs @ 12.27 hrs, Volume=	34,861 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 9 Peak Elev= 324.55' @ 12.27 hrs Surf.Area= 7,434 sf Storage= 13,040 cf

Plug-Flow detention time= 153.8 min calculated for 34,839 cf (96% of inflow) Center-of-Mass det. time= 130.5 min (874.8 - 744.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	322.00'	6,586 cf	63.25'W x 117.54'L x 3.50'H Field A
			26,020 cf Overall - 9,556 cf Embedded = 16,464 cf x 40.0% Voids
#2A	322.50'	9,556 cf	ADS_StormTech SC-740 +Cap x 208 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			208 Chambers in 13 Rows
		16,141 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	322.50'	15.0" Round Culvert L= 21.3' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 322.50' / 322.00' S= 0.0235 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf
#2	Device 1	325.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Device 1	324.25'	6.0" Vert. Orifice/Grate C= 0.600
#4	Device 1	323.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
#5	Device 1	322.50'	6.0" Vert. Orifice/Grate C= 0.600
#4	Device 1	323.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600

Proposed-6

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Type III 24-hr 25-Year Rainfall=6.35" Printed 2/2/2023 Page 36

Primary OutFlow Max=4.33 cfs @ 12.27 hrs HW=324.55' (Free Discharge) 1=Culvert (Passes 4.33 cfs of 7.05 cfs potential flow) 2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs) 3=Orifice/Grate (Orifice Controls 0.23 cfs @ 1.86 fps) 4=Orifice/Grate (Orifice Controls 2.84 cfs @ 4.07 fps) 5=Orifice/Grate (Orifice Controls 1.27 cfs @ 6.46 fps)

Pond SSD-1: Subsurface Detention Basin-1 - Chamber Wizard Field A

Chamber Model = ADS_StormTech SC-740 +Cap (ADS StormTech® SC-740 with cap length) Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

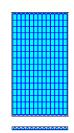
16 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 115.54' Row Length +12.0" End Stone x 2 = 117.54' Base Length 13 Rows x 51.0" Wide + 6.0" Spacing x 12 + 12.0" Side Stone x 2 = 63.25' Base Width 6.0" Base + 30.0" Chamber Height + 6.0" Cover = 3.50' Field Height

208 Chambers x 45.9 cf = 9,555.5 cf Chamber Storage

26,019.7 cf Field - 9,555.5 cf Chambers = 16,464.2 cf Stone x 40.0% Voids = 6,585.7 cf Stone Storage

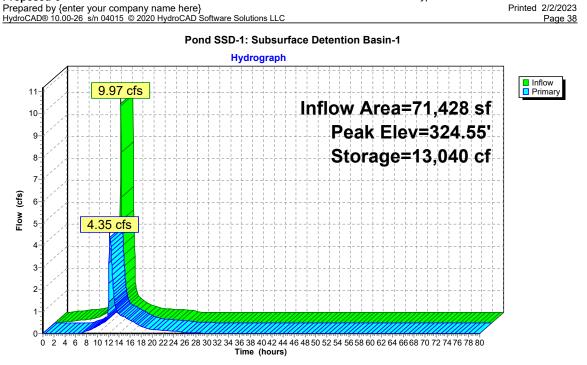
Chamber Storage + Stone Storage = 16,141.2 cf = 0.371 af Overall Storage Efficiency = 62.0%Overall System Size = 117.54' x 63.25' x 3.50'

208 Chambers 963.7 cy Field 609.8 cy Stone



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Type III 24-hr 25-Year Rainfall=6.35" Printed 2/2/2023





HydroCAD Analysis

Proposed Conditions - 100 Year Storm

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Area	CN	Description
(sq-ft)		(subcatchment-numbers)
18,555	79	50-75% Grass cover, Fair, HSG C (P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3)
80,078	98	Paved parking (P-1a, P-1b, P-2)
1,396	66	Rip Rap Swale (P-1c, P-1d, P-1e, P-1f)
5,338	70	Woods, Good, HSG C (P-1, P-1c, P-1d, P-1e, P-1f, P-3)
105,367	93	TOTAL AREA

Area Listing (all nodes)

Soil Listing (all nodes)

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Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
0	HSG A	
0	HSG B	
23,893	HSG C	P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3
0	HSG D	
81,474	Other	P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2
105,367		TOTAL AREA

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			Gro	und Covers	(all nodes))	
 HSG-A (sq-ft)	HSG-B (sq-ft)	HSG-C (sq-ft)	HSG-D (sq-ft)	Other (sq-ft)	Total (sq-ft)	Ground Cover	Subcatchment Numbers
 0	0	18,555	0	0	18,555	50-75% Grass cover, Fair	P-1, P-1a, P-1b, P-1c, P-1d, P-1e, P-1f, P-2, P-3
0	0	0	0	80,078	80,078	Paved parking	P-1a, P-1b, P-2
0	0	0	0	1,396	1,396	Rip Rap Swale	P-1c, P-1d, P-1e, P-1f
0	0	5,338	0	0	5,338	Woods, Good	P-1, P-1c, P-1d, P-1e, P-1f, P-3
0	0	23,893	0	81,474	105,367	TOTAL AREA	

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					Pip	e Listin	g (all nodes	5)		
Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)	
1	SSD-1	322.50	322.00	21.3	0.0235	0.013	15.0	0.0	0.0	

Time span=0.00-80.00 hrs, dt=0.05 hrs, 1601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment P-1: Flow to Existing Basin	Runoff Area=18,417 sf 0.00% Impervious Runoff Depth=5.42" Flow Length=461' Tc=10.0 min CN=77 Runoff=2.30 cfs 8,320 cf
Subcatchment P-1a: Flow to SSI-1	Runoff Area=71,428 sf 99.59% Impervious Runoff Depth=7.92" Tc=6.0 min CN=98 Runoff=12.83 cfs 47,143 cf
Subcatchment P-1b: Flow to Treatment Unit	Runoff Area=3,571 sf 87.17% Impervious Runoff Depth=7.68" Tc=6.0 min CN=96 Runoff=0.64 cfs 2,286 cf
Subcatchment P-1c: Fllow to Swale	Runoff Area=1,317 sf 0.00% Impervious Runoff Depth=4.84" Tc=6.0 min CN=72 Runoff=0.17 cfs 531 cf
Subcatchment P-1d: Fllow to Swale	Runoff Area=1,184 sf 0.00% Impervious Runoff Depth=5.07" Tc=6.0 min CN=74 Runoff=0.16 cfs 500 cf
Subcatchment P-1e: Fllow to Swale	Runoff Area=1,184 sf 0.00% Impervious Runoff Depth=5.19" Tc=6.0 min CN=75 Runoff=0.16 cfs 512 cf
Subcatchment P-1f: Fllow to Swale	Runoff Area=1,071 sf 0.00% Impervious Runoff Depth=5.07" Tc=6.0 min CN=74 Runoff=0.14 cfs 452 cf
Subcatchment P-2: Flow onsite Southeast	Runoff Area=6,213 sf 93.87% Impervious Runoff Depth=7.80" Tc=6.0 min CN=97 Runoff=1.11 cfs 4,039 cf
Subcatchment P-3: Flow off-site West	Runoff Area=982 sf 0.00% Impervious Runoff Depth=5.66" Tc=6.0 min CN=79 Runoff=0.14 cfs 463 cf
Reach DP-1: Basin	Inflow=8.86 cfs 57,484 cf Outflow=8.86 cfs 57,484 cf
Reach DP-2: Onsite southeast	Inflow=1.11 cfs 4,039 cf Outflow=1.11 cfs 4,039 cf

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Reach DP-3: Off-site West	Inflow=0.14 cfs 463 cf
	Outflow=0.14 cfs 463 cf
Pond S-1: Rip Rap Swale	Peak Elev=334.07' Storage=219 cf Inflow=0.17 cfs 531 cf
	Outflow=0.09 cfs 319 cf
Pond S-2: Rip Rap Swale	Peak Elev=332.54' Storage=205 cf Inflow=0.18 cfs 819 cf
	Outflow=0.17 cfs 619 cf
Pond S-3: Rip Rap Swale	Peak Elev=331.05' Storage=203 cf Inflow=0.27 cfs 1,131 cf
	Outflow=0.25 cfs 940 cf
Pond S-4: Rip Rap Swale	Peak Elev=329.31' Storage=156 cf Inflow=0.34 cfs 1,392 cf
	Outflow=0.32 cfs 1,248 cf
Pond SSD-1: Subsurface Detention Basin-1	Peak Elev=325.21' Storage=15,266 cf Inflow=12.83 cfs 47,143 cf
	Outflow=6.21 cfs 45,629 cf

Total Runoff Area = 105,367 sf Runoff Volume = 64,245 cf Average Runoff Depth = 7.32" 24.00% Pervious = 25,289 sf 76.00% Impervious = 80,078 sf

4		0-rearry	ainfall=8.16	0	
	Area (sf)		Description Noods, Go		
	4,454 13,963	79 5	50-75% Gra	ass cover, I	, Fair, HSG C
	18,417 18,417	77	Weighted A 100.00% Pe	verage ervious Are	pa
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
7.3	50	0.0800	0.11	()	Sheet Flow, Sheet Flow Woods: Light underbrush n= 0.400 P2= 3.05" Shallow Concentrated Flow.
10.0		Total			Unpaved Kv= 16.1 fps
Propos Prepare HydroCA	ed by {en	ter your -26 s/n 04	company 4015 © 202	0 HydroCAE	D Software Solutions LLC Page 8
Prepare	ed by {en	ter your -26 s/n 0-	company 4015 © 202	0 HydroCAE	e} Printed 2/2/2023

		Summary for Sub	ocatchment P-1a: Flow to SSI-1	
noff =	12.83 cfs @ 12.09 h	rs, Volume= 47	,143 cf, Depth= 7.92"	
noff by SCS TR be III 24-hr 100	8-20 method, UH=SCS 9-Year Rainfall=8.16"	S, Weighted-CN, Time Sp	pan= 0.00-80.00 hrs, dt= 0.05 hrs	
Area (sf)	CN Description			
71,133 295		cover, Fair, HSG C		
71,428 295	98 Weighted Aver 0.41% Perviou	is Area		
71,133	99.59% Imperv			
Tc Length min) (feet)	Slope Velocity C (ft/ft) (ft/sec)	apacity Description (cfs)		
6.0		Direct Entry, I	Min.	
oposed-6			Type III 24-hr 100-Yi	ear Rainfall=8.16"
	er your company na	me here}	Type III 24-hr 100-Ye	Printed 2/2/2023
	er your company na 26 s/n 04015 © 2020 H	me here} lydroCAD Software Solutio	ns LLC	
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14 13 12 11		Subcatchn	ns LLC nent P-1a: Flow to SSI-1 ograph Type III 24-hr 100-Year Rainfall=8.16"	Printed 2/2/2023 Page 10
14-11-11-11-11-11-11-11-11-11-11-11-11-1		Subcatchn	ns LLC nent P-1a: Flow to SSI-1 ograph Type III 24-hr 100-Year Rainfall=8.16" Runoff Area=71,428 sf	Printed 2/2/2023 Page 10
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st s		Subcatchn	ns LLC nent P-1a: Flow to SSI-1 ograph Type III 24-hr 100-Year Rainfall=8.16" Runoff Area=71,428 sf Runoff Volume=47,143 cf Runoff Depth=7.92" Tc=6.0 min	Printed 2/2/2023 Page 10

Runoff	=	0.64	cfs @	2 12.09	9 hrs,	Volum	e=	2,286	cf, De	epth= 7.68	3"							
						eighteo	d-CN, Ti	me Span=	- 0.00-8	30.00 hrs,	dt= 0.0	5 hrs						
	24-hr 10				5"													
A	<u>rea (sf)</u> 3,113	<u>CN</u> 98		<u>cription</u> ed parki	ing													-
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upoff -	0.46	fc @ 10 (-	ubcatchment P-					
unoff = 0.000		•)9 hrs, Volu		512 cf, Depth=		e			
Type III 24-hr 10					e Span= 0.00-80.00	ms, ut- 0.05 M	5			
Area (sf) 350	<u>CN</u> 66	Description Rip Rap Sv								
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Tc Length (min) (feet				Description						
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pe III 2	/ SCS TI 4-hr 10	R-20 me 0-Year l	thod, UH Rainfall=8	=SCS, W .16"	eighte	ed-CN, T	Time Sp	pan= 0.00-80).00 hrs, dt= (0.05 hrs				
Ar	ea (sf)	CN	Descripti	on										
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							•		chment									
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Ai	<u>rea (sf)</u> 5,832	<u>CN</u> 98	Desci	ription d parki														
	<u>381</u> 6,213	79 97		i% Gra	ass co		Fair, HS	SG C										
	381 5,832	51	6.13%	% Perv % Imp	ious	Area	ea											
Тс	Length	Slop					Desci	ription										
<u>(min)</u> 6.0	(feet)	(ft/f	t) (ft	/sec)		(cfs)	Direc	t Entry, I	Vin.									
															1 br 1			-0 16"
		ter you	Ir com	pany	nam	e here	ə}						Туј	be III 2-	4-hr 10		ar Rainfall= Printed 2/2	/2023
		ter you 26 s/n	ır com 04015	pany © 2020	nam 0 Hyd			are Solutio						pe III 2-	4-hr 10		Printed 2/2	
		ter you 26 s/n	ır com 04015	pany © 2020	nam 0 Hyd			chment	: P-2: Flo	ow onsi	te Sou	Itheas		pe III 2	4-hr 10		Printed 2/2	/2023
Propos Prepare HydroCAI		ter you 26 s/n	1r com 04015	pany © 2020	nam 0 Hyd			chment		ow onsi	te Sou	Itheas		pe III 2	4-hr 10		Printed 2/2 Pa	2/2023 lige 22
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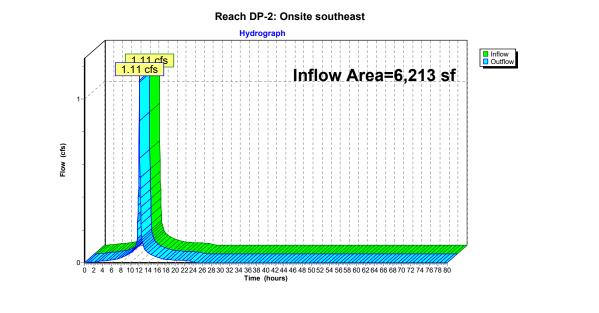
Runoff	=	0.14 c	fs @ 12.0	09 hrs, Volur	me=	463 0	r, Depth= 5.66	6"			
	SCS TR		-				0.00-80.00 hrs,				
Type III 24							,				
Are	ea (sf) 10		Description	n bod, HSG C							
	972	79	50-75% Gi	ass cover, F	air, HSG C						
	982 982		Veighted / 100.00% F	Average Pervious Area	1						
	_ength			Capacity	Descriptio	n					
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		er you	company	name here	}				Type III 24-hr	100-Year Ra Print	ed 2/2/2023
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Prepared HydroCAD@	by {ent <u>® 10.00-</u> 2						C Reach DP-1:		Type III 24-hr		ed 2/2/2023
Prepared HydroCAD@ [40] Hint: N	by {ent <u>® 10.00-2</u> Not Desc	ribed (Outflow=In	flow)	Summ	nary for I	Reach DP-1:	Basin	Гуре III 24-hr		ed 2/2/2023
Prepared HydroCAD@ [40] Hint: N Inflow Area Inflow	by {ent <u>8</u> 10.00-2 Not Desc a = =	ribed (98, 8.86 c	Dutflow=In 172 sf, 75. fs @ 12.′	flow) 63% Impervi 18 hrs, Volur	Summ ious, Inflov me=	nary for I v Depth = 57,484 c	Reach DP-1: 7.03" for 10	Basin 0-Year event	Гуре III 24-hr		ed 2/2/2023
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Prepared HydroCAD@ [40] Hint: N Inflow Area Inflow Outflow	by {ent <u>® 10.00-</u> 2 Not Deso a = = =	ribed (98, 8.86 c 8.86 c	Dutflow=In 172 sf, 75. fs @ 12. fs @ 12. s method,	flow) 63% Impervi 18 hrs, Volur 8 hrs, Volur Time Span=	Summ ious, Inflov ne= ne= 0.00-80.00	nary for I v Depth = 57,484 c 57,484 c hrs, dt= 0 Reach I	Reach DP-1: 7.03" for 10 f f, Atten= 0%, .05 hrs DP-1: Basin h	Basin 0-Year event Lag= 0.0 min			ed 2/2/2023
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Prepared HydroCAD@ [40] Hint: N Inflow Area Inflow Outflow	by {ent <u>® 10.00-</u> 2 Not Deso a = = =	eribed (98, 8.86 c 8.86 c d+Tran	Dutflow=In 172 sf, 75. fs @ 12. fs @ 12. s method,	flow) 63% Impervi 18 hrs, Volur 8 hrs, Volur Time Span=	Summ ious, Inflov ne= ne= 0.00-80.00	nary for I v Depth = 57,484 c 57,484 c hrs, dt= 0 Reach I	Reach DP-1: 7.03" for 10 f f, Atten= 0%, .05 hrs DP-1: Basin h	Basin 0-Year event Lag= 0.0 min		Print	ed 2/2/2023
Prepared HydroCAD@ [40] Hint: N Inflow Area Inflow Outflow	by {ent <u>® 10.00-</u> 2 Not Deso a = = =	cribed (98, 8.86 c 8.86 c d+Tran	Dutflow=In 172 sf, 75. fs @ 12. fs @ 12. s method,	flow) 63% Impervi 18 hrs, Volur 8 hrs, Volur Time Span=	Summ ious, Inflov ne= ne= 0.00-80.00	nary for I v Depth = 57,484 c 57,484 c hrs, dt= 0 Reach I	Reach DP-1: 7.03" for 10 f f, Atten= 0%, .05 hrs DP-1: Basin h	Basin 0-Year event Lag= 0.0 min		Print	ed 2/2/2023
[40] Hint: N Inflow Area Inflow Outflow	by {ent <u>₿ 10.00-2</u> Not Desc a = = = / Stor-In	cribed (98, 8.86 c 8.86 c d+Tran	Dutflow=In 172 sf, 75. fs @ 12. fs @ 12. s method,	flow) 63% Impervi 18 hrs, Volur 8 hrs, Volur Time Span=	Summ ious, Inflov ne= ne= 0.00-80.00	nary for I v Depth = 57,484 c 57,484 c hrs, dt= 0 Reach I	Reach DP-1: 7.03" for 10 f f, Atten= 0%, .05 hrs DP-1: Basin h	Basin 0-Year event Lag= 0.0 min		Print	ed 2/2/2023
Prepared HydroCAD@ [40] Hint: N Inflow Area Inflow Outflow	by {ent <u>® 10.00-</u> 2 Not Deso a = = =	cribed (98, 8.86 c 8.86 c d+Tran	Dutflow=In 172 sf, 75. fs @ 12. fs @ 12. s method,	flow) 63% Impervi 18 hrs, Volur 8 hrs, Volur Time Span=	Summ ious, Inflov ne= ne= 0.00-80.00	nary for I v Depth = 57,484 c 57,484 c hrs, dt= 0 Reach I	Reach DP-1: 7.03" for 10 f f, Atten= 0%, .05 hrs DP-1: Basin h	Basin 0-Year event Lag= 0.0 min		Print	ed 2/2/2023
Prepared HydroCAD@ [40] Hint: N Inflow Area Inflow Outflow	by {ent <u>₿ 10.00-2</u> Not Desc a = = = / Stor-In	cribed (98, 8.86 c 8.86 c d+Tran	Dutflow=In 172 sf, 75. fs @ 12. fs @ 12. s method,	flow) 63% Impervi 18 hrs, Volur 8 hrs, Volur Time Span=	Summ ious, Inflov ne= ne= 0.00-80.00	nary for I v Depth = 57,484 c 57,484 c hrs, dt= 0 Reach I	Reach DP-1: 7.03" for 10 f f, Atten= 0%, .05 hrs DP-1: Basin h	Basin 0-Year event Lag= 0.0 min		Print	ed 2/2/2023
Prepared HydroCAD@ [40] Hint: N Inflow Area Inflow Outflow	by {ent <u>₿ 10.00-2</u> Not Desc a = = = / Stor-In	cribed (98, 8.86 c 8.86 c d+Tran	Dutflow=In 172 sf, 75. fs @ 12. fs @ 12. s method,	flow) 63% Impervi 18 hrs, Volur 8 hrs, Volur Time Span=	Summ ious, Inflov ne= ne= 0.00-80.00	nary for I v Depth = 57,484 c 57,484 c hrs, dt= 0 Reach I	Reach DP-1: 7.03" for 10 f f, Atten= 0%, .05 hrs DP-1: Basin h	Basin 0-Year event Lag= 0.0 min		Print	ed 2/2/2023
Prepared HydroCAD@ [40] Hint: N Inflow Area Inflow Outflow	by {ent <u>₿ 10.00-2</u> Not Desc a = = = / Stor-In	bribed (98,86 c 8.86 c 4+Tran	Dutflow=In 172 sf, 75. fs @ 12. fs @ 12. s method,	flow) 63% Impervi 18 hrs, Volur 8 hrs, Volur Time Span=	Summ ious, Inflov ne= ne= 0.00-80.00	nary for I v Depth = 57,484 c 57,484 c hrs, dt= 0 Reach I	Reach DP-1: 7.03" for 10 f f, Atten= 0%, .05 hrs DP-1: Basin h	Basin 0-Year event Lag= 0.0 min		Print	ed 2/2/2023
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Summary for Reach DP-2: Onsite southeast

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	a =	6,213 sf, 93.87% Imperv	ous, Inflow Depth = 7.80"	for 100-Year event
Inflow	=	1.11 cfs @ 12.09 hrs, Volu	ne= 4,039 cf	
Outflow	=	1.11 cfs @ 12.09 hrs, Volu	ne= 4,039 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs



Proposed-6

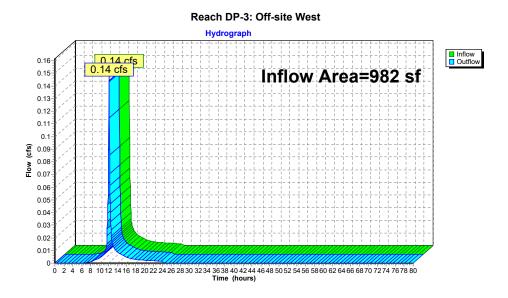
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Summary for Reach DP-3: Off-site West

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	982 sf,	0.00% Impervious,	Inflow Depth = 5.66"	for 100-Year event
Inflow	=	0.14 cfs @ 1	12.09 hrs, Volume=	463 cf	
Outflow	=	0.14 cfs @ 1	12.09 hrs, Volume=	463 cf, Atte	n= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs



Summary for Pond S-1: Rip Rap Swale

Inflow Area =	1,317 sf, 0.00% Impervious,	Inflow Depth = 4.84" for 100-Year event
Inflow =	0.17 cfs @ 12.09 hrs, Volume=	531 cf
Outflow =	0.09 cfs @ 12.25 hrs, Volume=	319 cf, Atten= 48%, Lag= 9.4 min
Primary =	0.09 cfs @ 12.25 hrs, Volume=	319 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 334.07' @ 12.25 hrs Surf.Area= 200 sf Storage= 219 cf

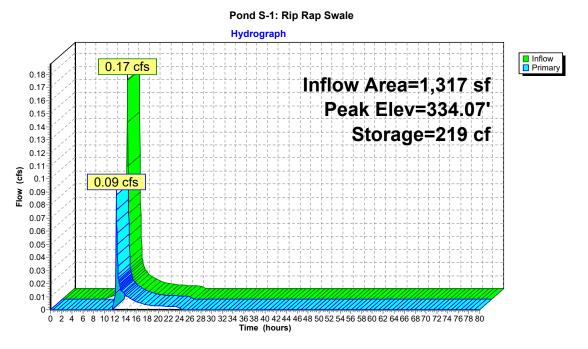
Plug-Flow detention time= 188.5 min calculated for 319 cf (60% of inflow) Center-of-Mass det. time= 82.2 min (901.9 - 819.7)

#1	331.15'	120 cf	Custom Stage Dat				
	000 051		414 cf Overall - 114				
#2	332.65'	114 cf	Custom Stage Dat	a (Irregular)Listed	below (Recalc)	Inside #1	
		234 cf	Total Available Stor	rage			
Elevation	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)		
331.15	0	0.0	0	0	0		
331.50	21	27.2	2	2	59		
332.50	171	99.3	84	86	788		
332.65	200	104.4	28	114	872		
334.15	200	104.4	300	414	1,028		
Elevation	Surf.Area	Perim.	Inc.Store	Cum.Store	Wet.Area		
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)		
332.65	0	0.0	0	0	0		
333.00	21	27.2	2	2	59		
334.00	171	99.3	84	86	788		
334.15	200	104.4	28	114	872		
Device Re	outing li	nvert Outl	et Devices				

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Primary OutFlow Max=0.08 cfs @ 12.25 hrs HW=334.07' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.08 cfs @ 0.49 fps)



Summary for Pond S-2: Rip Rap Swale

Inflow Area =	2,501 sf, 0.00% Impervious,	Inflow Depth = 3.93" for 100-Year event
Inflow =	0.18 cfs @ 12.22 hrs, Volume=	819 cf
Outflow =	0.17 cfs @ 12.23 hrs, Volume=	619 cf, Atten= 4%, Lag= 0.7 min
Primary =	0.17 cfs @ 12.23 hrs, Volume=	619 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 332.54' @ 12.23 hrs Surf.Area= 200 sf Storage= 205 cf

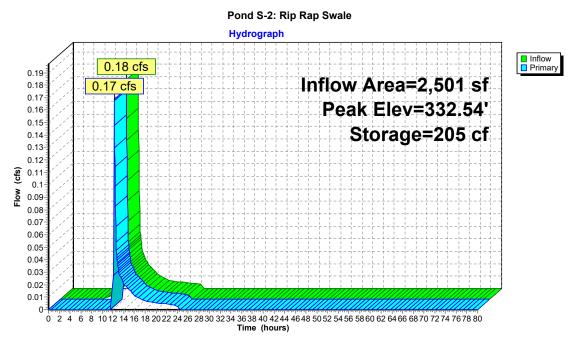
Plug-Flow detention time= 142.4 min calculated for 619 cf (76% of inflow) Center-of-Mass det. time= 51.0 min (900.2 - 849.2)

Volume	Invert	Avail.Storag	0				
#1	329.65'	120			a (Irregular)Listed		
					cf Embedded = 3		
#2	331.15'	107	of Custom Sta	age Dat	a (Irregular)Listed	below (Recalc)	Inside #1
		227	cf Total Availa	ble Stor	age		
Elevation	Surf.	Area Peri	m. Inc.S	store	Cum.Store	Wet.Area	
(feet)	(9	sq-ft) (fee	et) (cubic-t	feet)	(cubic-feet)	(sq-ft)	
329.65		0 0	0.0	0	0	0	
330.50		74 61	.1	21	21	298	
331.15		200 104	.5	86	107	873	
332.65		200 104	.5	300	407	1,029	
Elevation	Surf.	Area Peri	m. Inc.S	store	Cum.Store	Wet.Area	
(feet)	(5	sq-ft) (fee	et) (cubic-	feet)	(cubic-feet)	(sq-ft)	
331.15		0 0	0.0	0	0	0	
332.00		74 61	.1	21	21	298	
332.65		200 104	.5	86	107	873	
Device R	outing	Invert C	utlet Devices				
	rimarv	-		2 reated	Rectangular We	a O Fred Control	tion(a)

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Type III 24-hr 100-Year Rainfall=8.16" Printed 2/2/2023 Page 30

Primary OutFlow Max=0.16 cfs @ 12.23 hrs HW=332.54' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.16 cfs @ 0.63 fps)



Summary for Pond S-3: Rip Rap Swale

Inflow Area =	3,685 sf, 0.00% Impervious,	Inflow Depth = 3.68" for 100-Year event
Inflow =	0.27 cfs @ 12.22 hrs, Volume=	1,131 cf
Outflow =	0.25 cfs @ 12.24 hrs, Volume=	940 cf, Atten= 8%, Lag= 1.0 min
Primary =	0.25 cfs @ 12.24 hrs, Volume=	940 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 331.05' @ 12.24 hrs Surf.Area= 200 sf Storage= 203 cf

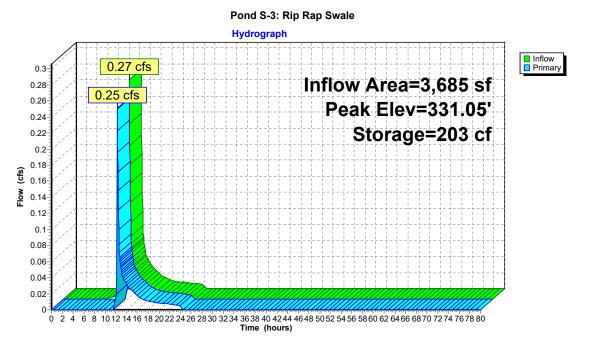
Plug-Flow detention time= 108.5 min calculated for 940 cf (83% of inflow) Center-of-Mass det. time= 35.7 min (896.6 - 860.9)

Volume	Invert	Avail.	Storage	Storage Description	า		
#1 #2	329.70' 328.20'		112 cf 120 cf	Custom Stage Dat Custom Stage Dat 412 cf Overall - 112	a (Irregular)Listed	below (Recalc))
			232 cf	Total Available Stor	rage		
Elevation (feet)		.Area sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
329.70		0	0.0	0	0	0	
330.00		16	23.0	2	2	42	
331.00		158	95.1	75	76	722	
331.20		200	104.5	36	112	873	
Elevation	Surf	Area	Perim.	Inc.Store	Cum.Store	Wet.Area	
(feet)	(sq-ft)	(feet)	(cubic-feet)	(cubic-feet)	(sq-ft)	
328.20		0	0.0	0	0	0	
328.50		16	23.0	2	2	42	
329.50		158	95.1	75	76	722	
329.70		200	104.5	36	112	873	
331.20		200	104.5	300	412	1,030	
Device Ro	outing	Inv	ert Outle	et Devices			
#1 Pr	rimary	331.0	00' 7.0'	ong Sharp-Crested	Rectangular Wei	r 2 End Contrac	ction(s)

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Primary OutFlow Max=0.25 cfs @ 12.24 hrs HW=331.05' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.25 cfs @ 0.72 fps)



Summary for Pond S-4: Rip Rap Swale

Inflow Area =	4,756 sf, 0.00% Impervious,	Inflow Depth = 3.51" for 100-Year event
Inflow =	0.34 cfs @ 12.22 hrs, Volume=	1,392 cf
Outflow =	0.32 cfs @ 12.23 hrs, Volume=	1,248 cf, Atten= 5%, Lag= 0.7 min
Primary =	0.32 cfs @ 12.23 hrs, Volume=	1,248 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 6 Peak Elev= 329.31' @ 12.23 hrs Surf.Area= 188 sf Storage= 156 cf

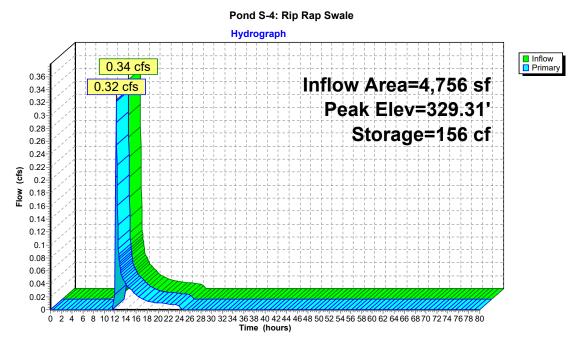
Plug-Flow detention time= 72.3 min calculated for 1,248 cf (90% of inflow) Center-of-Mass det. time= 21.9 min (892.2 - 870.3)

Volume	Invert	ert Avail.Storage		Storage Description				
#1 #2	328.20' 326.70'		108 cf 113 cf	Custom Stage Data (Irregular)Listed below (Recalc) Inside #2 Custom Stage Data (Irregular)Listed below (Recalc) 390 cf Overall - 108 cf Embedded = 282 cf x 40.0% Voids				
			221 cf	Total Available Storage				
Elevation (feet)	Surf. (s	Area sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
328.20 329.00		0 71	0.0 56.9	0 19	0 19	0 259		
329.25 329.70		112 188	76.4 102.0	23 67	42 108	466 832		
Elevation (feet)	Surf.	Area sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
326.70	(*	0	0.0	0	0	<u>(34-11)</u> 0		
327.50 327.75		71 112	56.9 76.4	19 23	19 42	259 466		
328.20 329.70		188 188	102.0 102.0	67 282	108 390	832 985		
Device Ro	outing	Inv	ert Outle	et Devices				
#1 Pr	#1 Primary 329.25' 7.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)							

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Primary OutFlow Max=0.32 cfs @ 12.23 hrs HW=329.31' (Free Discharge) 1=Sharp-Crested Rectangular Weir (Weir Controls 0.32 cfs @ 0.79 fps)



Summary for Pond SSD-1: Subsurface Detention Basin-1

Inflow Area =	71,428 sf, 99.59% Impervious,	Inflow Depth = 7.92" for 100-Year event
Inflow =	12.83 cfs @ 12.09 hrs, Volume=	47,143 cf
Outflow =	6.21 cfs @ 12.25 hrs, Volume=	45,629 cf, Atten= 52%, Lag= 9.6 min
Primary =	6.21 cfs @ 12.25 hrs, Volume=	45,629 cf

Routing by Stor-Ind method, Time Span= 0.00-80.00 hrs, dt= 0.05 hrs / 9 Peak Elev= 325.21' @ 12.25 hrs Surf.Area= 7,434 sf Storage= 15,266 cf

Plug-Flow detention time= 135.8 min calculated for 45,629 cf (97% of inflow) Center-of-Mass det. time= 115.6 min (856.6 - 741.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	322.00'	6,586 cf	63.25'W x 117.54'L x 3.50'H Field A
			26,020 cf Overall - 9,556 cf Embedded = 16,464 cf x 40.0% Voids
#2A	322.50'	9,556 cf	ADS_StormTech SC-740 +Cap x 208 Inside #1
			Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf
			Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap
			208 Chambers in 13 Rows
		16,141 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	322.50'	15.0" Round Culvert L= 21.3' RCP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 322.50' / 322.00' S= 0.0235 '/' Cc= 0.900 n= 0.013, Flow Area= 1.23 sf
#2	Device 1	325.25'	4.0' long Sharp-Crested Rectangular Weir 2 End Contraction(s)
#3	Device 1	324.25'	6.0" Vert. Orifice/Grate C= 0.600
#4	Device 1	323.50'	8.0" Vert. Orifice/Grate X 2.00 C= 0.600
#5	Device 1	322.50'	6.0" Vert. Orifice/Grate C= 0.600

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Primary OutFlow Max=6.21 cfs @ 12.25 hrs HW=325.20' (Free Discharge) 1=Culvert (Passes 6.21 cfs of 8.52 cfs potential flow) 2=Sharp-Crested Rectangular Weir (Controls 0.00 cfs) 3=Orifice/Grate (Orifice Controls 0.79 cfs @ 4.04 fps) 4=Orifice/Grate (Orifice Controls 3.94 cfs @ 5.64 fps) 5=Orifice/Grate (Orifice Controls 1.48 cfs @ 7.54 fps)

Pond SSD-1: Subsurface Detention Basin-1 - Chamber Wizard Field A

Chamber Model = ADS_StormTechSC-740 +Cap (ADS StormTech® SC-740 with cap length) Effective Size= 44.6"W x 30.0"H => 6.45 sf x 7.12'L = 45.9 cf Overall Size= 51.0"W x 30.0"H x 7.56'L with 0.44' Overlap

51.0" Wide + 6.0" Spacing = 57.0" C-C Row Spacing

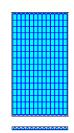
16 Chambers/Row x 7.12' Long +0.81' Cap Length x 2 = 115.54' Row Length +12.0" End Stone x 2 = 117.54' Base Length 13 Rows x 51.0" Wide + 6.0" Spacing x 12 + 12.0" Side Stone x 2 = 63.25' Base Width 6.0" Base + 30.0" Chamber Height + 6.0" Cover = 3.50' Field Height

208 Chambers x 45.9 cf = 9,555.5 cf Chamber Storage

26,019.7 cf Field - 9,555.5 cf Chambers = 16,464.2 cf Stone x 40.0% Voids = 6,585.7 cf Stone Storage

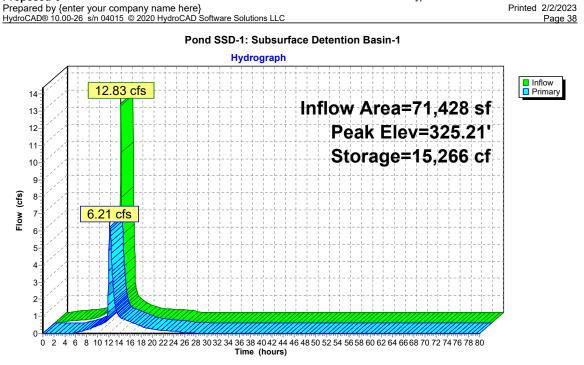
Chamber Storage + Stone Storage = 16,141.2 cf = 0.371 af Overall Storage Efficiency = 62.0%Overall System Size = 117.54' x 63.25' x 3.50'

208 Chambers 963.7 cy Field 609.8 cy Stone



Proposed-6

Type III 24-hr 100-Year Rainfall=8.16" Printed 2/2/2023





Operation and Maintenance Plan



OPERATION AND MAINTENANCE PLAN FOR 15 LIBERTY WAY FRANKLIN, MA

DATED: JANUARY 17, 2023

Prepared By: Level Design Group, L.L.C. 249 South Street, Unit 1 Plainville, MA 02762

Prepared For: Oliver Street Capital 125 High Street, Suite 220 Boston, MA 02110

LDG Project No.: 2081.00



The proposed Stormwater Management System is designed to function properly provided that routine maintenance is performed. It is the responsibility during construction and until purchase of the units and turnover of the project to a Home Owners Association to be formed, that the Owner and Developer, Oliver Street Capital, shall be responsible for the long-term maintenance and to provide the required maintenance outlined in this plan for the site infiltration systems as well as the remainder of the on-site storm drainage system.

Upon completion of construction and the formation of the Home Owners Association, maintenance of driveways and the stormwater appurtenances required to ensure that sedimentation and pollution is controlled and that storm water detention and infiltration capacity is sustained are the on-going responsibility of the Home Owners Association. To ensure the proper functioning of these facilities the following maintenance practices will be used:

DRIVEWAYS AND PARKING AREAS

Spring Maintenance

Driveways and Parking Areas are to be swept monthly to remove sand which has accumulated. Sand shall be removed from the site and legally disposed of.

Summer & Fall Maintenance

Leaves and debris which accumulates within the Driveways and Parking Areas during the summer and fall shall be collected and legally disposed of.

Winter Maintenance & Snow Removal

Snow removal within Driveways and Parking Area shall be stockpiled in the designated Snow Stockpile Areas outside of the traveled driveways. These areas should be located within or adjacent to the parking surface and should drain to the stormwater management system. Under no circumstances shall snow be directed onto abutting parcels or into the on-site resource areas (wetlands, wetland buffer zone, and riverfront areas).

Estimated Yearly Cost <u>\$1,000.00 (not including cost for snow plowing)</u>

DEEP SUMP CATCH BASINS

Catch basins shall be inspected and cleaned four times per year or when the sumps are 50% full.

Spring Maintenance

Catch basins require the removal of sediment each spring. This procedure is comprised of removing the catch basin grate followed by removal of sediment trapped in the structure with a clamshell shovel. The outlet pipe from the catch basin shall be inspected and any obstructions are to be removed. The sediment and debris removed from the catch basin shall be legally disposed of.

Fall Maintenance

Catch basin grates shall be cleared of leaves and debris so they may function properly.



Estimated Yearly Cost \$2,000.00

SUBSURFACE DETENTION BASIN

Spring Maintenance

The subsurface detention basin requires monthly inspections for accumulations of settled solids. If these materials have accumulated to a point where removal is necessary this shall be completed immediately. Accumulated trash and debris shall also be removed and legally disposed of during the monthly inspections.

Estimated Yearly Cost \$500.00

PUBLIC SAFETY FEATURES

Many of the Public Safety Features of the Stormwater Management System are incorporated into its design. The Infiltration basin was designed to minimize its depth to 2-feet deep. This combined with sediment forebay being approximately 1-foot deep provide for a safe and effective system.

Despite all the well-designed safety features within the Stormwater Management System all components of the system must be properly maintained to be effective. All maintenance procedures detailed above must be done on schedule and documented. Standing or stagnant water provides mosquito-breeding habitat and increases the potential for disease transmission. The basin is designed to fully infiltrate within 72 hours after a storm even which will prevent standing water from becoming a safety hazard. Routine monitoring for and management of mosquito-breeding conditions by qualified maintenance staff is required during the peak breading season between April and September ensure that unforeseen conditions do not develop.

While risks can be mitigated through proper design and maintenance, it is impossible to entirely eliminate risk. Therefore, education regarding stormwater management facilities and their inherent risks is valuable and should be a part of every community's activity. Employees and tenants of the Facility shall be given an overview of the Stormwater System and which areas to avoid. Public participation also increases the level of maintenance as community members can notify staff if a component of the stormwater system is not functioning properly.

The O&M shall be recorded with the Home Owners Agreement or other approving maintenance agreement to properly notify future owners of maintenance requirements.



STORMWATER MANAGEMENT OPREATOIN AND MAINTENANCE LOG

It is the responsibility of the owner and developer, Oliver Street Capital, to provide the maintenance of the Stormwater Management System Maintenance in accordance with the Town of Franklin Stormwater Management Standards until such time as an entity is created for overall site management at which time the agreement will spell out responsibility with appropriate contact information for all parties. The log form below is a template and shall be reproduced as needed. Copies of all log forms shall be kept on file for a minimum of three years from the date of inspection.

Name of Inspector:	
Date and Time of Inspection:	
Weather Conditions:	

Stormwater BMP	Observations	Action Required



Long Term Pollution Prevention Plan



LONG TERM POLLUTION PREVENTION PLAN FOR 15 LIBERTY WAY

FRANKLIN, MA

DATED: JANUARY 13, 2023

Prepared By: Level Design Group, L.L.C. 249 South Street, Unit 1 Plainville, MA 02762

Prepared For: Oliver Street Capital 125 High Street, Suite 220 Boston, MA 02110

LDG Project No.: 2081.00



GOOD HOUSEKEEPING PRACTICES

It is the responsibility of the developer, Oliver Street Capital, to provide for maintenance of the parking areas and the storm drainage system until the site is turned over to the condominium association which will be created prior to the sale of any units. The Owner shall utilize good housekeeping practices as outlined in the Operation and Maintenance Plan required for the maintenance of the Stormwater Management System.

PROVISIONS FOR STORAGE OF MATERIALS AND WASTE PRODUCTS INSIDE OR UNDER COVER

The storage of hazardous materials and waste is prohibited from being stored outdoor at the site. Any hazardous materials shall be stored under cover.

VEHICLE WASHING CONTROLS

Outdoor vehicle washing is allowed only for occupants of the condominium development for noncommercial vehicles owned by the residents of the units. No commercial vehicle washing operations is allowed in this area.

REQUIREMENTS FOR ROUTINE INSPECTION AND MAINTENANCE OF STORMWATER BMPS

The Owner / Operator shall keep a Maintenance Log Sheets of scheduled tasks outlined Operation and Maintenance Plan.

SPILL PREVENTION AND RESPONSE PLANS

The risk of significant spills requiring action at this site is limited and will most likely be associated with motor vehicle use or maintenance. In the event of a significant spill contact:

Massachusetts Department of Environmental Protection 24-hour emergency response notification line – (888) 304-1133

PROVISIONS FOR MAINTENANCE OF LAWNS, GARDENS, AND OTHER LANDSCAPED AREAS

The use of chemical fertilizers shall not be used on-site. If chemical fertilizers are required to be used, the fertilizers must be worked into the soil to prevent washouts and stormwater contamination of fertilizers.



REQUIREMENTS FOR STORAGE AND USE OF FERTILIZERS, HERBICIDES, AND PESTICIDES

If fertilizers, herbicides, and pesticides are to be used and stored on site they are to be stored in their original containers and keep in a dry, safe area where children do not have access to.

PROVISIONS SOLID WASTE MANAGEMENT

Solid waste and recycling is to be disposed in designated areas in enclosed dumpsters and receptacles with covers and hauled by private certified waste management service operators. Solid waste management systems shall be inspected and maintained in accordance with state, local, and federal solid waste management regulations.

EMERGENCY AND REGULATORY CONTACTS

Franklin Fire Department:	911 / (508) 528-2323
Franklin Police Department:	911 / (508) 528-1212
Massachusetts Department of Environmental Protection – Central Regional Office:	(508) 792-7650
United State Environmental Protection Agency:	(617) 918-1111

LOCAL FILING FEE CALCULATION WORKSHEET

1. NOTICE OF INTENT (NOI)

1.1.	New Individual Single Family Home (SFH) This includes all projects associated with a SFH	\$200.00	
1.2.	Work Associated with Existing Residential Pr	operty \$50.00	
	Above-ground pools, fences or other incidental pro- involving land disturbance that are not covered by	•	
1.3.	Control of Nuisance Vegetation This category shall not apply to any non-natural deposition of material e.g. vegetative debris	\$50.00	
1.4.	Subdivisions		
		\$600.00 r feet x \$2.00 0.00 each re feet x \$0.50	=
	(If single family homes are proposed as part of a s application, for each house in jurisdiction, individua		apply.)
1.5.	Multifamily Dwellings, including Condominium ^	9 Units : 4FDU x \$100.0	0
1.6.	Commercial/Industrial		
	Base Fee Infrastructure in Buffer Zone or Resource Area	\$600.00	<u>\$600.00</u>

Rev. 10/8/19	Local Filing Fee Calculation Worksheet	Page 1 of 2

	Roads *Drainage Structures Wetland Resource Area Di Buildings All Accessory Improvemen	sturbeds	linear feet x \$2.00 X \$10.00 each square feet x \$0.50 X \$125 each \$100.00	= = = =
2.	REQUEST FOR DETERMINATI	ON (RDA)		\$100.00
3.	MINOR BUFFER ZONE ACTIV	TY (MBZA)		\$50.00
4.	ABBREVIATED NOTICE OF RE (ANRAD)		REA DETERMINAT resource area:	ION =
5.	OTHER PERMITS/SERVICES			
	Order of Conditions Extension Certificate of Compliance Reques Certificate Re-Inspection Status Letter for Financial Institut Permit Amendment		\$50.00 \$50.00 \$50.00 \$100.00 \$100.00	
6.	FILING FEE CALCULATION			
	Town Share of State Fees (Se Fee Transmittal Form) Local Filing Fee Calculated Al TOTAL Due Town of Franklin State Share of Filing Fee (See Fee Transmittal Form) TOTAL Due DEP (Check No. 2	oove (Check No.: NOI Wetland	\$ <u>60</u> 1) \$	<u>)0</u>

7. ADVERTISING FEE (Check No. 3)

The fee will be the exact amount the newspaper charges for that specific advertisement. Once the advertisement is placed with the paper, by the Conservation Commission, the applicant will be notified of the cost and will be expected to submit a check for that exact amount, payable to the Town of Franklin, to the Conservation Department prior to the first hearing.

*Drainage structures: catch basins, manholes, leaching basins, gutter inlet or any other man-made structure (other than a pipe) for purposes of controlling drainage.

TBD

PROPERTY ACCESS SIGNATURE FORM

I hereby request that the Franklin Conservation Commission review this NOI/RDA/ANRAD application. I (we) grant authority to the Franklin Conservation Commission members and agents to go onto my (our) property solely for purposes directly related to the inspection and approval of this application and for follow-up compliance with the permit conditions.

Signature of Property Owner Date Date Date

APPLICATION PROCESS SIGNATURE FORM

There are three different applications that can be submitted to undertake work in a jurisdictional area: a Notice of Intent (NOI), a Request for Determination (RDA) and a Minor Buffer Zone Activity (MBZA). All three applications have different criteria for submission and approval and the NOI and RDA are governed by both the state law and the local bylaw. The MBZA is issued under the local bylaw only.

When a potential applicant requests advice from the Conservation Agent on which application to file, the opinion of the Agent is based on the information given by the potential applicant and any other information available to the Agent, e.g. the town's GIS system. The Agent has no legal right to go onto private property at any time until after an application is filed or permission of the property owner is given.

It is important that all applicants understand that after an application is filed, additional information may come to light e.g. via a field inspection or a review of the application, that may impact the scope of the submitted application and the approval process. Therefore, it is the ultimate responsibility of the applicant to decide which application to file.

In light of the above, please sign below indicating an understanding of this policy and submit it with the application.

Signature of Property Owner Scott Guddaw on behalt of applicant

RESOURCE AREA IMPACT SUMMARY FORM

The Franklin Wetlands Protection Bylaw Franklin Town Code Section 181

Resource Area	Alteration Proposed	Mitigation Proposed
Bordering Vegetated Wetland (SF)	0	0
Bank (LF)	0	0
Land Under Water Bodies (SF)	0	0
Isolated Wetland (SF)	264	1,000 (invasive species management)
Vernal Pool (SF)	0	0
25-foot Buffer Zone (SF)	5,000	10,000 (invasive species management)
Riverfront (SF)	0	0
100-Year Floodplain (CF)	0	0
(SF) = Square Feet (LF) = Linear Feet (CF) = Cubic Feet Flood Storage		



May 24, 2023

Municipal Building Franklin Conservation Commission 355 E. Central Street Franklin, MA 02038

Re: Request for Variance - 15 Liberty Way, Franklin MA (Map: 320, Parcel: 4)

Dear Franklin Conservation Commission,

On behalf of Atlantic Oliver 15 Liberty Way LLC (the applicant), Goddard Consulting, LLC (representative) is hereby submitting this request for variance for a project which consists of the expansion of a parking lot and associated stormwater management at 15 Liberty Way, Franklin MA. This report is a supplement to the Notice of Intent application submitted concurrently as required by the Franklin Wetlands Protection Bylaw. Site constraints, including zoning requirements for provided parking area and the lack of usable space on the lot, make it unfeasible to relocate or scale back the proposed parking and driveway expansion.

As noted in this Notice of Intent submittal, the proposed project requires alteration to the existing IVW onsite. This will be in the form of supplemental riprap added to the existing riprap present in the IVW. This alteration totals 264SF. In addition, encroachment of pavement into the 25-foot buffer zone is required to provide the necessary parking. This alteration of the 25-foot buffer zone totals approximately 5,000SF.

As mitigation for these alterations, the applicant proposes to manage invasive plant species in the IVW and surrounding area. The proposed invasive species management area is approximately 11,000SF. Invasive species present in this area include oriental bittersweet (*Celastrus orbiculatus*), multiflora rose (*Rosa multiflora*), purple loosestrife (*Lythrum salicaria*), and common reed (*Phragmites australis*). These species are known to outcompete native plant species that are important to native wildlife for food and habitat. Management of these species will allow native vegetation present in the area to thrive. It is Goddard's opinion that the proposed mitigation will result in a net improvement of habitat value in this area.



Figure 1: Photo showing proliferation of invasive plant species in proposed invasive species management area.



Below is a table summarizing potential alternatives to the proposed work.

Summary of Alternatives					
Alternative option	Impact to wetland resources and buffer zone	Mitigation	Cost		
Alternative 1: No change to existing conditions	No impacts to buffer zone or wetland resources.	None required. Project would not include invasive species management or improvement of stormwater management as proposed.	No cost, but applicant is unable to develop land as needed.		
Alternative 2: Scale back proposed parking and driveway area to avoid buffer zones	No impacts to buffer zone, but proposed alteration to IVW onsite would still be necessary to support stormwater management system for the area.	Project would provide significantly scaled-back mitigation.	Similar cost, but applicant is unable to develop land to extent needed.		
Alternative 3: Current proposal	Impacts to buffer zone and IVW onsite as described.	Project provides approx. 11,000SF of invasive species management, improving habitat value of the wetland resource and surrounding area.	Current design is most costly but meets the needs of the applicant.		

In summary, the proposed project provides an opportunity to improve the IVW onsite and its surrounding area over existing conditions. Based on the above analysis, Goddard believes that adverse impacts to wetland resources and buffer zones have been avoided to the greatest extent practicable, and respectfully requests that the Commission approve this request for variance.

Sincerely, Goddard Consulting, LLC

Chris Frattaroli Wetland Scientist

DEP Bordering Vegetated Wetland (310 CMR 10.55) Delineation Field Data Form Applicant: Atlantic Oliver 15 Liberty Prepared by: Goddard Consulting LLC Project location: 15 Liberty Way Franklin MA pply: Vegetation alone presumed adequate to delineate BVW boundary: fill out Section I only Project location: 15 Liberty Way Franklin MA v Vegetation and other indicators of hydrology used to delineate BVW boundary: fill out Section I only Method other than dominance test used (attach additional information) Vegetations I and II Project location: 15 Liberty Way Franklin MA

DEP File #:

Check all that apply:

Section I. Vegetation	Observation Plot Number:	Transect Num	nber: Upgradient	Date of Delineat	ion:
Sample Layer and Plant Species	Scientific name	% Cover	% Dominance	Dominant Plant (yes or no)	Wetland Indicator Category*
<u>Tree Laver</u>		200/	55 10/	X7	FACU
white pine	Pinus strobus Acer rubrum	38% 21%	55.1% 29.7%	Yes Yes	FACU FAC*
red maple northern white oak	Acer rubrum Quercus alba	21% 11%	29.7% 15.2%	Y es No	FAC≁ FACU
nottien white oak	Quercus arba	1170	13.270	110	FACU
Sapling Laver					
northern white oak	Quercus alba	3%	50.0%	Yes	FACU
white pine	Pinus strobus	3%	50.0%	Yes	FACU
<u>Shrub Layer</u>					
<u>Climbing Woody Vine</u>					
Ground Cover					
green brier	Smilax rotundifolia	3%	50.0%	Yes	FAC*
late lowbush blueberry	Vaccinium angustifolium	3%	50.0%	Yes	FACU
	common plant name indicates stunted growth; ** indicates of	extremely stunted growth			
Morphological Adaptations: 0	Description:				
	plants: plants listed in the Wetlands Protection Act (MGL c.131,	s.40); plants in the genus Sphagnum; o	or plants listed as FAC, FACW,	or OBL.	
Vegetation conclusion:					
Number of dominant wetland indicator	plants: 2	Number of dom	inant non-wetland indi	cator plants: 4	
	its equal to or greater than the number of domin			K	
â	a the RVW houndary submit this form with the Request for Dat				MA DED: 3/0

If vegetation alone is presumes adequate to delineate the BVW boundary, submit this form with the Request for Determination of Applicability or Notice of Intent.

Section II. Indicators of Hydrology	Other Indicators of Hydrology: (check all that apply and describe)
Hydric Soil Interpretation	Depth to free water in observation hole:
1. Soil Survey	Depth to soil saturation in observation hole:
Is there a published soil survey for this site? title/date: Soil Survey of Norfolk and Suffolk Counties - 1989 map number: MA616	Water marks:
soil type mapped: Woodbridge FSL 312B hydric soil inclusions: yes	Drift Lines:
Are field observations consistent with soil survey? ves no	Drainage patterns in BVW:
	Oxidized rhizoshperes:
2. Soil DescriptionHorizonDepth (inches)Matrix ColorMottles Color or TextureO0-410YR3/4FSLA4-810YR3/2FSLB8-2410YR4/1FSL	Water-stained leaves: Recorded data (stream, lake, or tidal gauge; aerial photo; other): Other:
25% concentrations 10YR6/8	Vegetation and Hudnels on Conclusion for Unovedient of
Remarks:	Vegetation and Hydrology Conclusion for Upgradient of ves no Number of wetland indicator plants >= number of non-wetland plants X
	Wetland hydrology present: hydric soils present X
3. Other:	other indicators of hydrology present X
Conclusion: Is soil hydric?	Sample location is in a BVW X Submit this form with the Request for Determination of Applicability or Notice of Intent
	l

DEP Bordering Vegetated Wetland (310 CMR 10.55) Delineation Field Data Form Applicant: Atlantic Oliver 15 Liberty Prepared by: Goddard Consulting LLC Project location: 15 Liberty Way Franklin MA upply: Vegetation alone presumed adequate to delineate BVW boundary: fill out Section I only Project location: 15 Liberty Way Franklin MA v Vegetation and other indicators of hydrology used to delineate BVW boundary: fill out Sections I and II Method other than dominance test used (attach additional information)

Project location: 15 Liberty Way Franklin MA

Check all that apply:

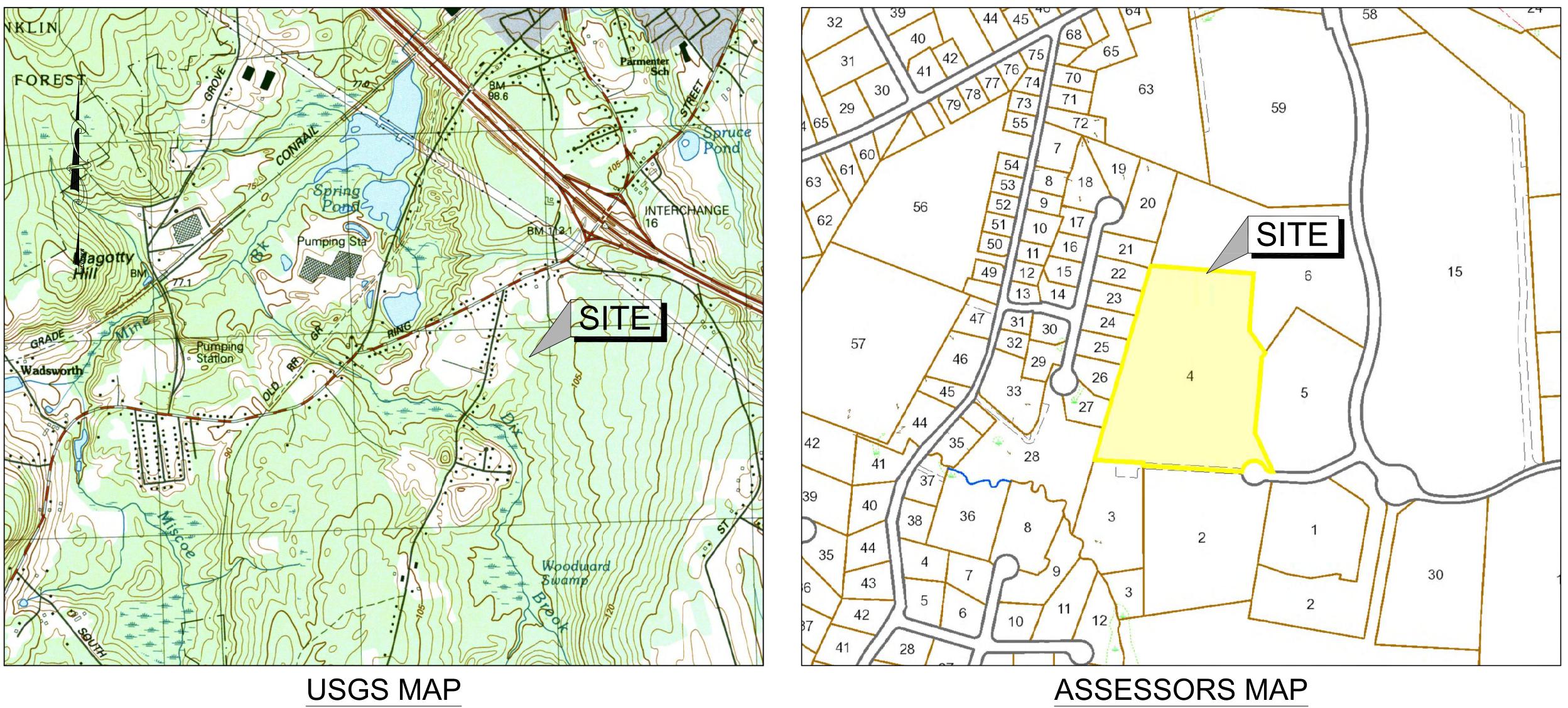
Section I. Vegetation	Observation Plot Number:	Transect Num	ber: Downgradient	Date of Delineat	ion:
Sample Layer and Plant Species	Scientific name	% Cover	% Dominance	Dominant Plant (yes or no)	Wetland Indicator Category*
Tree Layer					
red maple	Acer rubrum	21%	87.2%	yes	FAC*
eastern cottonwood	Populus deltoides	3%	12.8%	no	FAC*
<u>Sapling Layer</u> sitka willow	Salix sitchensis	21%	87.2%	Vec	NI
pin oak		3%	12.8%	yes no	FACW*
рш бак	Quercus palustris	570	12.870	що	FAC W
Shrub Layer					
sweet pepperbush	Clethra alnifolia	11%	43.8%	yes	FAC*
rambler rose white meadowsweet	Rosa multiflora	11% 3%	43.8% 12.5%	yes	FACU FACW*
	Spiraea alba	576	12.576	no	
<u>Climbing Woody Vine</u>					
Ground Cover					
sensitive fem	Onoclea sensibilis	21%	38.7%	Yes	FACW*
sedge	Carex sp.	3%	5.7%	no	FACW*
common reed	Phragmites australis	21%	38.7%	yes	FACW*
jewelweed	Impatiens capensis	3%	5.7%	no	FACW*
purple loosestrife	Lythrum salicaria	3%	5.7%	no	OBL*
eastern poison ivy	Toxicodendron radicans	3%	5.7%	no	FAC*
Remarks: * An asterisk after	common plant name indicates stunted growth; ** indicates e	xtremely stunted growth			
Morphological Adaptations: 0	Description:				
* An asterisk after indicator status denotes wetlands p	plants: plants listed in the Wetlands Protection Act (MGL c.131,	s.40); plants in the genus Sphagnum; o	or plants listed as FAC, FACW,	or OBL.	
Vegetation conclusion:					
Number of dominant wetland indicator	plants: 4	Number of domi	inant non-wetland indi	cator plants: 2	
	ts equal to or greater than the number of domin			•	
A		Î			MA DED: 2/0/

If vegetation alone is presumes adequate to delineate the BVW boundary, submit this form with the Request for Determination of Applicability or Notice of Intent.

DEP File #:

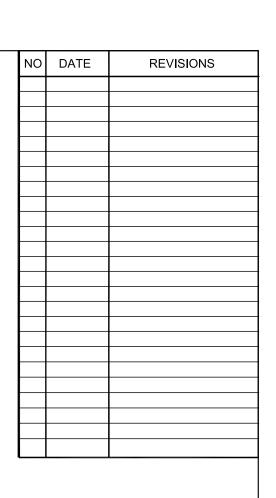
Hydric Soil Interpretation	ndated:
	o free water in observation hole: surface
	soil saturation in observation hole: <u>surface</u>
Is there a published soil survey for this site? title/date: Soil Survey of Norfolk and Suffolk Counties - 1989 map number: MA616	arks:
soil type mapped: Woodbridge FSL 312B Drift Lin hydric soil inclusions: Yes	nes:
-	t deposits:
Remarks:	e patterns in BVW:
	d rhizoshperes:
2. Soil Description	ained leaves: d data (stream, lake, or tidal gauge; aerial photo; other):
Other:	
Vegetation and Hydrolog	gy Conclusion for Downgradient of
Number of wetland indica	
Remarks: >= number of non-wetland	nd plants X
Wetland hydrology prese hydric so	ent: oils present X
3. Other: other ind present	dicators of hydrology X
Conclusion: Is soil hydric? yes I ho Submit this form with the Request for	WW X r Determination of Applicability or Notice of Intent

LIBERTY PARKING EXPANSION OLIVER STREET CAPITAL **15 LIBERTY WAY** FRANKLIN, MASSACHUSETTS DATE: JANUARY 13, 2023



SCALE: 1"= 1,000'±

SCALE: $1" = 300' \pm$



APPLICANT:

OLIVER STREET CAPITAL 125 HIGH STREET, SUITE 220 BOSTON, MA 02110

CIVIL ENGINEER:



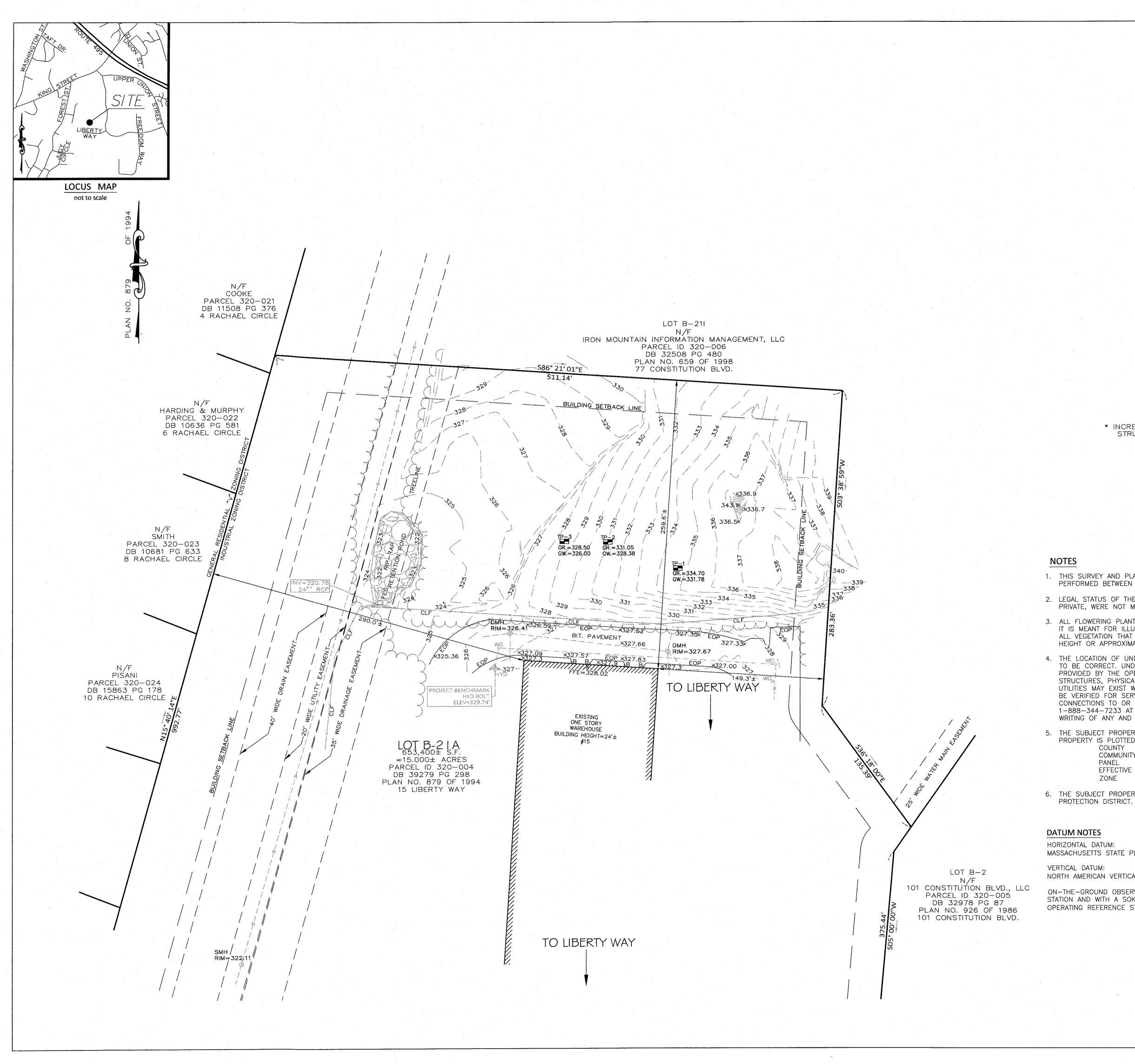
PROPERTY ADDRESS: **15 LIBERTY WAY** FRANKLIN, MASSACHUSETTS

ASSESSORS MAP/PARCEL: MAP 320 , PARCEL 004

ZONING DISTRICT: INDUSTRIAL

DRAWING LIST:

C-0.0	COVER
C-1.0	EXISTING CONDITIONS
C-2.0	LAYOUT & MATERIALS
C-3.0	GRADING & DRAINAGE
C-4.0	EROSION CONROL PLAN
C-5.0	TYPICAL DETAILS
C-5.1	TYPICAL DETAILS
C-5.2	TYPICAL DETAILS



LEGEND

BIT.

CLF

-D-

DB

DMH

ELEV EOP

FES

FFE

HYD

INV N/F

RBF

-S-

SMH

PG

RCP

-W-

WG

BOLLARD BITUMINOUS CHAIN LINK FENCE UNDERGROUND DRAIN LINE DEED BOOK DRAIN MANHOLE ELEVATION EDGE OF PAVEMENT FLARED END SECTION FINISHED FLOOR ELEVATION FIRE HYDRANT INVERT NOW OR FORMERLY REBAR FOUND UNDERGROUND SEWER LINE SEWER MANHOLE PAGE REINFORCED CONCRETE PIPE UNDERGROUND WATER LINE

WATER GATE

ZONING DISTRICT

INDUSTRIAL

MINIMUM BUILDING SETBACKS

FRONT=40 FEET SIDE=30 FEET* REAR=30 FEET* * INCREASE BY THE COMMON BUILDING HEIGHT OF THE STRUCTURE WHEN ABUTTING A RESIDENTIAL USE.

DEED REFERENCES

ATLANTIC OLIVER 15 LIBERTY WAY, LLC DEED BOOK 39279 PAGE 278

PLAN REFERENCES

PLAN NO. 879 OF 1994 PLAN NO. 878 OF 1994 PLAN NO. 47 OF 1995

1. THIS SURVEY AND PLAN ARE BASED UPON AN ACTUAL ON THE GROUND INSTRUMENT SURVEY PERFORMED BETWEEN NOVEMBER 2, 2022 AND NOVEMBER 16, 2022.

2. LEGAL STATUS OF THE STREETS AND/OR WAYS SHOWN HEREON, WHETHER THEY ARE PUBLIC OR PRIVATE, WERE NOT MADE PART OF THIS SURVEY.

3. ALL FLOWERING PLANTS, SHRUBS, OR TREES MAY NOT BE SHOWN WITHIN LANDSCAPING AREAS. IT IS MEANT FOR ILLUSTRATIVE PURPOSES ONLY AND NOT MEANT AS A COMPLETE INVENTORY OF ALL VEGETATION THAT MAY EXIST. SIZES REFLECT APPROXIMATE TRUNK DIAMETER AT BREAST HEIGHT OR APPROXIMATE DIAMETER OF SHRUB WIDTH.

4. THE LOCATION OF UNDERGROUND UTILITIES ARE APPROXIMATE ONLY, AND ARE NOT WARRANTED TO BE CORRECT. UNDERGROUND UTILITIES ARE SHOWN BASED ON EITHER RECORD DATA PROVIDED BY THE OPERATING AUTHORITIES, VISUAL INSPECTION OF AVAILABLE ABOVEGROUND STRUCTURES, PHYSICAL SURFACE MARKINGS FOUND, OR DATA PROVIDED BY OTHERS. ADDITIONAL UTILITIES MAY EXIST WHICH ARE NOT INDICATED ON THESE PLANS. ALL EXISTING UTILITIES SHALL BE VERIFIED FOR SERVICE, SIZE, INVERT ELEVATION, LOCATIONS, ETC. PRIOR TO NEW CONNECTIONS TO OR RELOCATION OF SAME. CONTRACTOR MUST NOTIFY DIG-SAFE AT 1-888-344-7233 AT LEAST 72 HOURS PRIOR TO ANY CONSTRUCTION. NOTIFY THIS FIRM IN WRITING OF ANY AND ALL DISCREPANCIES PRIOR TO COMMENCING ANY WORK.

BJECT	PROPERTY DOE	S NOT FALL IN	A SPECIAL	FLOOD	HAZARD Z	ONE AS	THE	SUBJECT
YIS	PLOTTED BY SC	ALE ONTO THE	FLOOD INS	URANCE	RATE MAP	FOUND	AS,	
CC	UNTY	NORFOL	_K					
CO	MMUNITY	TOWN	OF FRANKLI	N				
PA	NEL	250240	0317E					
EF	FECTIVE DATE	JULY 1	7, 2012					
ZC	NE	"X"						1

6. THE SUBJECT PROPERTY DOES NOT FALL IN THE TOWN OF FRANKLIN'S WATER RESOURCE

MASSACHUSETTS STATE PLANE - MAINLAND ZONE NAD83 (2011) EPOCH 2010.00 - US FEET

NORTH AMERICAN VERTICAL DATUM OF 1988 (GEOID 12B) - US FEET

ON-THE-GROUND OBSERVATIONS WERE PERFORMED USING A LEICA TS13 (3") ROBOTIC TOTAL STATION AND WITH A SOKKIA GCX3 GPS RECEIVER USING THE MASSACHUSETTS CONTINUOUSLY OPERATING REFERENCE STATION (MaCORS) RTK NETWORK.

SEAL DATE : JANUARY 13, 2023 DRAWN: AH/DRC SCALE : 1" = 40' 004 ARCEL -320, Шζ ANKLIN,

ACHUSETTS

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MAS

MA 151 FR/

DESIGN GROUP

Civil Engineers & Land Surveyors

249 SOUTH STREET, UNIT 1 PLAINVILLE, MA 02762

TEL. (508) 695-2221 FAX. (508) 695-2219

EXISTING

CONDITIONS

SHEET 2 OF 8

80

20' 40'

NO DATE

REVISIONS

ZONING DISTRICT

ZONED: INDUSTRIAL

FRONT=40 FEET SIDE=30 FEET REAR=30 FEET

DEED REFERENCES ATLANTIC OLIVER 15 LIBERTY WAY, LLC

DEED BOOK 39279 PAGE 278

PLAN REFERENCES

PLAN NO. 879 OF 1994 PLAN NO. 878 OF 1994 PLAN NO. 47 OF 1995

ZONING REQUIREMENTS	· · ·	
ZONING DISTRICT: INDUSTRIAL		
	REQUIRED	PROVIDED
MIN. LOT AREA	40,000 S.F.	653,400± S.F.
FRONTAGE	175'	946.73'
FRONT YARD SETBACK	40'	95.9'***
SIDE YARD SETBACK	30'	149.3'
REAR YARD SETBACK	30'	259.6'
BUILDING COVERAGE	70% MAX.	14.6%**
IMPERVIOUS COVERAGE	80% MAX.	57.8%***
MAX. BUILDING HEIGHT	3 STORIES*	1 STORY/24'
PARKING REQUIREMENTS	L₂	
<u>USE: WAREHOUSE</u> : ONE SPACE PER 1,000 SF OF GROSS FLOOR AREA.	95,475 SF /1,000 SF =96 SPACES	EXISTING

PROPOSED_ TREELINE

EXISTING

BASÍN

EXISTING

FENCE (TYP)

EXISTING

TREELINE J

(TYP)

DETENTION

 * BUILDINGS UP TO 60' IN HEIGHT MAY BE PERMITTED BY A SPECIAL PERMIT FROM THE PLANNING BOARD.
 ** BUILDING AREA IS BASED ON FRANKLIN ASSESSORS RECORDS

*** TAKEN FROM THE RECORD SURVEY IN ADDITION TO AERIAL IMAGERY.

PLAN NOTES:

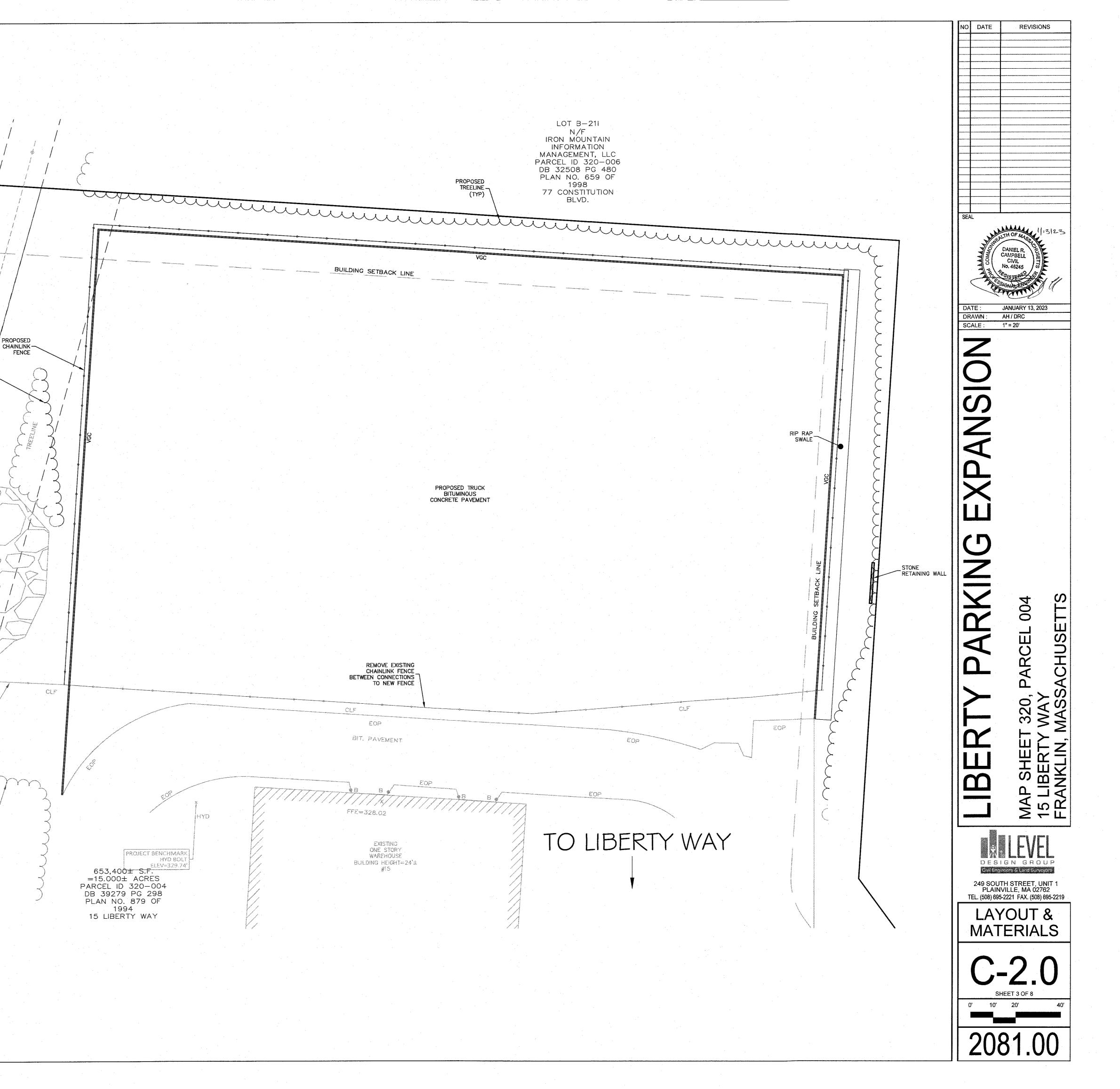
- EXISTING CONDITIONS WERE OBTAINED FROM FIELD SURVEY PREPARED BY LEVEL DESIGN GROUP, LLC BETWEEN NOVEMBER 2, 2022 AND NOVEMBER 16, 2022. SOIL TESTING COMPLETED BY ADAM P. HUNT ON NOVEMBER 16, 2022.
- 2. THE LOCATION OF EXISTING UTILITIES IS APPROXIMATE, THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL UTILITIES IN THE FIELD PRIOR TO THE START OF CONSTRUCTION. NOTIFY "DIG-SAFE" AT 1-888-344-7233 AT LEAST 72 HOURS PRIOR TO ANY SITE DEMOLITION OR EXCAVATION.
- 3. CONTRACTOR SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES IN THE DESIGN PLANS PRIOR TO THE START OF CONSTRUCTION.
- 4. ALL DISTURBED AREAS NOT RECEIVING IMPROVEMENTS SHALL BE LOAMED AND SEEDED.
- 5. THE SITE IS NOT LOCATED WITHIN A ZONE II WATER RESOURCE DISTRICT.
- 6. THE SITE IS NOT LOCATED WITHIN A FLOOD PLAIN DISTRICT.

CONSTRUCTION NOTES:

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- 2. CONTRACTOR SHALL NOTIFY ENGINEER OF ANY DISCREPANCIES IN THE DESIGN PLANS PRIOR TO THE START OF CONSTRUCTION.
- 3. ALL EXISTING PAVEMENT SHALL BE SAWCUT PRIOR TO REMOVAL.
- 4. ALL EXISTING PAVEMENT, CURB, WALKS, UTILITIES, LIGHT POLES, TREES, SHRUBS, ETC., SHALL BE REMOVED FROM THE AREAS TO BE DEVELOPED. ALL SUCH ITEMS NOT WITHIN THE WORK AREA SHALL BE PROTECTED AND UNDISTURBED.
- 5. ALL DISTURBED AREAS NOT RECEIVING IMPROVEMENTS SHALL BE LOAMED AND SEEDED.
- 6. ALL CONSTRUCTION AND CONSTRUCTION ACTIVITIES SHALL CONFORM TO STATE AND LOCAL REQUIREMENTS. INCLUDING BUT NOT LIMITED TO THE TOWN OF FRANKLIN, THE COMMONWEALTH OF MASSACHUSETTS AND ANY OTHER AGENCIES HAVING JURISDICTION.
- 7. MATERIAL TO BE EXPORTED FROM THE SITE WILL BE LIMITED TO TOPSOIL IN AREAS TO BE PAVED AND ANY DELETERIOUS MATERIAL ENCOUNTERED DURING EXCAVATION. MATERIAL TO BE EXPORTED WILL BE LIMITED TO GRAVEL BASE AND SEPTIC AGGREGATE.

SITE PLAN NOTES:

1. THIS PLAN SET HAS BEEN ISSUED FOR PERMITTING ONLY. A FULL CONSTRUCTION PLAN SET SHALL BE ISSUED ONCE ALL LOCAL, STATE, AND FEDERAL PERMIT APPROVALS HAVE BEEN GRANTED.



ZONING DISTRICT ZONED: INDUSTRIAL

MINIMUM BUILDING SETBACKS

FRONT=40 FEET SIDE=30 FEET REAR=30 FEET

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EXISTING

DETENTION-

BASIN

OUTLET-2 🚽

EXISTING

FENCE T

(TYP)

EXISTING

TREELINE -

(TYP)

CHAINLINK

RIP-RAP APRON.

INV(END)=322.00

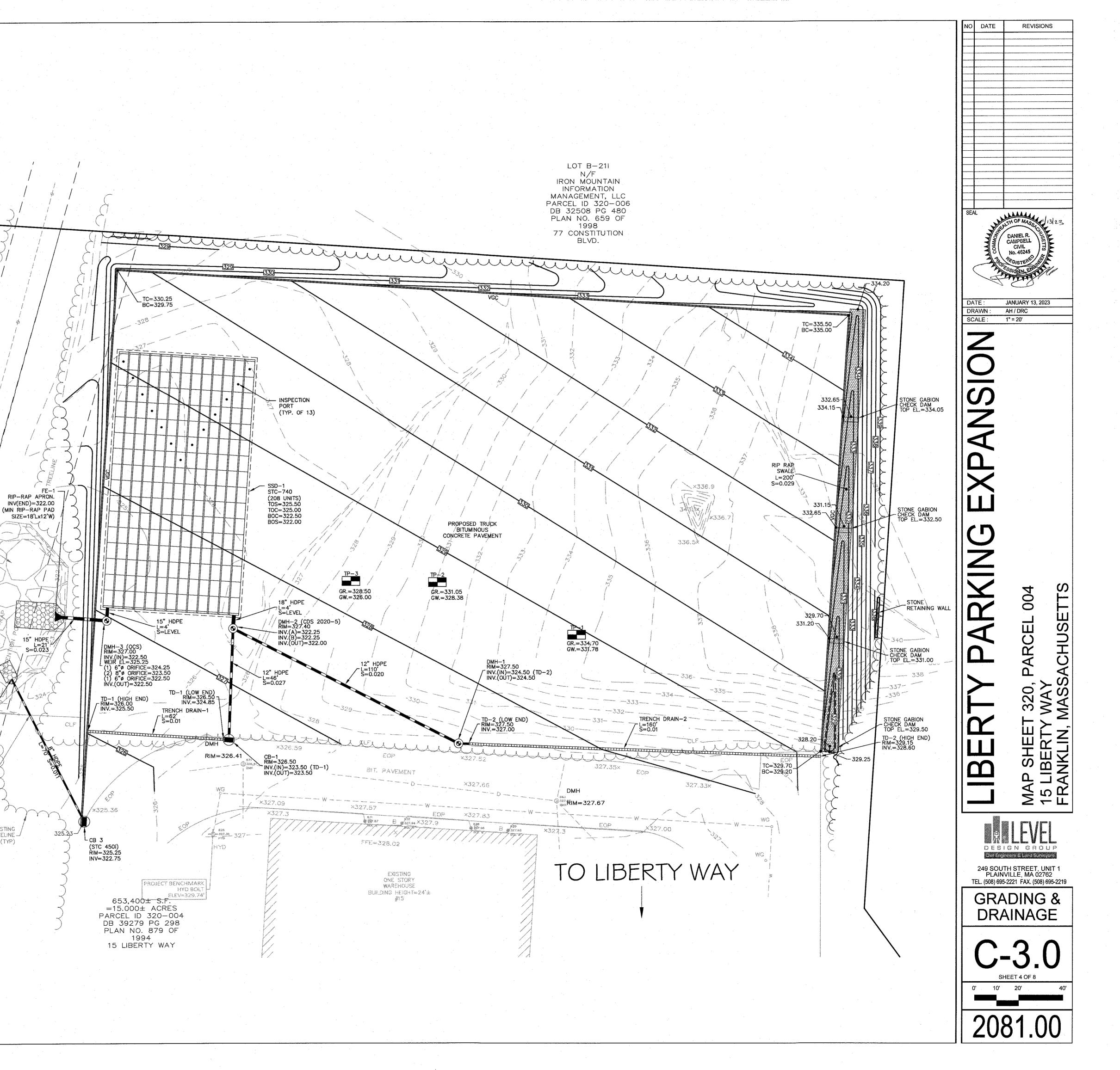
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EROSION CONTROL PLAN NOTES:

SILTATION CONTROL USING EROSION CONTROL FENCE WITH STRAW WATTLE. OR APPROVED EQUAL

SILTSOXX IS TO BE VISUALLY INSPECTED AFTER EVERY RAIN FALL AND REPAIRS MADE AS REQUIRED TO THE SILTATION CONTROL FENCE AND STRAW WATTLE AFTER EACH RAIN FALL. CLEANOUT OF ACCUMULATED SEDIMENT BEHIND THE WATTLE IS NECESSARY IF 1/2 OF THE ORIGINAL HEIGHT OF THE WATTLE APPEARS TO HAVE BEEN INUNDATED WITH SEDIMENT.

PRESERVE TOPSOIL

SITE OWNERS AND OPERATORS MUST PRESERVE EXISTING TOPSOIL ON THE CONSTRUCTION SITE TO THE MAXIMUM EXTENT FEASIBLE AND AS NECESSARY TO SUPPORT HEALTHY VEGETATION, PROMOTE SOIL STABILIZATION, AND INCREASE STORMWATER INFILTRATION RATES IN THE POST-CONSTRUCTION PHASE OF THE PROJECT.

STABILIZATION OF SOILS

UPON COMPLETION AND ACCEPTANCE OF SITE PREPARATION AND INITIAL INSTALLATION OF EROSION, RUNOFF, AND SEDIMENT CONTROLS AND TEMPORARY POLLUTION PREVENTION MEASURES, THE OPERATOR SHALL INITIATE APPROPRIATE TEMPORARY OR PERMANENT STABILIZATION PRACTICES DURING ALL PHASES OF CONSTRUCTION ON ALL DISTURBED AREAS AS SOON AS POSSIBLE BUT NOT MORE THAN FOURTEEN (14) DAYS AFTER THE CONSTRUCTION ACTIVITY IN THAT AREA HAS TEMPORARILY OR PERMANENTLY CEASED UNLESS THE ACTIVITY IS TO RESUME WITHIN TWENTY-ONE (21) DAYS.

ALL TEMPORARY EROSION AND SEDIMENT CONTROL MEASURES SHALL BE REMOVED AFTER FINAL SITE STABILIZATION. DISTURBED SOIL AREAS RESULTING FROM THE REMOVAL OF TEMPORARY MEASURES SHALL BE PERMANENTLY STABILIZED WITHIN 30 DAYS OF REMOVAL.

ONLY AREAS THAT CAN BE REASONABLY EXPECTED TO HAVE ACTIVE CONSTRUCTION WORK BEING PERFORMED WITHIN 14 DAYS OF DISTURBANCE WILL BE CLEARED/GRUBBED AT ANY ONE TIME. IT IS NOT ACCEPTABLE TO GRUB AND STRIP TOP SOIL THE ENTIRE CONSTRUCTION SITE IF PORTIONS WILL NOT BE ACTIVE WITHIN THE 14-DAY TIME FRAME, PROPER PHASING OF CLEARING AND GRUBBING ACTIVITIES SHALL INCLUDE TEMPORARY STABILIZATION TECHNIQUES FOR AREAS CLEARED AND GRUBBED THAT WILL NOT BE ACTIVE WITHIN THE 14-DAY TIME FRAME.

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STEEP SLOPES SHALL NOT BE LEFT UNATTENDED OR EXPOSED FOR EXCESSIVE PERIODS OF TIME SUCH AS THE INACTIVE WINTER SEASON. THE CONTRACTOR SHALL INITIATE APPROPRIATE VEGETATIVE PRACTICES ON ALL DISTURBED AREAS IN AREAS OF STEEP SLOPES AS SOON AS POSSIBLE BUT NOT MORE THAN FOURTEEN (14) DAYS AFTER THE CONSTRUCTION ACTIVITY IN THAT AREA HAS TEMPORARILY OR PERMANENTLY CEASED, UNLESS THE ACTIVITY IS TO RESUME WITHIN TWENTY-ONE (21) DAYS. ONCE AN STEEP SLOPE AREA HAS BEEN TEMPORARY AND/OR FINAL SEEDED IT SHALL BE PROTECTED WITH 4' HIGH ORANGE CONSTRUCTION TO PREVENT FURTHER DISTURBANCE OF THE AREA.

THE TEMPORARY SEEDING DESIGN MIX SHALL BE COMPRISED OF THE FOLLOWING:

ANNUAL

PERENNI

TYPE	% BY WEIGHT
AL RYE GRASS	40
INIAL RYE GRASS	60

STORMWATER INLET PROTECTION

INLET PROTECTION - WILL BE UTILIZED TO PREVENT SOIL AND DEBRIS FROM ENTERING STORM DRAIN INLETS. THESE MEASURES ARE USUALLY TEMPORARY AND ARE IMPLEMENTED BEFORE A SITE IS DISTURBED. ALL STORMWATER INLETS &/OR CATCH BASINS THAT ARE OPERATIONAL DURING CONSTRUCTION AND HAVE THE POTENTIAL TO RECEIVE SEDIMENT-LADEN STORMWATER FLOW FROM THE CONSTRUCTION SITE MUST BE PROTECTED USING CONTROL MEASURES OUTLINED IN THE STORMWATER POLLUTION PREVENTION PLAN. EROSION CONTROL FENCE WITH STRAW WATTLES ARE TO BE PLACED AT TO OPENING OF ALL ROUGH GRADED SEDIMENT FOREBAYS UNTIL THE FOREBAY AND BASIN HAS BEEN STABILIZED AND BROUGHT ON-LINE.

MAINTENANCE - THE OPERATOR MUST CLEAN, OR REMOVE AND REPLACE THE INLET PROTECTION MEASURES AS SEDIMENT ACCUMULATES, THE FILTER BECOMES CLOGGED, AND/OR AS PERFORMANCE IS COMPROMISED. ACCUMULATED SEDIMENT ADJACENT TO THE INLET PROTECTION MEASURES SHOULD BE REMOVED BY THE END OF THE SAME WORK DAY IN WHICH IT IS FOUND OR BY THE END OF THE FOLLOWING WORK DAY IF REMOVAL BY THE SAME WORK DAY IS NOT FEASIBLE.

INFILTRATION BASINS - ALL AREAS CONTAINING INFILTRATION BASINS (ABOVE OR BELOW GROUND) SHALL BE PROTECTED THROUGHOUT CONSTRUCTION. THESE AREAS ARE NOT BE USED FOR MATERIAL STOCKPILES OR FOR PARKING EQUIPMENT. SURFACE BASINS ARE TO BE ROUGH GRADED AND PROTECTED UNTIL STABILIZED AND BROUGHT ON-LINE FOR STORMWATER MANAGEMENT OF THE STABILIZED SITE.

CONSTRUCTION ENTRANCES

CONSTRUCTION ENTRANCES SHALL BE ESTABLISHED PRIOR TO REMOVAL OF EXISTING DRIVEWAY PAVEMENT TO BE USED IN CONJUNCTION WITH THE STABILIZATION OF CONSTRUCTION ROADS TO REDUCE THE AMOUNT OF SEDIMENT TRACKING OFF THE PROJECT. ANY CONSTRUCTION SITE ACCESS POINT MUST EMPLOY THE CONTROL MEASURES ON THE APPROVED SITE PLANS AND IN ACCORDANCE WITH THE STORMWATER POLLUTION PREVENTION PLAN. CONSTRUCTION ENTRANCES SHALL BE USED IN CONJUNCTION WITH THE STABILIZATION OF CONSTRUCTION ROADS TO REDUCE THE AMOUNT OF MUD PICKED UP BY CONSTRUCTION VEHICLES. ALL CONSTRUCTION ACCESS ROADS SHALL BE CONSTRUCTED PRIOR TO ANY ROADWAY ACCEPTING CONSTRUCTION TRAFFIC.

THE SITE OWNER AND OPERATOR MUST WILL RESTRICT VEHICLE USE TO PROPERLY DESIGNATED EXIT POINTS, USE PROPERLY DESIGNED AND CONSTRUCTED CONSTRUCTION ENTRANCES AT ALL POINTS THAT EXIT ONTO PAVED ROADS SO THAT SEDIMENT REMOVAL OCCURS PRIOR TO VEHICLE EXIT. WHEN AND WHERE NECESSARY, USE ADDITIONAL CONTROLS TO REMOVE SEDIMENT FROM VEHICLE TIRES PRIOR TO EXIT (I.E. WHEEL WASHING RACKS, RUMBLE STRIPS, AND RATTLE PLATES). WHERE SEDIMENT HAS BEEN TRACKED OUT FROM THE CONSTRUCTION SITE ONTO THE SURFACE OF OFFSET STREETS, OTHER PAVED AREAS, AND SIDEWALKS, THE DEPOSITED SEDIMENT MUST BE REMOVED BY THE END OF THE SAME WORK DAY IN WHICH THE TRACK OUT OCCURS. TRACK-OUT MUST BE REMOVED BY SWEEPING, SHOVELING, OR VACUUMING THESE SURFACES, OR BY USING OTHER SIMILARLY EFFECTIVE MEANS OF SEDIMENT REMOVAL.

STOCKPILE CONTAINMENT

PROPOSED 12"DIA

SILT SOXX (TYP)

FF-1

RIP-RAP APRON.

INV(END) = 322.00

SIZE=18'Lx12'W)

15" HDP

(MIN RIP-RAP PAD

EXISTING

DETENTION

BASÍN

OUTLET-2 -

EXISTING

FENCE⁻

(TYP)

EXISTING

TREELINE J

(TYP)

325.23-

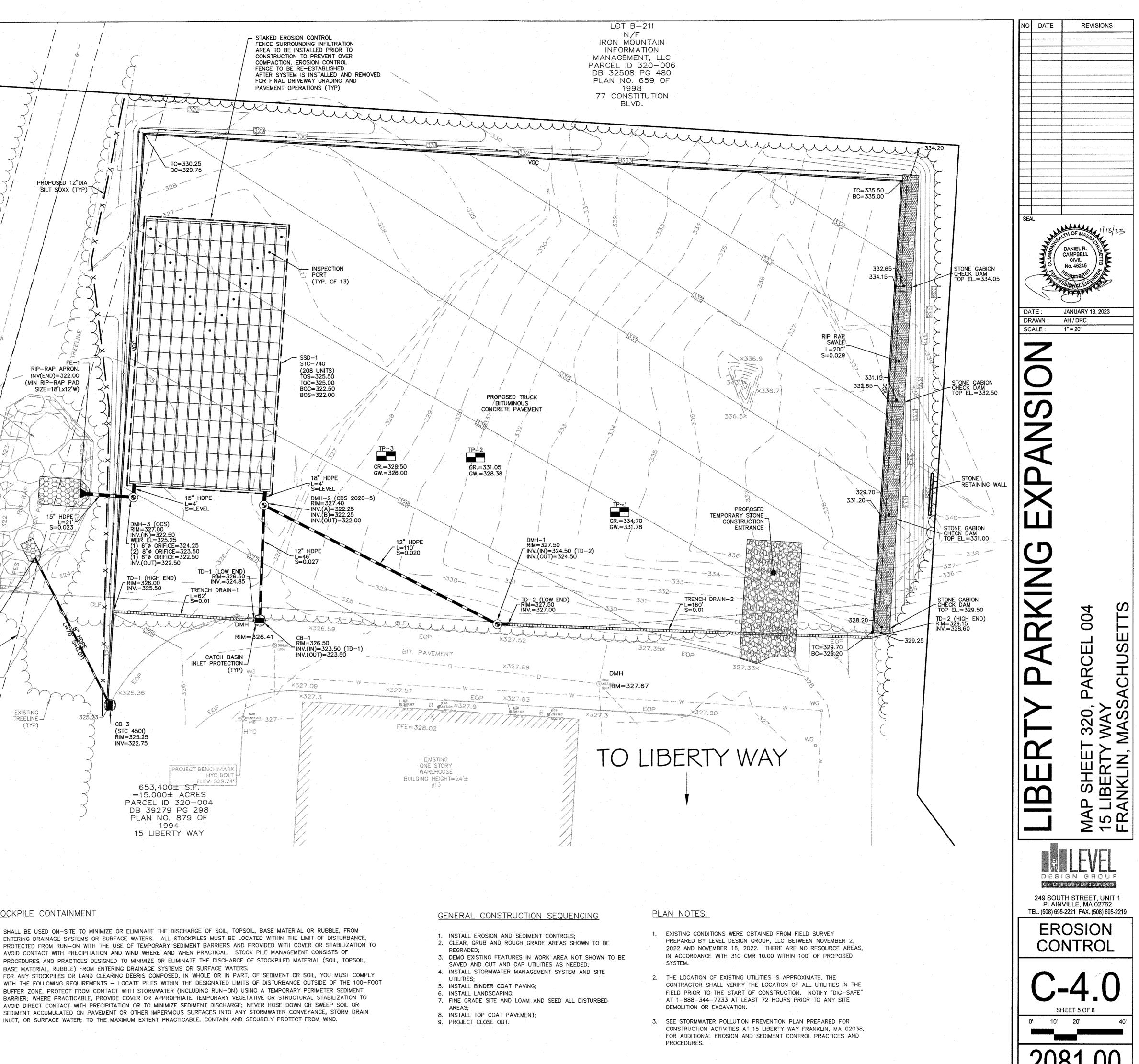
CHAINLINK

RIP-RAP APRON.

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FOR ANY STOCKPILES OR LAND CLEARING DEBRIS COMPOSED, IN WHOLE OR IN PART, OF SEDIMENT OR SOIL, YOU MUST COMPLY BUFFER ZONE, PROTECT FROM CONTACT WITH STORMWATER (INCLUDING RUN-ON) USING A TEMPORARY PERIMETER SEDIMENT BARRIER: WHERE PRACTICABLE, PROVIDE COVER OR APPROPRIATE TEMPORARY VEGETATIVE OR STRUCTURAL STABILIZATION TO AVOID DIRECT CONTACT WITH PRECIPITATION OR TO MINIMIZE SEDIMENT DISCHARGE; NEVER HOSE DOWN OR SWEEP SOIL OR SEDIMENT ACCUMULATED ON PAVEMENT OR OTHER IMPERVIOUS SURFACES INTO ANY STORMWATER CONVEYANCE, STORM DRAIN

