STORM WATER
DRAINAGE ANALYSIS
For
Upper Union Solar Project
0 Upper Union Street
Franklin, Massachusetts

Prepared for:
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ADE Project No. 3328.00

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### 1.0 INTRODUCTION

The purpose of this drainage study is to analyze the stormwater drainage conditions that will occur as a result of the construction of the proposed Upper Union Solar Project along with associated access drives and infrastructure at 0 Upper Union Street, (Parcel 009 on the Town of Franklin Assessors Map 319). The project site is a $6.2 \pm$-acre parcel on the east side of Upper Union Street, just north of Ribero Drive. The property is comprised of partially cleared areas and undeveloped woodland with a 325 ' wide New England Power Easement running through the center of the site.

The site does not lie within a DEP designated Zone II or a Town of Franklin designated Water Resource District per Town of Franklin Water Resource District maps. The site is not located within FEMA Flood Zone based upon a review of FEMA Flood Mapping. The property is not located within an Estimated Habitat of Rare Wildlife or Priority Habitat of Rare Species, as mapped by the Natural Heritage and Endangered Species Program (NHESP). The site is not located within an Area of Critical Environmental Concern (ACEC).

Based upon a review of the Web Soil Survey, soils within the proposed development area have been identified as either Charlton-Hollis-Rock outcrop complex or Woodbridge fine sandy loam, both with a "complex" of hydrological soil groups. The term "complex" consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps, ranging from soils groups A through D.

HydroCAD Stormwater modeling software was used to analyze the hydrological impacts of the development of the Site, calculate pre- and post-development runoff, design the proposed stormwater management system, and to confirm the adequacy of the system to accommodate the flows from the development. The Stormwater analysis and project design was completed in accordance with the requirements of the Massachusetts Department of Environmental Protection (DEP) Stormwater Management Standards, and the Town of Franklin Stormwater Management Bylaw.

### 2.0 PROPOSED STORMWATER MANAGEMENT SYSTEM

The stormwater system for the project has been evaluated and designed based upon DEP Wetland Program Policy 17.1: Photovoltaic System Solar Array Review, the Town's Stormwater Management bylaw, as well as the DEP Stormwater Policy of encouraging environmentally sensitive design with minimal point source discharges. Grading proposed on the site will, for the most part, follow the existing contours to mimic existing runoff patterns, except for areas within the array that presently exceed $20 \%$.

Stormwater runoff from portions of the proposed development area will be directed with ditches/swales, and detention basins, which will prevent direct discharge of untreated stormwater to any wetland resource areas or offsite.

The proposed stormwater management system has also been designed to attenuate any increase in peak flows resulting from development of the site.

Erosion control measures (sediment logs or approved equal) will be in place and maintained at the proposed limit of work throughout construction, until vegetation has stabilized, to protect the wetlands and adjacent properties.

### 3.0 COMPLIANCE WITH DEP STORMWATER MANAGEMENT STANDARDS

## Standard 1: No New Untreated Discharges

Except for a 50 foot long paved apron at the site entrance, there are no new impervious surfaces proposed as part of the solar project that will generate suspended solids or other measurable stormwater contaminants. The only measurable impervious surface on the site is the concrete electrical equipment pads and those will be limited to foot traffic only. All access roads will be gravel which, DEP Program Policy 17.1 does not consider impervious and the area under the arrays will be comprised of tall grasses. Runoff from the small, paved apron at the site entrance is treated by a deep sump catch basin and a subsurface infiltration system. Therefore, there will be no untreated discharge and it is our opinion that Standard 1 has been met.

## Standard 2: Peak Rate Attenuation

Pre- and Post-Development stormwater calculations were performed for the 2, 10-, 25-, and 100year, Type III storm events. A comparison of the Pre- vs. Post-Development peak runoff rates for each storm event at the off-site design points is summarized in the tables below:

| Design Point \#DP 1 - Offsite West |  |  |
| :---: | :---: | :---: |
| Storm Event | Pre-Development | Post-Development |
| 2-year | 1.66 cfs | 1.54 cfs |
| $10-$ year | 3.25 cfs | 2.95 cfs |
| $25-$ year | 4.55 cfs | 4.10 cfs |
| 100-year | 7.25 cfs | 6.48 cfs |


| Design Point \# DP 2 - Offsite North |  |  |
| :---: | :---: | :---: |
| Storm Event | Pre-Development | Post-Development |
| 2-year | 1.82 cfs | 1.41 cfs |
| 10-year | 3.71 cfs | 2.95 cfs |
| 25 -year | 5.28 cfs | 4.03 cfs |
| 100-year | 8.58 cfs | 6.07 cfs |

Design Point \# DP 3 - Offsite West Wetland

| Design Point \# DP 3 - Offsite West Wetland |  |  |
| :---: | :---: | :---: |
| Storm Event | Pre-Development | Post-Development |
| 2-year | 1.82 cfs | 1.80 cfs |
| 10-year | 3.37 cfs | 3.28 cfs |
| $25-$ year | 4.61 cfs | 4.46 cfs |
| 100-year | 7.17 cfs | 6.88 cfs |


| Design Point \# DP 4 - Offsite East Wetland |  |  |
| :---: | :---: | :---: |
| Storm Event | Pre-Development | Post-Development |
| 2-year | 1.44 cfs | 1.25 cfs |
| 10-year | 2.81 cfs | 2.45 cfs |
| 25-year | 3.93 cfs | 3.43 cfs |
| 100-year | 6.27 cfs | 5.48 cfs |


| Design Point \# DP 5 - Offsite Northwest |  |  |
| :---: | :---: | :---: |
| Storm Event | Pre-Development | Post-Development |
| 2-year | 1.89 cfs | 1.71 cfs |
| 10-year | 3.92 cfs | 3.27 cfs |
| 25-year | 5.62 cfs | 4.53 cfs |
| 100-year | 9.21 cfs | 7.15 cfs |


| Design Point \# DP 6 - Offsite East |  |  |
| :---: | :---: | :---: |
| Storm Event | Pre-Development | Post-Development |
| 2-year | 2.04 cfs | 1.59 cfs |
| 10-year | 4.50 cfs | 2.73 cfs |
| 25-year | 6.61 cfs | 3.58 cfs |
| 100-year | 11.13 cfs | 7.96 cfs |

As shown in the tables, the peak rates for stormwater runoff generated under Post Development condition will be equal to or less than the peak rates generated under Pre-Development conditions for the all storm events.

Complete runoff calculations for the $2,10,25$, and 100-year Type III storm events including cover, soil types and times of concentration paths for the Pre-Development conditions and Post Development conditions are provided in Appendix A. Also watershed plans are provided in Appendix B.

## Standard 3: Groundwater Recharge

Based upon a review of the Web Soil Survey, soils within the proposed development area have been identified as a "complex" of hydrological soil groups ranging from A to D; therefore, we are choosing the least favorable hydrological soil group with the lowest infiltration rate. The groundwater recharge volume required for the proposed impervious surfaces is calculated by the following formula:

$$
\begin{gathered}
R v=(F)(\text { AIMP }) \\
R v=\text { Required Recharge Volume } \\
F=\text { Target Depth Factor: } 0.10 \text { inch } \\
\text { AIMP }=\text { Proposed Impervious Area }
\end{gathered}
$$

The total area of new impervious is equal to $1,749 \mathrm{SF}$. The calculations in Appendix C show that the BMP recharge volumes exceeds the required recharge volumes. As a result, it is our opinion that Standard 3 has been met.

## Standard 4: Water Quality Volume

Except for a 50 -foot-long paved apron at the site entrance, there are no new impervious surfaces proposed as part of the solar project that will generate suspended solids or other measurable stormwater contaminants. The only measurable impervious surface on the site is the concrete electrical equipment pads and those will be limited to foot traffic only. All access roads will be gravel which, DEP Program Policy 17.1 does not consider impervious and the area under the arrays will be comprised of tall grasses. Runoff from the small, paved apron at the site entrance is treated by a deep sump catch basin and a subsurface infiltration system.

Therefore, it is our opinion that Standard 4 has been met.
Standard 5: Land Uses with Higher Pollutant Loads (LUHPPLs)
The proposed development is not a LUHPPL and therefore Standard 5 is not applicable.

## Standard 6: Critical Areas

The project does not have any discharges within a Zone II, Interim Wellhead Protection Areas or near or to any Critical Areas as defined by the Massachusetts Stormwater Handbook and therefore Standard 6 is not applicable.

## Standard 7: Redevelopment Projects

The proposed project is not a redevelopment project and therefore Standard 7 is not applicable.
Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control Construction Period Pollution Prevention and Erosion and Sedimentation Control A Construction Period Erosion and Sedimentation Control Plan is provided on the Site plans along with notes/instructions for the contractor and details/location of all erosion control measures.

## Standard 9: Post-Construction Long Term Stormwater Operation and Maintenance Plan

 A Post-Construction Long Term Stormwater Operation and Maintenance Plan is provided in Appendix E.
## Standard 10: Prohibition of Illicit Discharges

To our knowledge, there are no existing illicit discharges to existing stormwater systems on the Site and measures to prevent illicit discharges from the proposed development to proposed stormwater systems on the Site will be included within the Post-Construction Long Term Pollution Prevention Plan. As required, an Illicit Discharge Compliance Statement will be submitted prior to the discharge of any stormwater to the post-construction stormwater Best Management Practices (BMPs).

### 4.0 COMPLIANCE WITH TOWN OF FRANKLIN STORMWATER MANAGEMENT BYLAW

Per Section 153-16.B.(1)(a) of the Town of Franklin Stormwater Management Bylaw Chapter 153, which states "in addition to meeting the requirements of the Massachusetts Stormwater Standards,
as required under the Town of Franklin MS4 stormwater permit, all stormwater management systems shall meet the following criteria:
(1) For new development sites, all stormwater management systems shall be designed to:
(a) Retain the volume of runoff equivalent to, or greater than, 1.0 inch multiplied by the total post-construction impervious surface area on the site.

Calculations are provided in Appendix C which show that the proposed BMP's consisting of a Cultec Infiltration System and a proposed stone infiltration trench are sized to accommodate 1.0 inches of rainfall over the proposed impervious areas on the site. Therefore, in our opinion, the design meets the standards of bylaw.

APPENDIX A
MassDEP Checklist for Stormwater Report

Massachusetts Department of Environmental Protection Bureau of Resource Protection - Wetlands Program

## Checklist for Stormwater Report

## A. Introduction

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.


A Stormwater Report must be submitted with the Notice of Intent permit application to document compliance with the Stormwater Management Standards. The following checklist is NOT a substitute for the Stormwater Report (which should provide more substantive and detailed information) but is offered here as a tool to help the applicant organize their Stormwater Management documentation for their Report and for the reviewer to assess this information in a consistent format. As noted in the Checklist, the Stormwater Report must contain the engineering computations and supporting information set forth in Volume 3 of the Massachusetts Stormwater Handbook. The Stormwater Report must be prepared and certified by a Registered Professional Engineer (RPE) licensed in the Commonwealth.

The Stormwater Report must include:

- The Stormwater Checklist completed and stamped by a Registered Professional Engineer (see page 2) that certifies that the Stormwater Report contains all required submittals. ${ }^{1}$ This Checklist is to be used as the cover for the completed Stormwater Report.
- Applicant/Project Name
- Project Address
- Name of Firm and Registered Professional Engineer that prepared the Report
- Long-Term Pollution Prevention Plan required by Standards 4-6
- Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan required by Standard $8^{2}$
- Operation and Maintenance Plan required by Standard 9

In addition to all plans and supporting information, the Stormwater Report must include a brief narrative describing stormwater management practices, including environmentally sensitive site design and LID techniques, along with a diagram depicting runoff through the proposed BMP treatment train. Plans are required to show existing and proposed conditions, identify all wetland resource areas, NRCS soil types, critical areas, Land Uses with Higher Potential Pollutant Loads (LUHPPL), and any areas on the site where infiltration rate is greater than 2.4 inches per hour. The Plans shall identify the drainage areas for both existing and proposed conditions at a scale that enables verification of supporting calculations.

As noted in the Checklist, the Stormwater Management Report shall document compliance with each of the Stormwater Management Standards as provided in the Massachusetts Stormwater Handbook. The soils evaluation and calculations shall be done using the methodologies set forth in Volume 3 of the Massachusetts Stormwater Handbook.

To ensure that the Stormwater Report is complete, applicants are required to fill in the Stormwater Report Checklist by checking the box to indicate that the specified information has been included in the Stormwater Report. If any of the information specified in the checklist has not been submitted, the applicant must provide an explanation. The completed Stormwater Report Checklist and Certification must be submitted with the Stormwater Report.

[^0]Massachusetts Department of Environmental Protection
Bureau of Resource Protection - Wetlands Program

## Checklist for Stormwater Report

## B. Stormwater Checklist and Certification

The following checklist is intended to serve as a guide for applicants as to the elements that ordinarily need to be addressed in a complete Stormwater Report. The checklist is also intended to provide conservation commissions and other reviewing authorities with a summary of the components necessary for a comprehensive Stormwater Report that addresses the ten Stormwater Standards.

Note: Because stormwater requirements vary from project to project, it is possible that a complete Stormwater Report may not include information on some of the subjects specified in the Checklist. If it is determined that a specific item does not apply to the project under review, please note that the item is not applicable (N.A.) and provide the reasons for that determination.

A complete checklist must include the Certification set forth below signed by the Registered Professional Engineer who prepared the Stormwater Report.

## Registered Professional Engineer's Certification

I have reviewed the Stormwater Report, including the soil evaluation, computations, Long-term Pollution Prevention Plan, the Construction Period Erosion and Sedimentation Control Plan (if included), the Longterm Post-Construction Operation and Maintenance Plan, the Illicit Discharge Compliance Statement (if included) and the plans showing the stormwater management system, and have determined that they have been prepared in accordance with the requirements of the Stormwater Management Standards as further elaborated by the Massachusetts Stormwater Handbook. I have also determined that the information presented in the Stormwater Checklist is accurate and that the information presented in the Stormwater Report accurately reflects conditions at the site as of the date of this permit application.

Registered Professional Engineer Block and Signature


## Checklist

Project Type: Is the application for new development, redevelopment, or a mix of new and redevelopment?

New development
$\square$ Redevelopment
$\square$ Mix of New Development and Redevelopment

## Checklist for Stormwater Report

## Checklist (continued)

LID Measures: Stormwater Standards require LID measures to be considered. Document what environmentally sensitive design and LID Techniques were considered during the planning and design of the project:
$\boxtimes$ No disturbance to any Wetland Resource Areas
$\square$ Site Design Practices (e.g. clustered development, reduced frontage setbacks)
$\square$ Reduced Impervious Area (Redevelopment Only)
$\square$ Minimizing disturbance to existing trees and shrubs
$\square$ LID Site Design Credit Requested:
Credit 1
Credit 2
Credit 3
$\boxtimes$ Use of "country drainage" versus curb and gutter conveyance and pipe
Bioretention Cells (includes Rain Gardens)
$\square$ Constructed Stormwater Wetlands (includes Gravel Wetlands designs)
Treebox FilterWater Quality Swale
Q Grass Channel
$\square$ Green Roof
$\boxtimes$ Other (describe):
Detention Basins and Subsurface Infiltration System

## Standard 1: No New Untreated Discharges

$\boxtimes$ No new untreated discharges
$\boxtimes$ Outlets have been designed so there is no erosion or scour to wetlands and waters of the Commonwealth
$\boxtimes$ Supporting calculations specified in Volume 3 of the Massachusetts Stormwater Handbook included.

# Checklist for Stormwater Report 

## Checklist (continued)

## Standard 2: Peak Rate Attenuation

$\square$ Standard 2 waiver requested because the project is located in land subject to coastal storm flowage and stormwater discharge is to a wetland subject to coastal flooding.
$\square$ Evaluation provided to determine whether off-site flooding increases during the 100-year 24-hour storm.
$\boxtimes$ Calculations provided to show that post-development peak discharge rates do not exceed predevelopment rates for the 2 -year and 10 -year 24 -hour storms. If evaluation shows that off-site flooding increases during the 100-year 24-hour storm, calculations are also provided to show that post-development peak discharge rates do not exceed pre-development rates for the 100-year 24hour storm.

## Standard 3: Recharge

$\square$ Soil Analysis provided.
$\boxtimes$ Required Recharge Volume calculation provided.
$\square$ Required Recharge volume reduced through use of the LID site Design Credits.
$\boxtimes$ Sizing the infiltration, BMPs is based on the following method: Check the method used.
$\boxtimes$ Static
$\square$ Simple Dynamic
$\square$ Dynamic Field ${ }^{1}$
$\boxtimes$ Runoff from all impervious areas at the site discharging to the infiltration BMP.
Runoff from all impervious areas at the site is not discharging to the infiltration BMP and calculations are provided showing that the drainage area contributing runoff to the infiltration BMPs is sufficient to generate the required recharge volume.
$\boxtimes$ Recharge BMPs have been sized to infiltrate the Required Recharge Volume.
$\square$ Recharge BMPs have been sized to infiltrate the Required Recharge Volume only to the maximum extent practicable for the following reason:
$\square$ Site is comprised solely of $C$ and $D$ soils and/or bedrock at the land surface
$\square$ M.G.L. c. 21E sites pursuant to 310 CMR 40.0000
$\square$ Solid Waste Landfill pursuant to 310 CMR 19.000
$\square$ Project is otherwise subject to Stormwater Management Standards only to the maximum extent practicable.

Calculations showing that the infiltration BMPs will drain in 72 hours are provided.
$\square$ Property includes a M.G.L. c. 21E site or a solid waste landfill and a mounding analysis is included.

[^1]
## Checklist for Stormwater Report

## Checklist (continued)

## Standard 3: Recharge (continued)

The infiltration BMP is used to attenuate peak flows during storms greater than or equal to the 10 year 24-hour storm and separation to seasonal high groundwater is less than 4 feet and a mounding analysis is provided.
$\square$ Documentation is provided showing that infiltration BMPs do not adversely impact nearby wetland resource areas.

## Standard 4: Water Quality

The Long-Term Pollution Prevention Plan typically includes the following:

- Good housekeeping practices;
- Provisions for storing materials and waste products inside or under cover;
- Vehicle washing controls;
- Requirements for routine inspections and maintenance of stormwater BMPs;
- Spill prevention and response plans;
- Provisions for maintenance of lawns, gardens, and other landscaped areas;
- Requirements for storage and use of fertilizers, herbicides, and pesticides;
- Pet waste management provisions;
- Provisions for operation and management of septic systems;
- Provisions for solid waste management;
- Snow disposal and plowing plans relative to Wetland Resource Areas;
- Winter Road Salt and/or Sand Use and Storage restrictions;
- Street sweeping schedules;
- Provisions for prevention of illicit discharges to the stormwater management system;
- Documentation that Stormwater BMPs are designed to provide for shutdown and containment in the event of a spill or discharges to or near critical areas or from LUHPPL;
- Training for staff or personnel involved with implementing Long-Term Pollution Prevention Plan;
- List of Emergency contacts for implementing Long-Term Pollution Prevention Plan.
$\boxtimes$ A Long-Term Pollution Prevention Plan is attached to Stormwater Report and is included as an attachment to the Wetlands Notice of Intent.
$\square$ Treatment BMPs subject to the $44 \%$ TSS removal pretreatment requirement and the one inch rule for calculating the water quality volume are included, and discharge:
$\square$ is within the Zone II or Interim Wellhead Protection Area
$\square$ is near or to other critical areas
$\square$ is within soils with a rapid infiltration rate (greater than 2.4 inches per hour)
$\square$ involves runoff from land uses with higher potential pollutant loads.
The Required Water Quality Volume is reduced through use of the LID site Design Credits.
$\square$ Calculations documenting that the treatment train meets the $80 \%$ TSS removal requirement and, if applicable, the $44 \%$ TSS removal pretreatment requirement, are provided.


# Checklist for Stormwater Report 

## Checklist (continued)

## Standard 4: Water Quality (continued)

$\boxtimes$ The BMP is sized (and calculations provided) based on:
$\boxtimes$ The $1 / 2^{\prime \prime}$ or 1 " Water Quality Volume or
$\square$ The equivalent flow rate associated with the Water Quality Volume and documentation is provided showing that the BMP treats the required water quality volume.

The applicant proposes to use proprietary BMPs, and documentation supporting use of proprietary BMP and proposed TSS removal rate is provided. This documentation may be in the form of the propriety BMP checklist found in Volume 2, Chapter 4 of the Massachusetts Stormwater Handbook and submitting copies of the TARP Report, STEP Report, and/or other third party studies verifying performance of the proprietary BMPs.
$\square$ A TMDL exists that indicates a need to reduce pollutants other than TSS and documentation showing that the BMPs selected are consistent with the TMDL is provided.

## Standard 5: Land Uses With Higher Potential Pollutant Loads (LUHPPLs)

$\square$ The NPDES Multi-Sector General Permit covers the land use and the Stormwater Pollution Prevention Plan (SWPPP) has been included with the Stormwater Report.
$\square$ The NPDES Multi-Sector General Permit covers the land use and the SWPPP will be submitted prior to the discharge of stormwater to the post-construction stormwater BMPs.
$\square$ The NPDES Multi-Sector General Permit does not cover the land use.
LUHPPLs are located at the site and industry specific source control and pollution prevention measures have been proposed to reduce or eliminate the exposure of LUHPPLs to rain, snow, snow melt and runoff, and been included in the long term Pollution Prevention Plan.All exposure has been eliminated.
$\square$ All exposure has not been eliminated and all BMPs selected are on MassDEP LUHPPL list.
$\square$ The LUHPPL has the potential to generate runoff with moderate to higher concentrations of oil and grease (e.g. all parking lots with $>1000$ vehicle trips per day) and the treatment train includes an oil grit separator, a filtering bioretention area, a sand filter or equivalent.

## Standard 6: Critical Areas

The discharge is near or to a critical area and the treatment train includes only BMPs that MassDEP has approved for stormwater discharges to or near that particular class of critical area.
$\square$ Critical areas and BMPs are identified in the Stormwater Report.

## Checklist for Stormwater Report

## Checklist (continued)

## Standard 7: Redevelopments and Other Projects Subject to the Standards only to the maximum extent practicable

The project is subject to the Stormwater Management Standards only to the maximum Extent Practicable as a:

Limited Project
$\square$ Small Residential Projects: 5-9 single family houses or 5-9 units in a multi-family development provided there is no discharge that may potentially affect a critical area.
$\square$ Small Residential Projects: 2-4 single family houses or 2-4 units in a multi-family development with a discharge to a critical area
$\square$ Marina and/or boatyard provided the hull painting, service and maintenance areas are protected from exposure to rain, snow, snow melt and runoff
$\square$ Bike Path and/or Foot PathRedevelopment Project
Redevelopment portion of mix of new and redevelopment.
Certain standards are not fully met (Standard No. 1, 8, 9, and 10 must always be fully met) and an explanation of why these standards are not met is contained in the Stormwater Report.
The project involves redevelopment and a description of all measures that have been taken to improve existing conditions is provided in the Stormwater Report. The redevelopment checklist found in Volume 2 Chapter 3 of the Massachusetts Stormwater Handbook may be used to document that the proposed stormwater management system (a) complies with Standards 2, 3 and the pretreatment and structural BMP requirements of Standards $4-6$ to the maximum extent practicable and (b) improves existing conditions.

## Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control

A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan must include the following information:

- Narrative;
- Construction Period Operation and Maintenance Plan;
- Names of Persons or Entity Responsible for Plan Compliance;
- Construction Period Pollution Prevention Measures;
- Erosion and Sedimentation Control Plan Drawings;
- Detail drawings and specifications for erosion control BMPs, including sizing calculations;
- Vegetation Planning;
- Site Development Plan;
- Construction Sequencing Plan;
- Sequencing of Erosion and Sedimentation Controls;
- Operation and Maintenance of Erosion and Sedimentation Controls;
- Inspection Schedule;
- Maintenance Schedule;
- Inspection and Maintenance Log Form.
$\boxtimes$ A Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan containing the information set forth above has been included in the Stormwater Report.


## Checklist for Stormwater Report

## Checklist (continued)

## Standard 8: Construction Period Pollution Prevention and Erosion and Sedimentation Control (continued)

The project is highly complex and information is included in the Stormwater Report that explains why it is not possible to submit the Construction Period Pollution Prevention and Erosion and Sedimentation Control Plan with the application. A Construction Period Pollution Prevention and Erosion and Sedimentation Control has not been included in the Stormwater Report but will be submitted before land disturbance begins.
$\square$ The project is not covered by a NPDES Construction General Permit.
$\square$ The project is covered by a NPDES Construction General Permit and a copy of the SWPPP is in the Stormwater Report.
$\boxtimes$ The project is covered by a NPDES Construction General Permit but no SWPPP been submitted. The SWPPP will be submitted BEFORE land disturbance begins.

## Standard 9: Operation and Maintenance Plan

The Post Construction Operation and Maintenance Plan is included in the Stormwater Report and includes the following information:
$\boxtimes$ Name of the stormwater management system owners;
$\boxtimes$ Party responsible for operation and maintenance;
$\boxtimes$ Schedule for implementation of routine and non-routine maintenance tasks;
$\square$ Plan showing the location of all stormwater BMPs maintenance access areas;
$\square$ Description and delineation of public safety features;
$\boxtimes$ Estimated operation and maintenance budget; and
$\boxtimes$ Operation and Maintenance Log Form.
The responsible party is not the owner of the parcel where the BMP is located and the Stormwater Report includes the following submissions:
$\square$ A copy of the legal instrument (deed, homeowner's association, utility trust or other legal entity) that establishes the terms of and legal responsibility for the operation and maintenance of the project site stormwater BMPs;
$\square$ A plan and easement deed that allows site access for the legal entity to operate and maintain BMP functions.

## Standard 10: Prohibition of Illicit Discharges

The Long-Term Pollution Prevention Plan includes measures to prevent illicit discharges;
$\boxtimes$ An Illicit Discharge Compliance Statement is attached;
$\square$
NO Illicit Discharge Compliance Statement is attached but will be submitted prior to the discharge of any stormwater to post-construction BMPs.

## APPENDIX B

Pre- and Post-Development HydroCAD Calculations


### 3328.00-PRE

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Page 2

## Area Listing (all nodes)

| Area <br> $(\mathrm{sq}-\mathrm{ft})$ | CN | Description <br> (subcatchment-numbers) |
| ---: | ---: | :--- |
| 40,762 | 80 | $>75 \%$ Grass cover, Good, HSG D (1S, 2S) |
| 57,290 | 83 | Brush, Poor, HSG D (3S, 4S) |
| 1,869 | 96 | Gravel surface, HSG D (3S, 4S) |
| 3,240 | 98 | Roofs, HSG D (1S, 2S) |
| 37,709 | 70 | Woods, Good, HSG C (4S, 5S, 6S) |
| 178,720 | 77 | Woods, Good, HSG D (1S, 2S, 3S, 4S, 5S, 6S) |
| $\mathbf{3 1 9 , 5 9 0}$ | $\mathbf{7 8}$ | TOTAL AREA |

### 3328.00-PRE

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## Soil Listing (all nodes)

| Area <br> $(\mathrm{sq-ft})$ | Soil <br> Group | Subcatchment <br> Numbers |
| ---: | :--- | :--- |
| 00 | HSG A |  |
| 0 | HSG B |  |
| 37,709 | HSG C | $4 \mathrm{~S}, 5 \mathrm{~S}, 6 \mathrm{~S}$ |
| 281,881 | HSG D | $1 \mathrm{~S}, 2 \mathrm{~S}, 3 \mathrm{~S}, 4 \mathrm{~S}, 5 \mathrm{~S}, 6 \mathrm{~S}$ |
| 0 | Other |  |
| 319,590 |  | TOTAL AREA |

### 3328.00-PRE

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## Ground Covers (all nodes)

| HSG-A <br> $(\mathrm{sq-ft})$ | HSG-B <br> $(\mathrm{sq-ft})$ | HSG-C <br> $(\mathrm{sq-ft})$ | HSG-D <br> $(\mathrm{sq-ft})$ | Other <br> $(\mathrm{sq-ft})$ | Total <br> $(\mathrm{sq-ft})$ | Ground <br> Cover |
| ---: | ---: | ---: | ---: | ---: | ---: | :--- |
| 0 | 0 | 0 | 40,762 | 0 | 40,762 | $>75 \%$ Grass |
|  |  |  |  |  |  | cover, Good |
| 0 | 0 | 0 | 57,290 | 0 | 57,290 | Brush, Poor |
| 0 | 0 | 0 | 1,869 | 0 | 1,869 | Gravel surface |
| 0 | 0 | 0 | 3,240 | 0 | 3,240 | Roofs |
| 0 | 0 | 37,709 | 178,720 | 0 | 216,429 | Woods, Good |
| 0 | $\mathbf{0}$ | $\mathbf{3 7 , 7 0 9}$ | $\mathbf{2 8 1 , 8 8 1}$ | $\mathbf{0}$ | $\mathbf{3 1 9 , 5 9 0}$ | TOTAL AREA |

Sub
Nun

Time span $=0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}, 7201$ points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

## Subcatchment1S:

## Subcatchment2S:

## Subcatchment3S:

## Subcatchment4S:

## Subcatchment5S:

Subcatchment6S: (new Subcat)

Reach DP 1: Towards Offsite West

## Reach DP 2: Towards Offsite North

Reach DP 3: Towards West Wetland

Reach DP 4: Towards East Wetland

Reach DP 5: Towards Offsite Northwest

Reach DP 6: Towards Offsite East

Runoff Area=44,718 sf $5.44 \%$ Impervious Runoff Depth $=1.42^{\prime \prime}$ Flow Length=312' $\quad \mathrm{Cc}=8.5 \mathrm{~min} \quad \mathrm{CN}=80$ Runoff=1.66 cfs $5,282 \mathrm{cf}$

Runoff Area=60,728 sf $1.33 \%$ Impervious Runoff Depth=1.29" Flow Length=324' Tc=11.4 min CN=78 Runoff=1.82 cfs 6,518 cf

Runoff Area=39,495 sf $0.00 \%$ Impervious Runoff Depth=1.63" Flow Length=212' Tc=6.7 min CN=83 Runoff=1.82 cfs 5,350 cf

Runoff Area $=38,018$ sf $0.00 \%$ Impervious Runoff Depth $=1.42^{\prime \prime}$ Flow Length=122' $\quad$ cc=8.2 min $C N=80$ Runoff $=1.44$ cfs 4,490 cf

Runoff Area=61,964 sf $0.00 \%$ Impervious Runoff Depth $=1.23$ " Flow Length=357' Tc=9.6 min CN=77 Runoff=1.89 cfs $6,331 \mathrm{cf}$

Runoff Area=74,667 sf $0.00 \%$ Impervious Runoff Depth=1.05" Flow Length=427' Tc=8.2 min CN=74 Runoff=2.04 cfs 6,538 cf

Inflow=1.66 cfs 5,282 cf Outflow=1.66 cfs 5,282 cf

Inflow=1.82 cfs $6,518 \mathrm{cf}$ Outflow=1.82 cfs $6,518 \mathrm{cf}$

Inflow=1.82 cfs 5,350 cf Outflow=1.82 cfs 5,350 cf

Inflow=1.44 cfs 4,490 cf Outflow=1.44 cfs $4,490 \mathrm{cf}$

Inflow=1.89 cfs 6,331 cf Outflow=1.89 cfs $6,331 \mathrm{cf}$

Inflow=2.04 cfs 6,538 cf Outflow=2.04 cfs 6,538 cf

$$
\begin{aligned}
& \text { Total Runoff Area }=319,590 \text { sf Runoff Volume }=34,510 \text { cf Average Runoff Depth }=1.30 \text { " } \\
& \text { 98.99\% Pervious }=316,350 \text { sf } 1.01 \% \text { Impervious }=3,240 \text { sf }
\end{aligned}
$$

## Summary for Subcatchment 1S:

Runoff $=1.66$ cfs @ 12.16 hrs, Volume $=\quad 5,282 \mathrm{cf}$, Depth= $1.42^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

| Area (sf) | CN | Description |
| ---: | ---: | :--- | :--- |
| 16,836 <br> 2,432 | 77 <br> 98 | Woods, Good, HSG D <br> Roofs, HSG D |
| 25,450 | 80 | >75\% Grass cover, Good, HSG D |

## Subcatchment 1S:


$\square$ Runoff

## Summary for Subcatchment 2S:

Runoff $=1.82$ cfs @ 12.20 hrs, Volume= $\quad 6,518 \mathrm{cf}$, Depth= $1.29{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"


## Subcatchment 2S:


$\square$ Runoff

## Summary for Subcatchment 3S:

Runoff $=1.82$ cfs @ 12.14 hrs, Volume $=\quad 5,350 \mathrm{cf}$, Depth= $1.63^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"


## Subcatchment 3S:



## Summary for Subcatchment 4S:

Runoff $=\quad 1.44$ cfs @ 12.16 hrs, Volume $=\quad 4,490 \mathrm{cf}$, Depth= $1.42^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"


## Subcatchment 4S:

Hydrograph


## Summary for Subcatchment 5S:

Runoff = 1.89 cfs @
12.17 hrs , Volume=
6,331 cf, Depth= 1.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{array}{r} 3,001 \\ 58,963 \\ \hline \end{array}$ |  | Woods, Good, HSG C Woods, Good, HSG D |  |  |
|  | $\begin{aligned} & 61,964 \\ & 61,964 \end{aligned}$ | 77 | Weighted Average 100.00\% Pervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 6.4 | 50 | 0.0940 | 0.13 |  | Sheet Flow, <br> Woods: Light underbrush $n=0.400 \quad \mathrm{P} 2=3.44$ " |
| 3.2 | 307 | 0.1050 | - 1.62 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 9.6 | 357 | Total |  |  |  |



## Summary for Subcatchment 6S: (new Subcat)

Runoff $=\quad 2.04$ cfs @ 12.16 hrs, Volume $=\quad 6,538 \mathrm{cf}$, Depth= $1.05{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"


Subcatchment 6S: (new Subcat)


## Summary for Reach DP 1: Towards Offsite West

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =
$44,718 \mathrm{sf}, \quad 5.44 \%$ Impervious, Inflow Depth = 1.42" for 2-Year event 1.66 cfs @ 12.16 hrs, Volume= 1.66 cfs @ 12.16 hrs, Volume=

5,282 cf
$5,282 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 1: Towards Offsite West



Summary for Reach DP 2: Towards Offsite North
[40] Hint: Not Described (Outflow=Inflow)
Inflow Area $=\quad 60,728$ sf, $1.33 \%$ Impervious, Inflow Depth $=1.29$ " for 2-Year event
Inflow = 1.82 cfs @ 12.20 hrs, Volume=

6,518 cf
Outflow =
1.82 cfs @ 12.20 hrs , Volume=
$6,518 \mathrm{cf}$, Atten $=0 \%, L a g=0.0 \mathrm{~min}$
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 2: Towards Offsite North


## Summary for Reach DP 3: Towards West Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =
$39,495 \mathrm{sf}, \quad 0.00 \%$ Impervious, Inflow Depth = 1.63" for 2-Year event 1.82 cfs @ 12.14 hrs, Volume= 5,350 cf

Routing by Stor-Ind+Trans method, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$

## Reach DP 3: Towards West Wetland



## Summary for Reach DP 4: Towards East Wetland

[40] Hint: Not Described (Outflow=Inflow)

| low A | 38,018 sf, | 0.00\% Impervious, |  |
| :---: | :---: | :---: | :---: |
| Inflow | 1.44 cfs @ | 12.16 hrs , Volume= | 4,490 cf |
| Outflow | 1.44 cfs @ | 12.16 hrs, Volume= | $4,490 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{mi}$ |

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 4: Towards East Wetland


## Summary for Reach DP 5: Towards Offsite Northwest

[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
61,964 sf, $0.00 \%$ Impervious, Inflow Depth = 1.23" for 2-Year event
Outflow =
1.89 cfs @ 12.17 hrs, Volume=

6,331 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 5: Towards Offsite Northwest
Hydrograph


## Summary for Reach DP 6: Towards Offsite East

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

74,667 sf, $0.00 \%$ Impervious, Inflow Depth = 1.05" for 2-Year event 2.04 cfs @ 12.16 hrs, Volume=

6,538 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 6: Towards Offsite East


Time span $=0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}, 7201$ points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

## Subcatchment1S:

## Subcatchment2S:

## Subcatchment3S:

## Subcatchment4S:

## Subcatchment5S:

Subcatchment6S: (new Subcat)

Reach DP 1: Towards Offsite West

Reach DP 2: Towards Offsite North

Reach DP 3: Towards West Wetland

Reach DP 4: Towards East Wetland

Reach DP 5: Towards Offsite Northwest

Reach DP 6: Towards Offsite East

Runoff Area=44,718 sf $5.44 \%$ Impervious Runoff Depth=2.77" Flow Length=312' $\quad$ cc=8.5 min CN=80 Runoff=3.25 cfs $10,326 \mathrm{cf}$

Runoff Area=60,728 sf $1.33 \%$ Impervious Runoff Depth=2.59" Flow Length=324' Tc=11.4 min CN=78 Runoff=3.71 cfs $13,124 \mathrm{cf}$

Runoff Area=39,495 sf $0.00 \%$ Impervious Runoff Depth=3.05" Flow Length=212' Tc=6.7 min CN=83 Runoff=3.37 cfs $10,031 \mathrm{cf}$

Runoff Area=38,018 sf $0.00 \%$ Impervious Runoff Depth=2.77" Flow Length=122' $\quad$ cc=8.2 min $C N=80$ Runoff $=2.81$ cfs 8,779 cf

Runoff Area=61,964 sf $0.00 \%$ Impervious Runoff Depth=2.51" Flow Length=357' Tc=9.6 min CN=77 Runoff=3.92 cfs $12,942 \mathrm{cf}$

Runoff Area=74,667 sf $0.00 \%$ Impervious Runoff Depth=2.25" Flow Length=427' Tc=8.2 min CN=74 Runoff=4.50 cfs $14,019 \mathrm{cf}$

Inflow=3.25 cfs 10,326 cf Outflow=3.25 cfs 10,326 cf

Inflow=3.71 cfs 13,124 cf Outflow=3.71 cfs 13,124 cf

Inflow=3.37 cfs 10,031 cf Outflow=3.37 cfs 10,031 cf

Inflow=2.81 cfs 8,779 cf Outflow=2.81 cfs $8,779 \mathrm{cf}$

Inflow=3.92 cfs $12,942 \mathrm{cf}$ Outflow=3.92 cfs 12,942 cf

Inflow=4.50 cfs $14,019 \mathrm{cf}$ Outflow=4.50 cfs 14,019 cf

## Summary for Subcatchment 1S:

Runoff $=\quad 3.25$ cfs @ 12.16 hrs, Volume $=\quad 10,326 \mathrm{cf}$, Depth= 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 10-Year Rainfall=4.86"


## Subcatchment 1S:


$\square$ Runoff

## Summary for Subcatchment 2S:

Runoff $=3.71$ cfs @ 12.19 hrs, Volume $=\quad 13,124 \mathrm{cf}$, Depth= 2.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 10-Year Rainfall=4.86"


## Subcatchment 2S:


$\square$ Runoff

## Summary for Subcatchment 3S:

Runoff $=3.37$ cfs @ 12.14 hrs, Volume $=\quad 10,031 \mathrm{cf}$, Depth= 3.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 10-Year Rainfall=4.86"


## Subcatchment 3S:


$\square$ Runoff

## Summary for Subcatchment 4S:

Runoff $=\quad 2.81 \mathrm{cfs} @ 12.16 \mathrm{hrs}$, Volume $=\quad 8,779 \mathrm{cf}$, Depth= 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 10-Year Rainfall=4.86"


## Subcatchment 4S:



## Summary for Subcatchment 5S:

Runoff $=3.92$ cfs @ 12.17 hrs, Volume $=12,942 \mathrm{cf}$, Depth= 2.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 10-Year Rainfall=4.86"


Subcatchment 5S:


## Summary for Subcatchment 6S: (new Subcat)

Runoff $=\quad 4.50$ cfs @ 12.16 hrs, Volume $=14,019 \mathrm{cf}$, Depth= 2.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 10-Year Rainfall=4.86"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{array}{r} 46,947 \\ 27,720 \\ \hline \end{array}$ |  | Woods, Good, HSG D Woods, Good, HSG C |  |  |
|  | $\begin{aligned} & \hline 74,667 \\ & 74,667 \end{aligned}$ | 74 | Weighted Average 100.00\% Pervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 4.4 | 50 | 0.2400 | 0.19 |  | Sheet Flow, <br> Woods: Light underbrush $n=0.400 \quad \mathrm{P} 2=3.44 "$ |
| 3.8 | 377 | 0.1120 | 1.67 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 8.2 | 427 | Total |  |  |  |

Subcatchment 6S: (new Subcat)


## Summary for Reach DP 1: Towards Offsite West

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =
$44,718 \mathrm{sf}, \quad 5.44 \%$ Impervious, Inflow Depth = 2.77" for 10-Year event 3.25 cfs @ 12.16 hrs, Volume= 10,326 cf 3.25 cfs @ 12.16 hrs, Volume= 10,326 cf, Atten= 0\%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 1: Towards Offsite West



Summary for Reach DP 2: Towards Offsite North
[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
$60,728 \mathrm{sf}$, 1.33\% Impervious, Inflow Depth = 2.59" for 10 -Year event
Outflow =
3.71 cfs @ 12.19 hrs , Volume=

13,124 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 2: Towards Offsite North


## Summary for Reach DP 3: Towards West Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

39,495 sf, 0.00\% Impervious, Inflow Depth = 3.05" for 10-Year event 3.37 cfs @ 12.14 hrs, Volume= 10,031 cf 3.37 cfs @ 12.14 hrs, Volume= 10,031 cf, Atten= 0\%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 3: Towards West Wetland

Hydrograph


Summary for Reach DP 4: Towards East Wetland
[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
$38,018 \mathrm{sf}, \quad 0.00 \%$ Impervious, Inflow Depth = 2.77" for 10-Year event
Outflow =
2.81 cfs @ 12.16 hrs, Volume=

8,779 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 4: Towards East Wetland


## Summary for Reach DP 5: Towards Offsite Northwest

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

61,964 sf, $0.00 \%$ Impervious, Inflow Depth $=2.51$ " for $10-$ Year event 3.92 cfs @ 12.17 hrs, Volume= 3.92 cfs @ 12.17 hrs, Volume=

12,942 cf
12,942 cf, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 5: Towards Offsite Northwest

Hydrograph


## Summary for Reach DP 6: Towards Offsite East

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

74,667 sf, $0.00 \%$ Impervious, Inflow Depth = 2.25" for 10-Year event
4.50 cfs @ 12.16 hrs, Volume= $14,019 \mathrm{cf}$ $14,019 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 6: Towards Offsite East


Time span $=0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}, 7201$ points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

## Subcatchment1S:

## Subcatchment2S:

## Subcatchment3S:

## Subcatchment4S:

## Subcatchment5S:

Subcatchment6S: (new Subcat)

Reach DP 1: Towards Offsite West

## Reach DP 2: Towards Offsite North

Reach DP 3: Towards West Wetland

Reach DP 4: Towards East Wetland

Reach DP 5: Towards Offsite Northwest

Reach DP 6: Towards Offsite East

Runoff Area=44,718 sf $5.44 \%$ Impervious Runoff Depth=3.92" Flow Length=312' $\quad$ Cc=8.5 min CN=80 Runoff=4.55 cfs $14,596 \mathrm{cf}$

Runoff Area=60,728 sf $1.33 \%$ Impervious Runoff Depth $=3.71^{\prime \prime}$ Flow Length=324' Tc=11.4 min CN=78 Runoff=5.28 cfs $18,784 \mathrm{cf}$

Runoff Area=39,495 sf $0.00 \%$ Impervious Runoff Depth=4.23" Flow Length=212' Tc=6.7 min CN=83 Runoff=4.61 cfs $13,925 \mathrm{cf}$

Runoff Area=38,018 sf $0.00 \%$ Impervious Runoff Depth=3.92" Flow Length=122' Tc=8.2 min CN=80 Runoff=3.93 cfs $12,409 \mathrm{cf}$

Runoff Area=61,964 sf $0.00 \%$ Impervious Runoff Depth $=3.61$ " Flow Length=357' Tc=9.6 min CN=77 Runoff=5.62 cfs $18,643 \mathrm{cf}$

Runoff Area=74,667 sf 0.00\% Impervious Runoff Depth=3.31" Flow Length=427' Tc=8.2 min CN=74 Runoff=6.61 cfs 20,605 cf

Inflow=4.55 cfs 14,596 cf Outflow=4.55 cfs 14,596 cf

Inflow=5.28 cfs 18,784 cf Outflow=5.28 cfs 18,784 cf

Inflow=4.61 cfs $13,925 \mathrm{cf}$ Outflow=4.61 cfs 13,925 cf

Inflow=3.93 cfs $12,409 \mathrm{cf}$ Outflow=3.93 cfs 12,409 cf

Inflow=5.62 cfs $18,643 \mathrm{cf}$ Outflow=5.62 cfs 18,643 cf

Inflow=6.61 cfs 20,605 cf Outflow=6.61 cfs 20,605 cf

## Summary for Subcatchment 1S:

Runoff $=\quad 4.55$ cfs @ 12.16 hrs, Volume $=\quad 14,596 \mathrm{cf}$, Depth= 3.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 25-Year Rainfall=6.15"


## Subcatchment 1S:


$\square$ Runoff

## Summary for Subcatchment 2S:

Runoff $=5.28 \mathrm{cfs} @ 12.19 \mathrm{hrs}$, Volume $=\quad 18,784 \mathrm{cf}$, Depth= $3.71^{\prime \prime}$
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 25-Year Rainfall=6.15"


## Subcatchment 2S:


$\square$ Runoff

## Summary for Subcatchment 3S:

Runoff $=\quad 4.61$ cfs @ 12.14 hrs, Volume $=\quad 13,925 \mathrm{cf}$, Depth= 4.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 25-Year Rainfall=6.15"


## Subcatchment 3S:


$\square$ Runoff

## Summary for Subcatchment 4S:

Runoff $=3.93$ cfs @ 12.15 hrs, Volume= $12,409 \mathrm{cf}$, Depth= 3.92"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 25-Year Rainfall=6.15"


## Subcatchment 4S:

Hydrograph


## Summary for Subcatchment 5S:

Runoff $=5.62$ cfs @ 12.17 hrs, Volume $=\quad 18,643 \mathrm{cf}$, Depth= 3.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 25-Year Rainfall=6.15"


Subcatchment 5S:


## Summary for Subcatchment 6S: (new Subcat)

Runoff $=\quad 6.61$ cfs @ 12.16 hrs, Volume $=\quad 20,605 \mathrm{cf}$, Depth= 3.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 25-Year Rainfall=6.15"

|  | rea (sf) | CN | escription |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{aligned} & \hline 46,947 \\ & 27,720 \end{aligned}$ | $\begin{aligned} & 77 \\ & 70 \\ & \hline \end{aligned}$ | Woods, Good, HSG D Woods, Good, HSG C |  |  |  |
|  | $\begin{aligned} & 74,667 \\ & 74,667 \end{aligned}$ | 74 | Weighted Average 100.00\% Pervious Area |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{tt})$ | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ (\mathrm{cfs}) \end{array}$ | Description |  |
| 4.4 | 50 | 0.2400 | 0.19 |  | Sheet Flow, <br> Woods: Light underbrush $n=0.400$ | P2=3.44" |
| 3.8 | 377 | 0.1120 | 1.67 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |  |
| 8.2 | 427 | Total |  |  |  |  |

Subcatchment 6S: (new Subcat)


## Summary for Reach DP 1: Towards Offsite West

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

44,718 sf, $5.44 \%$ Impervious, Inflow Depth = 3.92" for 25 -Year event 4.55 cfs @ 12.16 hrs, Volume= 4.55 cfs @ 12.16 hrs, Volume=

14,596 cf
$14,596 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 1: Towards Offsite West


Summary for Reach DP 2: Towards Offsite North
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = Inflow = Outflow =

60,728 sf, 1.33\% Impervious, Inflow Depth = 3.71" for 25 -Year event 5.28 cfs @ 12.19 hrs , Volume= 18,784 cf
$18,784 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 2: Towards Offsite North


## Summary for Reach DP 3: Towards West Wetland

[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
39,495 sf, 0.00\% Impervious, Inflow Depth = 4.23" for 25 -Year event
Outflow =
4.61 cfs @ 12.14 hrs, Volume= 13,925 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 3: Towards West Wetland

Hydrograph


Summary for Reach DP 4: Towards East Wetland
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

38,018 sf, $0.00 \%$ Impervious, Inflow Depth = 3.92" for 25 -Year event 3.93 cfs @ 12.15 hrs, Volume= 12,409 cf 3.93 cfs @ 12.15 hrs , Volume= $12,409 \mathrm{cf}$, Atten $=0 \%, \operatorname{Lag}=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 4: Towards East Wetland


## Summary for Reach DP 5: Towards Offsite Northwest

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

61,964 sf, $0.00 \%$ Impervious, Inflow Depth = 3.61" for 25 -Year event 5.62 cfs @ 12.17 hrs, Volume= 18,643 cf
$18,643 \mathrm{cf}$, Atten $=0 \%, \operatorname{Lag}=0.0 \mathrm{~min}$
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 5: Towards Offsite Northwest

Hydrograph


## Summary for Reach DP 6: Towards Offsite East

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = Inflow = Outflow =

74,667 sf, $0.00 \%$ Impervious, Inflow Depth $=3.31$ " for 25 -Year event 6.61 cfs @ 12.16 hrs, Volume= 20,605 cf $20,605 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 6: Towards Offsite East


Time span=0.00-72.00 hrs, $\mathrm{dt}=0.01 \mathrm{hrs}, 7201$ points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

## Subcatchment1S:

## Subcatchment2S:

## Subcatchment3S:

## Subcatchment4S:

## Subcatchment5S:

Subcatchment6S: (new Subcat)

## Reach DP 1: Towards Offsite West

## Reach DP 2: Towards Offsite North

Reach DP 3: Towards West Wetland

Reach DP 4: Towards East Wetland

Reach DP 5: Towards Offsite Northwest

Reach DP 6: Towards Offsite East

Runoff Area=44,718 sf $5.44 \%$ Impervious Runoff Depth=6.38" Flow Length=312' $\mathrm{Tc}=8.5 \mathrm{~min} \mathrm{CN}=80$ Runoff= $7.25 \mathrm{cfs} 23,770 \mathrm{cf}$

Runoff Area=60,728 sf $1.33 \%$ Impervious Runoff Depth=6.13" Flow Length=324' Tc=11.4 min CN=78 Runoff=8.58 cfs 31,047 cf

Runoff Area=39,495 sf $0.00 \%$ Impervious Runoff Depth=6.74" Flow Length=212' Tc=6.7 min CN=83 Runoff=7.17 cfs 22,196 cf

Runoff Area=38,018 sf $0.00 \%$ Impervious Runoff Depth=6.38" Flow Length=122' Tc=8.2 min CN=80 Runoff=6.27 cfs 20,209 cf

Runoff Area=61,964 sf $0.00 \%$ Impervious Runoff Depth=6.01" Flow Length=357' Tc=9.6 min CN=77 Runoff=9.21 cfs $31,049 \mathrm{cf}$

Runoff Area=74,667 sf $0.00 \%$ Impervious Runoff Depth=5.65" Flow Length=427' Tc=8.2 $\mathrm{min} \quad \mathrm{CN}=74$ Runoff=11.13 cfs $35,137 \mathrm{cf}$

Inflow=7.25 cfs 23,770 cf Outflow=7.25 cfs 23,770 cf

Inflow=8.58 cfs $31,047 \mathrm{cf}$ Outflow=8.58 cfs 31,047 cf

Inflow=7.17 cfs 22,196 cf Outflow=7.17 cfs 22,196 cf

Inflow=6.27 cfs 20,209 cf Outflow=6.27 cfs 20,209 cf

Inflow=9.21 cfs $31,049 \mathrm{cf}$ Outflow=9.21 cfs 31,049 cf

Inflow=11.13 cfs $35,137 \mathrm{cf}$ Outflow=11.13 cfs $35,137 \mathrm{cf}$

> Total Runoff Area $=319,590$ sf Runoff Volume $=163,408$ cf Average Runoff Depth $=6.14$ "
> 98.99\% Pervious $=316,350$ sf
> $\mathbf{1 . 0 1 \%}$ Impervious $=\mathbf{3 , 2 4 0} \mathbf{s f}$

## Summary for Subcatchment 1S:

Runoff $=7.25$ cfs @ 12.16 hrs, Volume $=\quad 23,770 \mathrm{cf}$, Depth= 6.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 100-Year Rainfall=8.80"


## Subcatchment 1S:


$\square$ Runoff

## Summary for Subcatchment 2S:

Runoff $=8.58$ cfs @ 12.19 hrs, Volume $=\quad 31,047 \mathrm{cf}$, Depth= 6.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 100-Year Rainfall=8.80"


## Subcatchment 2S:


$\square$ Runoff

## Summary for Subcatchment 3S:

Runoff $=7.17$ cfs @ 12.14 hrs, Volume $=\quad 22,196 \mathrm{cf}$, Depth= 6.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 100-Year Rainfall=8.80"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 5,968 <br> 1,725 <br> 31,802 |  |  | Woods, Good, HSG D Gravel surface, HSG D Brush, Poor, HSG D |  |  |
|  |  |  |  |  |
|  |  |  |  |  |
| $\begin{aligned} & 39,495 \\ & 39,495 \end{aligned}$ |  |  | 83 | Weighted Average 100.00\% Pervious Area |  | Description |
|  |  |  |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \\ \hline \end{array}$ |  |
| 4.5 | 50 | 0.0840 | 0.19 |  | Sheet Flow, <br> Grass: Dense n=0.240 P2=3.44" |  |
| 1.4 | 98 | 0.0286 | 1.18 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |  |
| 0.1 | 12 | 0.0588 | 3.90 |  | Shallow Concentrated Flow, Unpaved Kv= 16.1 fps |  |
| 0.7 | 52 | 0.0323 | 1.26 |  | Shallow Concentrated Flow, <br> Short Grass Pasture $\mathrm{Kv}=7.0 \mathrm{fps}$ |  |
| 6.7 | 212 | Total |  |  |  |  |

## Subcatchment 3S:



## Summary for Subcatchment 4S:

Runoff $=\quad 6.27$ cfs @ 12.15 hrs, Volume $=\quad 20,209 \mathrm{cf}$, Depth $=6.38{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 100-Year Rainfall=8.80"


## Subcatchment 4S:

Hydrograph


## Summary for Subcatchment 5S:

Runoff $=9.21$ cfs @ 12.17 hrs, Volume $=\quad 31,049 \mathrm{cf}$, Depth= 6.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 100-Year Rainfall=8.80"

| Area (sf) CN Description |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{array}{r} \hline 3,001 \\ 58,963 \\ \hline \end{array}$ | $\begin{aligned} & \hline 70 \\ & 77 \\ & \hline \end{aligned}$ | Woods, Good, HSG C Woods, Good, HSG D |  |  |
|  | $\begin{aligned} & \hline 61,964 \\ & 61,964 \end{aligned}$ | 77 Woods, Good, HSG D <br> 77 Weighted Average <br>  $100.00 \%$ Pervious Area | Weighted Average 100.00\% Pervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft)Velocity <br> $(\mathrm{ft} / \mathrm{sec})$Capacity <br> $(\mathrm{cfs})$ |  |  | Description |
| 6.4 | 50 | 0.0940 | 0.13 |  | Sheet Flow, <br> Woods: Light underbrush $n=0.400 \quad \mathrm{P} 2=3.44 "$ |
| 3.2 | 307 | 0.1050 | 1.62 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 9.6 | 357 | Total |  |  |  |

Subcatchment 5S:
Hydrograph


## Summary for Subcatchment 6S: (new Subcat)

Runoff $=11.13$ cfs @ 12.15 hrs, Volume= 35,137 cf, Depth= $5.65{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 100-Year Rainfall=8.80"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{array}{r} 46,947 \\ 27,720 \\ \hline \end{array}$ |  | Woods, Good, HSG D Woods, Good, HSG C |  |  |
|  | $\begin{aligned} & \hline 74,667 \\ & 74,667 \end{aligned}$ | 74 | Weighted Average 100.00\% Pervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 4.4 | 50 | 0.2400 | 0.19 |  | Sheet Flow, <br> Woods: Light underbrush $n=0.400 \quad \mathrm{P} 2=3.44 "$ |
| 3.8 | 377 | 0.1120 | 1.67 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 8.2 | 427 | Total |  |  |  |

Subcatchment 6S: (new Subcat)


## Summary for Reach DP 1: Towards Offsite West

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

44,718 sf, $5.44 \%$ Impervious, Inflow Depth = 6.38" for 100-Year event 7.25 cfs @ 12.16 hrs, Volume= 7.25 cfs @ 12.16 hrs, Volume=
$23,770 \mathrm{cf}$
$23,770 \mathrm{cf}$, Atten $=0 \%, L a g=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 1: Towards Offsite West


Summary for Reach DP 2: Towards Offsite North
[40] Hint: Not Described (Outflow=Inflow)

| Inflow Area $=$ | $60,728 \mathrm{sf}$, | $1.33 \%$ Impervious, | Inflow Depth $=6.13 " \mathrm{for} 100-$ Year event |
| :--- | :--- | :--- | :--- |
| Inflow | $=$ | $8.58 \mathrm{cfs} @$ | 12.19 hrs , Volume $=$ |
| Outflow | $=$ | $8.58 \mathrm{cfs} @$ | 12.19 hrs , Volume $=$ |

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 2: Towards Offsite North


## Summary for Reach DP 3: Towards West Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

39,495 sf, 0.00\% Impervious, Inflow Depth = 6.74" for 100-Year event 7.17 cfs @ 12.14 hrs, Volume= 22,196 cf 7.17 cfs @ 12.14 hrs, Volume= 22,196 cf, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 3: Towards West Wetland


Summary for Reach DP 4: Towards East Wetland
[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
38,018 sf, 0.00\% Impervious, Inflow Depth = 6.38" for 100-Year event
Inflow =
6.27 cfs @ 12.15 hrs, Volume=

20,209 cf
Outflow =
6.27 cfs @ 12.15 hrs , Volume=
$20,209 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 4: Towards East Wetland


## Summary for Reach DP 5: Towards Offsite Northwest

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

61,964 sf, $0.00 \%$ Impervious, Inflow Depth $=6.01 "$ for 100-Year event 9.21 cfs @ 12.17 hrs, Volume= 31,049 cf 9.21 cfs @ 12.17 hrs, Volume= $31,049 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 5: Towards Offsite Northwest
Hydrograph


## Summary for Reach DP 6: Towards Offsite East

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

35,137 cf
35,137 cf, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 6: Towards Offsite East



## Area Listing (all nodes)

| Area <br> $(\mathrm{sq-ft})$ | CN | Description <br> (subcatchment-numbers) |
| ---: | :--- | :--- |
| 18,864 | 74 | $>75 \%$ Grass cover, Good, HSG C (6A) |
| 115,033 | 80 | $>75 \%$ Grass cover, Good, HSG D (1S, 2A, 5S, 6A) |
| 51,483 | 83 | Brush, Poor, HSG D (2S, 3S, 4S) |
| 1,726 | 91 | Gravel roads, HSG D (6A) |
| 24,722 | 96 | Gravel surface, HSG D (1S, 2A, 2S, 3S, 4S, 5S) |
| 1,109 | 98 | Paved parking, HSG D (1S) |
| 2,717 | 98 | Roofs, HSG D (1S, 2A) |
| 620 | 98 | Unconnected pavement, HSG D (6A) |
| 32,548 | 70 | Woods, Good, HSG C (4S, 5S, 6A, 6S) |
| 70,768 | 77 | Woods, Good, HSG D (1S, 2A, 2S, 3S, 4S, 5S, 6A, 6S) |
| $\mathbf{3 1 9 , 5 9 0}$ | $\mathbf{8 0}$ | TOTAL AREA |

### 3328.00-POST

Prepared by \{enter your company name here\}
Printed 6/15/2023
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## Soil Listing (all nodes)

| Area <br> $(\mathrm{sq-ft})$ | Soil <br> Group | Subcatchment <br> Numbers |
| ---: | :--- | :--- |
| 0 | HSG A |  |
| 0 | HSG B |  |
| 51,412 | HSG C | $4 \mathrm{~S}, 5 \mathrm{~S}, 6 \mathrm{~A}, 6 \mathrm{~S}$ |
| 268,178 | HSG D | 1S, 2A, 2S, 3S, 4S, 5S, 6A, 6S |
| 0 | Other |  |
| 319,590 |  | TOTAL AREA |

## Ground Covers (all nodes)

| $\begin{aligned} & \text { HSG-A } \\ & \text { (sq-ft) } \end{aligned}$ | $\begin{array}{r} \text { HSG-B } \\ (\mathrm{sq}-\mathrm{ft}) \end{array}$ | $\begin{array}{r} \text { HSG-C } \\ (\mathrm{sq}-\mathrm{ft}) \end{array}$ | $\begin{array}{r} \text { HSG-D } \\ (\mathrm{sq-ft}) \end{array}$ | Other (sq-ft) | Total (sq-ft) | Ground Cover |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 0 | 0 | 18,864 | 115,033 | 0 | 133,897 | >75\% Grass |
|  |  |  |  |  |  | cover, Good |
| 0 | 0 | 0 | 51,483 | 0 | 51,483 | Brush, Poor |
| 0 | 0 | 0 | 1,726 | 0 | 1,726 | Gravel roads |
| 0 | 0 | 0 | 24,722 | 0 | 24,722 | Gravel surface |
| 0 | 0 | 0 | 1,109 | 0 | 1,109 | Paved parking |
| 0 | 0 | 0 | 2,717 | 0 | 2,717 | Roofs |
| 0 | 0 | 0 | 620 | 0 | 620 | Unconnected pavement |
| 0 | 0 | 32,548 | 70,768 | 0 | 103,316 | Woods, Good |
| 0 | 0 | 51,412 | 268,178 | 0 | 319,590 | TOTAL AREA |

Time span=0.00-72.00 hrs, $\mathrm{dt}=0.01 \mathrm{hrs}, 7201$ points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

## Subcatchment1S:

## Subcatchment2A:

## Subcatchment2S:

## Subcatchment3S:

## Subcatchment4S:

## Subcatchment5S:

## Subcatchment6A:

## Subcatchment6S:

Reach DP 1: Towards Offsite West

Reach DP 2: Towards Offsite North

Reach DP 3: Towards West Wetland

Reach DP 4: Towards East Wetland

Reach DP 5: Towards Offsite Northwest

Reach DP 6: Towards Offsite East

Pond 2P: South West Basin

Pond 6P: North East Basin

Runoff Area=39,843 sf $7.57 \%$ Impervious Runoff Depth $=1.48^{\prime \prime}$ Flow Length=298' $\mathrm{Tc}=9.0 \mathrm{~min} \mathrm{CN}=81$ Runoff=1.54 cfs $4,930 \mathrm{cf}$

Runoff Area=43,949 sf $1.84 \%$ Impervious Runoff Depth=1.48" Flow Length=261' Tc=8.2 min CN=81 Runoff=1.75 cfs 5,438 cf

Runoff Area=21,866 sf $0.00 \%$ Impervious Runoff Depth $=1.48$ " Flow Length=225' Tc=10.8 min CN=81 Runoff=0.78 cfs 2,706 cf

Runoff Area=37,420 sf $0.00 \%$ Impervious Runoff Depth=1.70" Flow Length=212' $\mathrm{Tc}=6.7 \mathrm{~min} \quad \mathrm{CN}=84$ Runoff=1.80 cfs $5,298 \mathrm{cf}$

Runoff Area=37,744 sf $0.00 \%$ Impervious Runoff Depth=1.42" Flow Length=90' $\mathrm{Tc}=11.5 \mathrm{~min} \quad \mathrm{CN}=80$ Runoff $=1.25 \mathrm{cfs} 4,458 \mathrm{cf}$

Runoff Area=39,402 sf $0.00 \%$ Impervious Runoff Depth=1.48" Flow Length=109' Tc=6.0 min CN=81 Runoff=1.71 cfs 4,875 cf

Runoff Area=81,711 sf $0.76 \%$ Impervious Runoff Depth $=1.29$ " Flow Length=592' Tc=8.1 min CN=78 Runoff=2.80 cfs 8,771 cf

Runoff Area=17,655 sf $0.00 \%$ Impervious Runoff Depth $=0.84$ " Flow Length=213' $\mathrm{Tc}=6.2 \mathrm{~min} \quad \mathrm{CN}=70$ Runoff $=0.40 \mathrm{cfs} 1,235 \mathrm{cf}$

Inflow=1.54 cfs 4,930 cf Outflow=1.54 cfs 4,930 cf

Inflow=1.41 cfs 8,142 cf Outflow=1.41 cfs 8,142 cf

Inflow=1.80 cfs 5,298 cf Outflow=1.80 cfs 5,298 cf

Inflow=1.25 cfs $4,458 \mathrm{cf}$ Outflow=1.25 cfs 4,458 cf

Inflow=1.71 cfs 4,875 cf Outflow=1.71 cfs 4,875 cf

Inflow=1.59 cfs 10,006 cf Outflow $=1.59$ cfs 10,006 cf

Peak Elev=457.23' Storage=1,431 cf Inflow=1.75 cfs 5,438 cf 10.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=14.0$ ' $\mathrm{S}=0.0100$ '/' Outflow= $0.75 \mathrm{cfs} 5,436 \mathrm{cf}$

Peak Elev=402.11' Storage=1,298 cf Inflow=2.80 cfs 8,771 cf Outflow=1.39 cfs 8,771 cf

## Total Runoff Area $=319,590$ sf Runoff Volume $=37,711$ cf Average Runoff Depth $=1.42$ " <br> $\mathbf{9 8 . 6 1 \%}$ Pervious = 315,144 sf 1.39\% Impervious = 4,446 sf

## Summary for Subcatchment 1S:

Runoff $=\quad 1.54$ cfs @ 12.17 hrs, Volume $=\quad 4,930 \mathrm{cf}$, Depth= $1.48{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 13,399 | 77 V | Woods, Good, HSG D |  |  |
|  | 1,909 | 98 R | Roofs, HSG D |  |  |
|  | 20,826 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 2,600 | 96 | Gravel surface, HSG D |  |  |
|  | 1,109 | 98 P | Paved parking, HSG D |  |  |
|  | 39,843 | 81 | Weighted Average |  |  |
|  | 36,825 |  | 92.43\% Pervious Area |  |  |
|  | 3,018 |  | 7.57\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 4.1 | 50 | 0.0400 | 0.20 |  | Sheet Flow, Grass: Short $n=0.150 \quad \mathrm{P} 2=3.44{ }^{\prime \prime}$ |
| 0.2 | 14 | 0.0230 | - 1.06 |  | Shallow Concentrated Flow, |
| 1.5 | 65 | 0.0200 | 0.71 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 0.4 | 59 | 0.0200 | - 2.28 |  | Shallow Concentrated Flow, Unpaved Kv= 16.1 fps |
| 2.8 | 110 | 0.0170 | - 0.65 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |

9.0298 Total


## Summary for Subcatchment 2A:

Runoff $=\quad 1.75 \mathrm{cfs} @ 12.16 \mathrm{hrs}$, Volume $=\quad 5,438 \mathrm{cf}$, Depth= $1.48{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 17,342 |  |  |  |  |
|  | 808 |  | >75\% Grass cover, Good, HSG D Roofs, HSG D |  |  |
|  | 20,539 |  | Woods, Good, HSG D |  |  |
|  | 5,260 | 96 | Gravel surface, HSG D |  |  |
|  | 43,949 | 81 | Weighted Average 98.16\% Pervious Area 1.84\% Impervious Area |  |  |
|  | 43,141 |  |  |  |  |
|  | 808 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | $\begin{gathered} \text { Velocity } \\ (\mathrm{ft} / \mathrm{sec}) \end{gathered}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 4.7 | 50 | 0.0280 | 0.18 |  | Sheet Flow, Grass: Short $n=0.150 \quad \mathrm{P} 2=3.44{ }^{\prime \prime}$ |
| 0.2 | 12 | 0.0210 | - 1.01 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |
| 1.7 | 76 | 0.0210 | - 0.72 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 0.2 | 22 | 0.0200 | - 2.28 |  | Shallow Concentrated Flow, Unpaved Kv= 16.1 fps |
| 1.4 | 101 | 0.0300 | - 1.21 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |

$8.2 \quad 261$ Total


## Summary for Subcatchment 2S:

Runoff $=\quad 0.78 \mathrm{cfs} @ 12.18 \mathrm{hrs}$, Volume $=\quad 2,706 \mathrm{cf}$, Depth= $1.48{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"


## Subcatchment 2S:



## Summary for Subcatchment 3S:

Runoff $=1.80$ cfs @ 12.14 hrs, Volume $=\quad 5,298 \mathrm{cf}$, Depth= $1.70^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"


## Subcatchment 3S:


$\square$ Runoff

## Summary for Subcatchment 4S:

Runoff $=\quad 1.25$ cfs @ 12.20 hrs, Volume $=\quad 4,458 \mathrm{cf}$, Depth= $1.42^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

| Area (sf) | CN | Description |  |
| ---: | ---: | :--- | :--- |
| 14,923 | 83 | Brush, Poor, HSG D |  |
| 4,794 | 96 | Gravel surface, HSG D |  |
| 10,680 | 77 | Woods, Good, HSG D |  |
| 7,347 | 70 | Woods, Good, HSG C |  |

## Subcatchment 4S:

Hydrograph


## Summary for Subcatchment 5S:

Runoff $=\quad 1.71$ cfs @ 12.13 hrs, Volume $=\quad 4,875 \mathrm{cf}$, Depth= $1.48^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"


## Subcatchment 5S:


$\square$ Runoff

## Summary for Subcatchment 6A:

Runoff $=\quad 2.80$ cfs @ 12.16 hrs, Volume $=\quad 8,771 \mathrm{cf}$, Depth= $1.29{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 2-Year Rainfall=3.22"



## Summary for Subcatchment 6S:

Runoff $=\quad 0.40$ cfs @ 12.14 hrs, Volume $=\quad 1,235 \mathrm{cf}$, Depth= $0.84{ }^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 2-Year Rainfall=3.22"

|  | rea (sf) | CN D | escription |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{array}{r} 17,291 \\ 364 \\ \hline \end{array}$ | $\begin{aligned} & 70 \\ & 77 \\ & \hline \end{aligned}$ | Woods, Good, HSG C Woods, Good, HSG D |  |  |  |
|  | $\begin{aligned} & \hline 17,655 \\ & 17,655 \end{aligned}$ | 70 | Weighted Average 100.00\% Pervious Area |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |  |
| 5.1 | 50 | 0.1670 | 0.16 |  | Sheet Flow, <br> Woods: Light underbrush $n=0.400$ | P2=3.44" |
| 0.5 | 88 | 0.1670 | 2.86 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |  |
| 0.2 | 21 | 0.2130 | 2.31 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |  |
| 0.4 | 54 | 0.0935 | 2.14 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |  |
| 6.2 | 213 | Total |  |  |  |  |

## Subcatchment 6S:



## Summary for Reach DP 1: Towards Offsite West

[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
39,843 sf, 7.57\% Impervious, Inflow Depth = 1.48" for 2-Year event
Outflow =
1.54 cfs @ 12.17 hrs, Volume= 4,930 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 1: Towards Offsite West



Summary for Reach DP 2: Towards Offsite North
[40] Hint: Not Described (Outflow=Inflow)

| Inflow Area | 65,815 sf, | 1.23\% Impervious, | Depth $=1.48{ }^{\prime \prime}$ for $2-Y$ Year event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.41 cfs @ | 12.21 hrs , Volume= | 8,142 cf |
| Outflow | 1.41 cfs @ | 12.21 hrs , Volume= | 8,142 cf, Atten=0\%, Lag= 0.0 m |

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 2: Towards Offsite North


## Summary for Reach DP 3: Towards West Wetland

[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
37,420 sf, 0.00\% Impervious, Inflow Depth = 1.70" for 2-Year event
Outflow =
1.80 cfs @ 12.14 hrs , Volume=

5,298 cf
$5,298 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 3: Towards West Wetland


Summary for Reach DP 4: Towards East Wetland
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

37,744 sf, 0.00\% Impervious, Inflow Depth = 1.42" for 2-Year event

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 4: Towards East Wetland


## Summary for Reach DP 5: Towards Offsite Northwest

[40] Hint: Not Described (Outflow=Inflow)

| Inflow Area $=$ | $39,402 \mathrm{sf}$, | $0.00 \%$ Impervious, | Inflow Depth $=1.48 "$ for $2-$ Year event |  |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $1.71 \mathrm{cfs} @$ | 12.13 hrs , Volume $=$ | $4,875 \mathrm{cf}$ |
| Outflow | $=$ | $1.71 \mathrm{cfs} @$ | 12.13 hrs , Volume $=$ | $4,875 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 5: Towards Offsite Northwest



## Summary for Reach DP 6: Towards Offsite East

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

99,366 sf, 0.62\% Impervious, Inflow Depth = 1.21" for 2-Year event 1.59 cfs @ 12.19 hrs, Volume= 10,006 cf 1.59 cfs @ 12.19 hrs, Volume= 10,006 cf, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 6: Towards Offsite East


## Summary for Pond 2P: South West Basin

| Inflow Area = | 43,949 sf, | 1.84\% Impervious, | Inflow Depth = 1.48" for 2 -Year event |
| :---: | :---: | :---: | :---: |
| Inflow | 1.75 cfs @ | 12.16 hrs, Volume= | 5,438 cf |
| Outflow | 0.75 cfs @ | 12.32 hrs , Volume= | 5,436 cf, Atten= 57\%, Lag= 9.6 min |
| Primary | 0.75 cfs @ | 12.32 hrs , Volume= | 5,436 cf |

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Peak Elev= 457.23' @ 12.32 hrs Surf.Area= 2,919 sf Storage= 1,431 cf
Plug-Flow detention time $=76.1 \mathrm{~min}$ calculated for $5,436 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time $=75.9 \mathrm{~min}(928.0-852.1)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $456.70^{\prime}$ | 6,701 cf | Custom Stage Data (Irregular)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Perim. <br> (feet) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> (sq-ft) |
| ---: | ---: | ---: | ---: | ---: | ---: |
| 456.70 | 2,487 | 270.0 | 0 | 0 | 2,487 |
| 458.70 | 4,296 | 320.0 | 6,701 | 6,701 | 4,908 |



Primary OutFlow Max=0.75 cfs @ 12.32 hrs HW=457.23' (Free Discharge)
——ADDS Round 10" (Barrel Controls 0.75 cfs @ 2.90 fps )

## Pond 2P: South West Basin



## Summary for Pond 6P: North East Basin

| Inflow Area = | 81,711 sf, | \% Impervious, | Inflow Depth = 1.29" for 2 -Year event |
| :---: | :---: | :---: | :---: |
| Inflow | 2.80 cfs @ | 12.16 hrs , Volume= | 8,771 cf |
| Outflow | 1.39 cfs @ | 12.29 hrs , Volume= | $8,771 \mathrm{cf}$, Atten $=50 \%$ Lag $=7.9 \mathrm{~min}$ |
| Primary | 1.39 cfs @ | 12.29 hrs , Volume= | 8,771 cf |

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Peak Elev= 402.11' @ 12.29 hrs Surf.Area= 1,994 sf Storage= 1,298 cf
Plug-Flow detention time $=14.6 \mathrm{~min}$ calculated for $8,769 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time $=14.6 \min (877.1-862.5)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | 401.00 | $12,521 \mathrm{cf}$ | Custom Stage Data (Irregular)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Perim. <br> (feet) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> (sq-ft) |
| ---: | ---: | ---: | ---: | ---: | ---: |
| 401.00 | 454 | 159.0 | 0 | 0 | 454 |
| 402.00 | 1,868 | 254.0 | 1,081 | 1,081 | 3,583 |
| 403.00 | 3,130 | 251.0 | 2,472 | 3,553 | 3,863 |
| 404.00 | 4,462 | 289.0 | 3,776 | 7,329 | 5,518 |
| 405.00 | 5,957 | 326.0 | 5,192 | 12,521 | 7,355 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 401.00' | 8.0" Round ADS_Round 8" |
|  |  |  | $\mathrm{L}=22.0^{\prime} \mathrm{CMP}$, end-section conforming to fill, $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 401.00' / 400.78' S=0.0100 '/' Cc= 0.900 |
| \#2 | Primary | 404.00' | n=0.013, Flow Area=0.35 si ${ }^{\prime}$ |
|  |  |  | 8.0' long x 4.0' breadth Broad-Crested Rectangular Weir |
|  |  |  | $\begin{array}{llllllllllll}\text { Head (feet) } 0.20 & 0.40 & 0.60 & 0.80 & 1.00 & 1.20 & 1.40 & 1.60 & 1.80 & 2.00\end{array}$ |
|  |  |  | $\begin{array}{llllllllll}2.50 & 3.00 & 3.50 & 4.00 & 4.50 & 5.00 & 5.50\end{array}$ |
|  |  |  | Coef. (English) 2.382 .542 .69 2.68 2.672 .6712 .6512 .662 .66 |
|  |  |  |  |

Primary OutFlow Max=1.39 cfs @ 12.29 hrs HW=402.11' (Free Discharge)
-1=ADS_Round 8" (Barrel Controls $1.39 \mathrm{cfs} @ 4.00 \mathrm{fps}$ )
-2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

Pond 6P: North East Basin


Time span $=0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}, 7201$ points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

## Subcatchment1S:

## Subcatchment2A:

## Subcatchment2S:

## Subcatchment3S:

## Subcatchment4S:

## Subcatchment5S:

## Subcatchment6A:

## Subcatchment6S:

Reach DP 1: Towards Offsite West

Reach DP 2: Towards Offsite North

## Reach DP 3: Towards West Wetland

## Reach DP 4: Towards East Wetland

Reach DP 5: Towards Offsite Northwest

Reach DP 6: Towards Offsite East

Runoff Area=39,843 sf $7.57 \%$ Impervious Runoff Depth=2.86" Flow Length=298' $\mathrm{Tc}=9.0 \mathrm{~min} \mathrm{CN}=81$ Runoff $=2.95 \mathrm{cfs} 9,502 \mathrm{cf}$

Runoff Area=43,949 sf $1.84 \%$ Impervious Runoff Depth=2.86" Flow Length=261' Tc=8.2 min CN=81 Runoff=3.35 cfs $10,482 \mathrm{cf}$

Runoff Area=21,866 sf $0.00 \%$ Impervious Runoff Depth=2.86" Flow Length=225' Tc=10.8 min CN=81 Runoff=1.51 cfs $5,215 \mathrm{cf}$

Runoff Area=37,420 sf $0.00 \%$ Impervious Runoff Depth=3.14" Flow Length=212' $\mathrm{Tc}=6.7 \mathrm{~min} \quad \mathrm{CN}=84$ Runoff=3.28 cfs 9,800 cf

Runoff Area=37,744 sf $0.00 \%$ Impervious Runoff Depth=2.77" Flow Length=90' $\mathrm{Tc}=11.5 \mathrm{~min} \mathrm{CN}=80$ Runoff $=2.45 \mathrm{cfs} 8,716 \mathrm{cf}$

Runoff Area=39,402 sf $0.00 \%$ Impervious Runoff Depth=2.86" Flow Length=109' Tc=6.0 min CN=81 Runoff=3.27 cfs 9,397 cf

Runoff Area=81,711 sf $0.76 \%$ Impervious Runoff Depth=2.59" Flow Length=592' Tc=8.1 min CN=78 Runoff=5.67 cfs $17,658 \mathrm{cf}$

Runoff Area=17,655 sf $0.00 \%$ Impervious Runoff Depth $=1.93$ " Flow Length=213' $\mathrm{Tc}=6.2 \mathrm{~min} \quad \mathrm{CN}=70$ Runoff $=0.98 \mathrm{cfs} 2,844 \mathrm{cf}$

Inflow=2.95 cfs 9,502 cf Outflow=2.95 cfs 9,502 cf

Inflow=2.95 cfs 15,695 cf Outflow=2.95 cfs 15,695 cf

Inflow=3.28 cfs 9,800 cf Outflow=3.28 cfs 9,800 cf

Inflow=2.45 cfs 8,716 cf Outflow=2.45 cfs 8,716 cf

Inflow=3.27 cfs 9,397 cf Outflow=3.27 cfs 9,397 cf

Inflow=2.73 cfs 20,502 cf Outflow=2.73 cfs 20,502 cf

Pond 6P: North East Basin

## Pond 2P: South West Basin

 10.0" Round Culvert $\mathrm{n}=0.013 \mathrm{~L}=14.0$ ' $\mathrm{S}=0.0100$ '/' Outflow=1.60 cfs $10,480 \mathrm{cf}$Peak Elev=402.95' Storage=3,385 cf Inflow=5.67 cfs 17,658 cf Outflow=2.09 cfs 17,658 cf

## Total Runoff Area $=319,590$ sf Runoff Volume $=73,614$ cf Average Runoff Depth $=2.76$ " <br> $\mathbf{9 8 . 6 1 \%}$ Pervious = 315,144 sf 1.39\% Impervious $=\mathbf{4 , 4 4 6}$ sf

## Summary for Subcatchment 1S:

Runoff $=\quad 2.95$ cfs @ 12.16 hrs, Volume $=\quad 9,502 \mathrm{cf}$, Depth= 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 10-Year Rainfall=4.86"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 13,399 | 77 V | Woods, Good, HSG D |  |  |
|  | 1,909 | 98 R | Roofs, HSG D |  |  |
|  | 20,826 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 2,600 | 96 | Gravel surface, HSG D |  |  |
|  | 1,109 | 98 P | Paved parking, HSG D |  |  |
|  | 39,843 | 81 | Weighted Average |  |  |
|  | 36,825 |  | 92.43\% Pervious Area |  |  |
|  | 3,018 |  | 7.57\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 4.1 | 50 | 0.0400 | 0.20 |  | Sheet Flow, Grass: Short $n=0.150 \quad \mathrm{P} 2=3.44{ }^{\prime \prime}$ |
| 0.2 | 14 | 0.0230 | - 1.06 |  | Shallow Concentrated Flow, |
| 1.5 | 65 | 0.0200 | 0.71 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 0.4 | 59 | 0.0200 | - 2.28 |  | Shallow Concentrated Flow, Unpaved Kv= 16.1 fps |
| 2.8 | 110 | 0.0170 | - 0.65 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |

9.0298 Total


## Summary for Subcatchment 2A:

Runoff $=3.35$ cfs @ 12.16 hrs, Volume $=10,482 \mathrm{cf}$, Depth= 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 10-Year Rainfall=4.86"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 17,342 |  |  |  |  |
|  | 808 |  | >75\% Grass cover, Good, HSG D Roofs, HSG D |  |  |
|  | 20,539 |  | Woods, Good, HSG D |  |  |
|  | 5,260 | 96 | Gravel surface, HSG D |  |  |
|  | 43,949 | 81 | Weighted Average 98.16\% Pervious Area 1.84\% Impervious Area |  |  |
|  | 43,141 |  |  |  |  |
|  | 808 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | $\begin{gathered} \text { Velocity } \\ (\mathrm{ft} / \mathrm{sec}) \end{gathered}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 4.7 | 50 | 0.0280 | 0.18 |  | Sheet Flow, Grass: Short $n=0.150 \quad \mathrm{P} 2=3.44{ }^{\prime \prime}$ |
| 0.2 | 12 | 0.0210 | - 1.01 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |
| 1.7 | 76 | 0.0210 | - 0.72 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 0.2 | 22 | 0.0200 | - 2.28 |  | Shallow Concentrated Flow, Unpaved Kv= 16.1 fps |
| 1.4 | 101 | 0.0300 | - 1.21 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |

$8.2 \quad 261$ Total


## Summary for Subcatchment 2S:

Runoff $=\quad 1.51$ cfs @ 12.18 hrs, Volume $=\quad 5,215 \mathrm{cf}$, Depth= 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 10-Year Rainfall=4.86"
$\left.\begin{array}{rrrl}\text { Area (sf) } & \text { CN } & \text { Description } \\ 12,429 & 77 & \text { Woods, Good, HSG D } \\ 2,697 & 96 & \begin{array}{l}\text { Gravel surface, HSG D } \\ 6,740\end{array} & 83\end{array} \begin{array}{rl}\text { Brush, Poor, HSG D }\end{array}\right]$

## Subcatchment 2S:



## Summary for Subcatchment 3S:

Runoff $=\quad 3.28 \mathrm{cfs} @ 12.14 \mathrm{hrs}$, Volume $=\quad 9,800 \mathrm{cf}$, Depth= 3.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 10-Year Rainfall=4.86"


## Subcatchment 3S:


$\square$ Runoff

## Summary for Subcatchment 4S:

Runoff $=\quad 2.45$ cfs @ 12.19 hrs, Volume $=\quad 8,716 \mathrm{cf}$, Depth= 2.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 10-Year Rainfall=4.86"

| Area (sf) | CN | Description |  |
| ---: | ---: | :--- | :--- |
| 14,923 | 83 | Brush, Poor, HSG D |  |
| 4,794 | 96 | Gravel surface, HSG D |  |
| 10,680 | 77 | Woods, Good, HSG D |  |
| 7,347 | 70 | Woods, Good, HSG C |  |

Subcatchment 4S:
Hydrograph


## Summary for Subcatchment 5S:

Runoff $=3.27$ cfs @ 12.13 hrs, Volume $=\quad 9,397 \mathrm{cf}$, Depth= 2.86"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 10-Year Rainfall=4.86"


## Subcatchment 5S:


$\square$ Runoff

## Summary for Subcatchment 6A:

Runoff $=5.67$ cfs @ 12.15 hrs, Volume $=17,658 \mathrm{cf}$, Depth= 2.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 10-Year Rainfall=4.86"



## Summary for Subcatchment 6S:

Runoff $=\quad 0.98$ cfs @ 12.14 hrs, Volume $=\quad 2,844 \mathrm{cf}$, Depth= $1.93^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 10-Year Rainfall=4.86"

| Area (sf) |  | CN | Description |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\begin{array}{r} 17,291 \\ 364 \\ \hline \end{array}$ | $\begin{aligned} & \hline 70 \\ & 77 \end{aligned}$ | oods, Go oods, Go | d, HSG C <br> d, HSG D |  |  |
|  | $\begin{aligned} & 17,655 \\ & 17,655 \end{aligned}$ | 70 | eighted $00.00 \% \text { F }$ | verage <br> rvious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |  |
| 5.1 | 50 | 0.1670 | 0.16 |  | Sheet Flow, <br> Woods: Light underbrush $n=0.400$ | $\mathrm{P} 2=3.44{ }^{\prime \prime}$ |
| 0.5 | 88 | 0.1670 | 2.86 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |  |
| 0.2 | 21 | 0.2130 | 2.31 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |  |
| 0.4 | 54 | 0.0935 | 2.14 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |  |

## Subcatchment 6S:


$\square$ Runoff

## Summary for Reach DP 1: Towards Offsite West

[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
39,843 sf, 7.57\% Impervious, Inflow Depth = 2.86" for 10-Year event
Outflow =
2.95 cfs @ 12.16 hrs, Volume=

9,502 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 1: Towards Offsite West



Summary for Reach DP 2: Towards Offsite North
[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
$65,815 \mathrm{sf}$, 1.23\% Impervious, Inflow Depth = 2.86" for 10-Year event
Outflow =
2.95 cfs @ 12.20 hrs , Volume= 15,695 cf
$15,695 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 2: Towards Offsite North


## Summary for Reach DP 3: Towards West Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

37,420 sf, 0.00\% Impervious, Inflow Depth = 3.14" for 10-Year event

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 3: Towards West Wetland


Summary for Reach DP 4: Towards East Wetland
[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
37,744 sf, 0.00\% Impervious, Inflow Depth = 2.77" for 10-Year event
Outflow =
2.45 cfs @ 12.19 hrs , Volume=

8,716 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 4: Towards East Wetland


## Summary for Reach DP 5: Towards Offsite Northwest

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = Inflow = Outflow =

39,402 sf, 0.00\% Impervious, Inflow Depth = 2.86" for 10-Year event 3.27 cfs @ 12.13 hrs, Volume= 9,397 cf
3.27 cfs @ 12.13 hrs , Volume=

9,397 cf, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 5: Towards Offsite Northwest



## Summary for Reach DP 6: Towards Offsite East

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = Inflow = Outflow =

99,366 sf, 0.62\% Impervious, Inflow Depth = 2.48" for 10-Year event
2.73 cfs @ 12.16 hrs, Volume=
2.73 cfs @ 12.16 hrs, Volume=

20,502 cf
20,502 cf, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 6: Towards Offsite East


## Summary for Pond 2P: South West Basin

| Inflow Area = | 43,949 sf, | 1.84\% Impervious, | Inflow Depth = 2.86" for 10-Year event |
| :---: | :---: | :---: | :---: |
| Inflow | 3.35 cfs @ | 12.16 hrs , Volume= | 10,482 cf |
| Outflow | 1.60 cfs @ | 12.29 hrs , Volume= | 10,480 cf, Atten= 52\%, Lag= 7.9 min |
| Primary | 1.60 cfs @ | 12.29 hrs , Volume= | 10,480 cf |

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Peak Elev= 457.58' @ 12.29 hrs Surf.Area=3,222 sf Storage= 2,504 cf
Plug-Flow detention time $=56.5 \mathrm{~min}$ calculated for $10,480 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time $=56.4 \mathrm{~min}$ ( 887.6-831.2)

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | 456.70 | $6,701 \mathrm{cf}$ | Custom Stage Data (Irregular)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Perim. <br> (feet) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> (sq-ft) |
| ---: | ---: | ---: | ---: | ---: | ---: |
| 456.70 | 2,487 | 270.0 | 0 | 0 | 2,487 |
| 458.70 | 4,296 | 320.0 | 6,701 | 6,701 | 4,908 |



Primary OutFlow Max=1.60 cfs @ 12.29 hrs HW=457.58' (Free Discharge)
—1=ADS Round 10" (Barrel Controls 1.60 cfs @ 3.46 fps )

## Pond 2P: South West Basin



## $\square$ Inflow

 $\square$ Primary
## Summary for Pond 6P: North East Basin

| Inflow Area $=$ | $81,711 \mathrm{sf}$, | $0.76 \%$ Impervious, | Inflow Depth $=2.59 "$ | for $10-$ Year event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $5.67 \mathrm{cfs} @$ | 12.15 hrs , Volume $=$ | $17,658 \mathrm{cf}$ |
| Outflow | $=$ | $2.09 \mathrm{cfs} @$ | 12.34 hrs , Volume $=$ | $17,658 \mathrm{cf}$, Atten $=63 \%$, Lag= $=11.3 \mathrm{~min}$ |
| Primary | $=$ | $2.09 \mathrm{cfs} @$ | 12.34 hrs , Volume $=$ | $17,658 \mathrm{cf}$ |

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Peak Elev= 402.95' @ 12.34 hrs Surf.Area=3,053 sf Storage= 3,385 cf
Plug-Flow detention time $=16.9$ min calculated for 17,656 cf ( $100 \%$ of inflow)
Center-of-Mass det. time $=16.9 \mathrm{~min}(857.0-840.1)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | 401.00 | 12,521 cf | Custom Stage Data (Irregular)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Perim. <br> (feet) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> $($ sq-ft) |
| ---: | ---: | ---: | ---: | ---: | ---: |
| 401.00 | 454 | 159.0 | 0 | 0 | 454 |
| 402.00 | 1,868 | 254.0 | 1,081 | 1,081 | 3,583 |
| 403.00 | 3,130 | 251.0 | 2,472 | 3,553 | 3,863 |
| 404.00 | 4,462 | 289.0 | 3,776 | 7,329 | 5,518 |
| 405.00 | 5,957 | 326.0 | 5,192 | 12,521 | 7,355 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 401.00' | 8.0" Round ADS_Round 8" |
|  |  |  | $\mathrm{L}=22.0{ }^{\prime}$ CMP, end-section conforming to fill, $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 401.00' / 400.78' S=0.0100 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$, Flow Area $=0.35 \mathrm{sf}$ |
| \#2 | Primary | 404.00' | 8.0' long x 4.0' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .001 .201 .401 .601 .80 |
|  |  |  | 2.503 .003 .504 .004 .505 .005 .50 |
|  |  |  | Coef. (English) 2.382 .542 .692 .682 .672 .6712 .6512 .662 .66 |
|  |  |  | 2.68 2.72 2.732 .762 .792 .883 .073 .32 |

Primary OutFlow Max=2.09 cfs @ 12.34 hrs HW=402.95' (Free Discharge)
中-1=ADS_Round 8" (Barrel Controls 2.09 cfs @ 5.99 fps )
-2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

Pond 6P: North East Basin


Time span $=0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}, 7201$ points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

## Subcatchment1S:

## Subcatchment2A:

## Subcatchment2S:

## Subcatchment3S:

## Subcatchment4S:

## Subcatchment5S:

## Subcatchment6A:

## Subcatchment6S:

Reach DP 1: Towards Offsite West

Reach DP 2: Towards Offsite North

Reach DP 3: Towards West Wetland

Reach DP 4: Towards East Wetland

Reach DP 5: Towards Offsite Northwest

Reach DP 6: Towards Offsite East

Pond 2P: South West Basin

Pond 6P: North East Basin

Runoff Area=39,843 sf $7.57 \%$ Impervious Runoff Depth=4.02" Flow Length=298' Tc=9.0 min CN=81 Runoff=4.10 cfs $13,350 \mathrm{cf}$

Runoff Area=43,949 sf $1.84 \%$ Impervious Runoff Depth=4.02" Flow Length=261' Tc=8.2 min CN=81 Runoff=4.65 cfs $14,725 \mathrm{cf}$

Runoff Area=21,866 sf $0.00 \%$ Impervious Runoff Depth=4.02" Flow Length=225' Tc=10.8 min CN=81 Runoff=2.10 cfs 7,326 cf

Runoff Area=37,420 sf $0.00 \%$ Impervious Runoff Depth=4.34" Flow Length=212' Tc=6.7 min CN=84 Runoff=4.46 cfs $13,524 \mathrm{cf}$

Runoff Area=37,744 sf $0.00 \%$ Impervious Runoff Depth=3.92" Flow Length=90' Tc=11.5 min CN=80 Runoff=3.43 cfs $12,320 \mathrm{cf}$

Runoff Area=39,402 sf $0.00 \%$ Impervious Runoff Depth=4.02" Flow Length=109' Tc=6.0 min CN=81 Runoff=4.53 cfs $13,202 \mathrm{cf}$

Runoff Area=81,711 sf $0.76 \%$ Impervious Runoff Depth=3.71" Flow Length=592' Tc=8.1 min CN=78 Runoff=8.06 cfs $25,274 \mathrm{cf}$

Runoff Area=17,655 sf $0.00 \%$ Impervious Runoff Depth=2.92" Flow Length=213' $\mathrm{Tc}=6.2 \mathrm{~min} \quad \mathrm{CN}=70$ Runoff $=1.50 \mathrm{cfs} 4,303 \mathrm{cf}$

Inflow=4.10 cfs $13,350 \mathrm{cf}$ Outflow=4.10 cfs 13,350 cf

Inflow=4.03 cfs 22,050 cf Outflow=4.03 cfs 22,050 cf

Inflow=4.46 cfs 13,524 cf Outflow=4.46 cfs 13,524 cf

Inflow=3.43 cfs $12,320 \mathrm{cf}$ Outflow=3.43 cfs 12,320 cf

Inflow=4.53 cfs $13,202 \mathrm{cf}$ Outflow=4.53 cfs 13,202 cf

Inflow=3.58 cfs 29,577 cf Outflow=3.58 cfs 29,577 cf

Peak Elev=457.86' Storage=3,430 cf Inflow=4.65 cfs 14,725 cf 10.0" Round Culvert n=0.013 L=14.0' $\mathrm{S}=0.0100$ '/' Outflow=2.07 cfs $14,724 \mathrm{cf}$

Peak Elev=403.56' Storage=5,517 cf Inflow=8.06 cfs 25,274 cf Outflow=2.49 cfs 25,274 cf

$$
\begin{aligned}
\text { Total Runoff Area }=319,590 \text { sf } \begin{aligned}
\text { Runoff Volume } & =104,025 \mathrm{cf}
\end{aligned} \quad \text { Average Runoff Depth }=3.91 " \\
98.61 \% \text { Pervious }=315,144 \mathrm{sf} \quad 1.39 \% \text { Impervious }=4,446 \mathrm{sf}
\end{aligned}
$$

## Summary for Subcatchment 1S:

Runoff $=\quad 4.10$ cfs @ 12.16 hrs, Volume $=\quad 13,350 \mathrm{cf}$, Depth= 4.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 25-Year Rainfall=6.15"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 13,399 | 77 V | Woods, Good, HSG D |  |  |
|  | 1,909 | 98 R | Roofs, HSG D |  |  |
|  | 20,826 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 2,600 | 96 | Gravel surface, HSG D |  |  |
|  | 1,109 | 98 P | Paved parking, HSG D |  |  |
|  | 39,843 | 81 | Weighted Average |  |  |
|  | 36,825 |  | 92.43\% Pervious Area |  |  |
|  | 3,018 |  | 7.57\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 4.1 | 50 | 0.0400 | 0.20 |  | Sheet Flow, Grass: Short $n=0.150 \quad \mathrm{P} 2=3.44{ }^{\prime \prime}$ |
| 0.2 | 14 | 0.0230 | - 1.06 |  | Shallow Concentrated Flow, |
| 1.5 | 65 | 0.0200 | 0.71 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 0.4 | 59 | 0.0200 | - 2.28 |  | Shallow Concentrated Flow, Unpaved Kv= 16.1 fps |
| 2.8 | 110 | 0.0170 | - 0.65 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |

9.0298 Total


## Summary for Subcatchment 2A:

Runoff $=\quad 4.65$ cfs @ 12.15 hrs, Volume $=\quad 14,725$ cf, Depth= 4.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 25-Year Rainfall=6.15"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 17,342 |  |  |  |  |
|  | 808 |  | >75\% Grass cover, Good, HSG D Roofs, HSG D |  |  |
|  | 20,539 |  | Woods, Good, HSG D |  |  |
|  | 5,260 | 96 | Gravel surface, HSG D |  |  |
|  | 43,949 | 81 | Weighted Average 98.16\% Pervious Area 1.84\% Impervious Area |  |  |
|  | 43,141 |  |  |  |  |
|  | 808 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | $\begin{gathered} \text { Velocity } \\ (\mathrm{ft} / \mathrm{sec}) \end{gathered}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 4.7 | 50 | 0.0280 | 0.18 |  | Sheet Flow, Grass: Short $n=0.150 \quad \mathrm{P} 2=3.44{ }^{\prime \prime}$ |
| 0.2 | 12 | 0.0210 | - 1.01 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |
| 1.7 | 76 | 0.0210 | - 0.72 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 0.2 | 22 | 0.0200 | - 2.28 |  | Shallow Concentrated Flow, Unpaved Kv= 16.1 fps |
| 1.4 | 101 | 0.0300 | - 1.21 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |

$8.2 \quad 261$ Total


## Summary for Subcatchment 2S:

Runoff $=\quad 2.10 \mathrm{cfs} @ 12.18 \mathrm{hrs}$, Volume= $\quad 7,326 \mathrm{cf}$, Depth= 4.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 25-Year Rainfall=6.15"


## Subcatchment 2S:



## Summary for Subcatchment 3S:

Runoff $=\quad 4.46$ cfs @ 12.14 hrs, Volume $=\quad 13,524 \mathrm{cf}$, Depth= $4.34^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 25-Year Rainfall=6.15"


## Subcatchment 3S:


$\square$ Runoff

## Summary for Subcatchment 4S:

Runoff $=\quad 3.43$ cfs @ 12.19 hrs, Volume $=\quad 12,320 \mathrm{cf}$, Depth= 3.92"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 25-Year Rainfall=6.15"

| Area (sf) | CN | Description |  |
| ---: | ---: | :--- | :--- |
| 14,923 | 83 | Brush, Poor, HSG D |  |
| 4,794 | 96 | Gravel surface, HSG D |  |
| 10,680 | 77 | Woods, Good, HSG D |  |
| 7,347 | 70 | Woods, Good, HSG C |  |

Subcatchment 4S:
Hydrograph


## Summary for Subcatchment 5S:

Runoff $=\quad 4.53$ cfs @ 12.13 hrs, Volume $=13,202 \mathrm{cf}$, Depth= 4.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 25-Year Rainfall=6.15"


## Subcatchment 5S:


$\square$ Runoff

## Summary for Subcatchment 6A:

Runoff $=8.06$ cfs @ 12.15 hrs, Volume $=\quad 25,274 \mathrm{cf}$, Depth= 3.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 25-Year Rainfall=6.15"



## Summary for Subcatchment 6S:

Runoff $=\quad 1.50$ cfs @ 12.14 hrs, Volume $=\quad 4,303 \mathrm{cf}$, Depth= 2.92"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 25-Year Rainfall=6.15"

| Area (sf) | CN | Description |  |
| ---: | ---: | ---: | :--- |
| 17,291 | 70 | Woods, Good, HSG C |  |
| 364 | 77 | Woods, Good, HSG D |  |

## Subcatchment 6S:


$\square$ Runoff

## Summary for Reach DP 1: Towards Offsite West

[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
39,843 sf, 7.57\% Impervious, Inflow Depth = 4.02" for 25 -Year event
Outflow =
4.10 cfs @ 12.16 hrs, Volume=

13,350 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 1: Towards Offsite West


Summary for Reach DP 2: Towards Offsite North
[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
65,815 sf, $1.23 \%$ Impervious, Inflow Depth = 4.02" for 25 -Year event
Outflow =
4.03 cfs @ 12.19 hrs, Volume= 22,050 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 2: Towards Offsite North



## Summary for Reach DP 3: Towards West Wetland

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

37,420 sf, $0.00 \%$ Impervious, Inflow Depth = 4.34" for 25 -Year event

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 3: Towards West Wetland

Hydrograph


Summary for Reach DP 4: Towards East Wetland
[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

37,744 sf, 0.00\% Impervious, Inflow Depth = 3.92" for 25 -Year event 3.43 cfs @ 12.19 hrs, Volume= 12,320 cf
3.43 cfs @ 12.19 hrs, Volume=
$12,320 \mathrm{cf}$, Atten $=0 \%, \operatorname{Lag}=0.0 \mathrm{~min}$
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 4: Towards East Wetland


## Summary for Reach DP 5: Towards Offsite Northwest

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = Inflow = Outflow =

39,402 sf, 0.00\% Impervious, Inflow Depth = 4.02" for 25 -Year event 4.53 cfs @ 12.13 hrs, Volume= $13,202 \mathrm{cf}$

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 5: Towards Offsite Northwest

Hydrograph


## Summary for Reach DP 6: Towards Offsite East

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

99,366 sf, 0.62\% Impervious, Inflow Depth = 3.57" for 25 -Year event
3.58 cfs @ 12.15 hrs, Volume=

29,577 cf
29,577 cf, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 6: Towards Offsite East


## Summary for Pond 2P: South West Basin

| Inflow Area = | 43,949 sf, | 1.84\% Impervious, | Inflow Depth = 4.02" for $25-$ Year event |
| :---: | :---: | :---: | :---: |
| Inflow | 4.65 cfs @ | 12.15 hrs , Volume= | 14,725 cf |
| Outflow | 2.07 cfs @ | 12.30 hrs , Volume= | 14,724 cf, Atten= 55\%, Lag= 8.6 min |
| Primary | 2.07 cfs @ | 12.30 hrs , Volume= | 14,724 cf |

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Peak Elev= 457.86' @ 12.30 hrs Surf.Area= 3,473 sf Storage= 3,430 cf
Plug-Flow detention time $=49.8 \mathrm{~min}$ calculated for $14,724 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time $=49.7 \mathrm{~min}(870.2-820.5)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | 456.70 | $6,701 \mathrm{cf}$ | Custom Stage Data (Irregular)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Perim. <br> (feet) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> $(\mathrm{sq}-\mathrm{ft})$ |
| ---: | ---: | ---: | ---: | ---: | ---: |
| 456.70 | 2,487 | 270.0 | 0 | 0 | 2,487 |
| 458.70 | 4,296 | 320.0 | 6,701 | 6,701 | 4,908 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 456.70' | 10.0" Round ADS Round 10" <br> $\mathrm{L}=14.0^{\prime}$ CPP, end-section conforming to fill, $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 456.70' / 456.56' S=0.0100 '//' Cc= 0.900 $\mathrm{n}=0.013$. Flow Area $=0.55 \mathrm{sf}$ |

Primary OutFlow Max=2.07 cfs @ 12.30 hrs HW=457.86' (Free Discharge)
——ADDS Round 10" (Barrel Controls 2.07 cfs @ 3.80 fps )

## Pond 2P: South West Basin



## $\square$ Inflow

 $\square$ Primary
## Summary for Pond 6P: North East Basin

| Inflow Area $=$ | $81,711 \mathrm{sf}$, | $0.76 \%$ Impervious, | Inflow Depth $=3.71 "$ | for $25-$ Year event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $8.06 \mathrm{cfs} @$ | 12.15 hrs , Volume $=$ | $25,274 \mathrm{cf}$ |
| Outflow | $=$ | $2.49 \mathrm{cfs} @$ | 12.38 hrs , Volume $=$ | $25,274 \mathrm{cf}$, Atten= $=69 \%$, Lag= 13.7 min |
| Primary | $=$ | $2.49 \mathrm{cfs} @$ | 12.38 hrs , Volume $=$ | $25,274 \mathrm{cf}$ |

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Peak Elev= 403.56' @ 12.38 hrs Surf.Area= 3,852 sf Storage= 5,517 cf
Plug-Flow detention time $=20.3$ min calculated for $25,271 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time $=20.3 \mathrm{~min}(849.1-828.8)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | 401.00 | $12,521 \mathrm{cf}$ | Custom Stage Data (Irregular)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Perim. <br> (feet) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> (sq-ft) |
| ---: | ---: | ---: | ---: | ---: | ---: |
| 401.00 | 454 | 159.0 | 0 | 0 | 454 |
| 402.00 | 1,868 | 254.0 | 1,081 | 1,081 | 3,583 |
| 403.00 | 3,130 | 251.0 | 2,472 | 3,553 | 3,863 |
| 404.00 | 4,462 | 289.0 | 3,776 | 7,329 | 5,518 |
| 405.00 | 5,957 | 326.0 | 5,192 | 12,521 | 7,355 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 401.00' | 8.0" Round ADS_Round 8" |
|  |  |  | $\mathrm{L}=22.0^{\prime} \mathrm{CMP}$, end-section conforming to fill, $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 401.00' / 400.78' S=0.0100 '/' Cc= 0.900 |
| \#2 | Primary | 404.00' | n=0.013, Flow Area=0.35 si ${ }^{\prime}$ |
|  |  |  | 8.0' long x 4.0' breadth Broad-Crested Rectangular Weir |
|  |  |  | $\begin{array}{llllllllllll}\text { Head (feet) } 0.20 & 0.40 & 0.60 & 0.80 & 1.00 & 1.20 & 1.40 & 1.60 & 1.80 & 2.00\end{array}$ |
|  |  |  | $\begin{array}{llllllllll}2.50 & 3.00 & 3.50 & 4.00 & 4.50 & 5.00 & 5.50\end{array}$ |
|  |  |  | Coef. (English) 2.382 .542 .69 2.68 2.672 .6712 .6512 .662 .66 |
|  |  |  |  |

Primary OutFlow Max=2.49 cfs @ 12.38 hrs HW=403.56' (Free Discharge)
-1=ADS_Round 8" (Barrel Controls 2.49 cfs @ 7.12 fps)
2=Broad-Crested Rectangular Weir ( Controls 0.00 cfs)

Pond 6P: North East Basin


Time span=0.00-72.00 hrs, $\mathrm{dt}=0.01 \mathrm{hrs}, 7201$ points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

## Subcatchment1S:

## Subcatchment2A:

## Subcatchment2S:

## Subcatchment3S:

## Subcatchment4S:

## Subcatchment5S:

## Subcatchment6A:

## Subcatchment6S:

Reach DP 1: Towards Offsite West

Reach DP 2: Towards Offsite North

Reach DP 3: Towards West Wetland

Reach DP 4: Towards East Wetland

Reach DP 5: Towards Offsite Northwest

Reach DP 6: Towards Offsite East

Pond 2P: South West Basin

Pond 6P: North East Basin

Runoff Area=39,843 sf $7.57 \%$ Impervious Runoff Depth=6.50" Flow Length=298' Tc=9.0 min CN=81 Runoff=6.48 cfs $21,583 \mathrm{cf}$

Runoff Area=43,949 sf $1.84 \%$ Impervious Runoff Depth=6.50" Flow Length=261' Tc=8.2 $\mathrm{min} \mathrm{CN}=81$ Runoff=7.35 cfs $23,808 \mathrm{cf}$

Runoff Area=21,866 sf $0.00 \%$ Impervious Runoff Depth=6.50" Flow Length=225' Tc=10.8 min CN=81 Runoff=3.32 cfs $11,845 \mathrm{cf}$

Runoff Area=37,420 sf $0.00 \%$ Impervious Runoff Depth=6.87" Flow Length=212' Tc=6.7 min CN=84 Runoff=6.88 cfs $21,410 \mathrm{cf}$

Runoff Area=37,744 sf $0.00 \%$ Impervious Runoff Depth=6.38" Flow Length=90' Tc=11.5 min CN=80 Runoff=5.48 cfs 20,063 cf

Runoff Area=39,402 sf $0.00 \%$ Impervious Runoff Depth=6.50" Flow Length=109' Tc=6.0 min CN=81 Runoff=7.15 cfs $21,345 \mathrm{cf}$

Runoff Area=81,711 sf $0.76 \%$ Impervious Runoff Depth=6.13" Flow Length=592' Tc=8.1 $\mathrm{min} \mathrm{CN}=78$ Runoff=13.07 cfs $41,774 \mathrm{cf}$

Runoff Area=17,655 sf $0.00 \%$ Impervious Runoff Depth=5.16" Flow Length=213' $\mathrm{Tc}=6.2 \mathrm{~min} \quad \mathrm{CN}=70$ Runoff $=2.62 \mathrm{cfs} 7,590 \mathrm{cf}$

Inflow=6.48 cfs $21,583 \mathrm{cf}$ Outflow $=6.48$ cfs $21,583 \mathrm{cf}$

Inflow=6.07 cfs 35,651 cf Outflow=6.07 cfs 35,651 cf

Inflow=6.88 cfs 21,410 cf Outflow=6.88 cfs 21,410 cf

Inflow=5.48 cfs 20,063 cf Outflow=5.48 cfs 20,063 cf

Inflow=7.15 cfs 21,345 cf Outflow=7.15 cfs 21,345 cf

Inflow=7.96 cfs 49,364 cf Outflow=7.96 cfs 49,364 cf

Peak Elev=458.40' Storage=5,473 cf Inflow=7.35 cfs 23,808 cf 10.0" Round Culvert $n=0.013 \mathrm{~L}=14.0$ ' $\mathrm{S}=0.0100$ '//' Outflow=2.98 cfs $23,806 \mathrm{cf}$

Peak Elev=404.34' Storage=8,927 cf Inflow=13.07 cfs 41,774 cf Outflow=6.86 cfs 41,774 cf

$$
\begin{aligned}
& \text { Total Runoff Area }=319,590 \text { sf Runoff Volume }=169,418 \text { cf Average Runoff Depth }=6.36 \text { " } \\
& \mathbf{9 8 . 6 1 \%} \text { Pervious }=\mathbf{3 1 5 , 1 4 4} \mathbf{s f} \quad 1.39 \% \text { Impervious }=4,446 \text { sf }
\end{aligned}
$$

## Summary for Subcatchment 1S:

Runoff $=\quad 6.48$ cfs @ 12.16 hrs, Volume $=\quad 21,583 \mathrm{cf}$, Depth= 6.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 100-Year Rainfall=8.80"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 13,399 | 77 V | Woods, Good, HSG D |  |  |
|  | 1,909 | 98 R | Roofs, HSG D |  |  |
|  | 20,826 | 80 > | >75\% Grass cover, Good, HSG D |  |  |
|  | 2,600 | 96 | Gravel surface, HSG D |  |  |
|  | 1,109 | 98 P | Paved parking, HSG D |  |  |
|  | 39,843 | 81 | Weighted Average |  |  |
|  | 36,825 |  | 92.43\% Pervious Area |  |  |
|  | 3,018 |  | 7.57\% Impervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 4.1 | 50 | 0.0400 | 0.20 |  | Sheet Flow, Grass: Short $n=0.150 \quad \mathrm{P} 2=3.44{ }^{\prime \prime}$ |
| 0.2 | 14 | 0.0230 | - 1.06 |  | Shallow Concentrated Flow, |
| 1.5 | 65 | 0.0200 | 0.71 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 0.4 | 59 | 0.0200 | - 2.28 |  | Shallow Concentrated Flow, Unpaved Kv= 16.1 fps |
| 2.8 | 110 | 0.0170 | - 0.65 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |

9.0298 Total


## Summary for Subcatchment 2A:

Runoff $=7.35$ cfs @ 12.15 hrs, Volume $=\quad 23,808 \mathrm{cf}$, Depth= 6.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 100-Year Rainfall=8.80"

|  | Area (sf) | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 17,342 |  |  |  |  |
|  | 808 |  | >75\% Grass cover, Good, HSG D Roofs, HSG D |  |  |
|  | 20,539 |  | Woods, Good, HSG D |  |  |
|  | 5,260 | 96 | Gravel surface, HSG D |  |  |
|  | 43,949 | 81 | Weighted Average 98.16\% Pervious Area 1.84\% Impervious Area |  |  |
|  | 43,141 |  |  |  |  |
|  | 808 |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope $(\mathrm{ft} / \mathrm{ft})$ | $\begin{gathered} \text { Velocity } \\ (\mathrm{ft} / \mathrm{sec}) \end{gathered}$ | $\begin{array}{r} \text { Capacity } \\ \text { (cfs) } \end{array}$ | Description |
| 4.7 | 50 | 0.0280 | 0.18 |  | Sheet Flow, Grass: Short $n=0.150 \quad \mathrm{P} 2=3.44{ }^{\prime \prime}$ |
| 0.2 | 12 | 0.0210 | - 1.01 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |
| 1.7 | 76 | 0.0210 | - 0.72 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 0.2 | 22 | 0.0200 | - 2.28 |  | Shallow Concentrated Flow, Unpaved Kv= 16.1 fps |
| 1.4 | 101 | 0.0300 | - 1.21 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |

$8.2 \quad 261$ Total


## Summary for Subcatchment 2S:

Runoff $=3.32$ cfs @ 12.18 hrs, Volume $=11,845 \mathrm{cf}$, Depth= 6.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 100-Year Rainfall=8.80"


## Subcatchment 2S:



## Summary for Subcatchment 3S:

Runoff $=\quad 6.88$ cfs @ 12.14 hrs, Volume $=\quad 21,410 \mathrm{cf}$, Depth= 6.87"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 100-Year Rainfall=8.80"

| Area (sf)3,70929,8203,891 |  | CN | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Woods, Good, HSG D Brush, Poor, HSG D Gravel surface, HSG D |  |  |
|  |  |  |  |  |
|  |  | 96 |  |  |  |
| $\begin{aligned} & 37,420 \\ & 37,420 \end{aligned}$ |  |  | 84 | Weighted Average 100.00\% Pervious Area |  |  |
|  |  |  |  |  |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \\ \hline \end{array}$ | Length (feet) | Slope <br> (ft/ft) | Velocity <br> (ft/sec) | Capacity (cfs) | Description |  |
| 4.5 | 50 | 0.0840 | 0.19 |  | Sheet Flow, Grass: Dense n=0.240 P2=3.44" |  |
| 1.4 | 98 | 0.0286 | 1.18 |  | Shallow Concentrated Flow, Short Grass Pasture Kv=7.0 fps |  |
| 0.1 | 12 | 0.0588 | 3.90 |  | Shallow Concentrated Flow, Unpaved Kv= 16.1 fps |  |
| 0.7 | 52 | 0.0323 | 1.26 |  | Shallow Concentrated Flow, <br> Short Grass Pasture $\mathrm{Kv}=7.0 \mathrm{fps}$ |  |
| 6.7 | 212 | Total |  |  |  |  |

## Subcatchment 3S:


$\square$ Runoff

## Summary for Subcatchment 4S:

Runoff $=5.48$ cfs @ 12.19 hrs, Volume $=\quad 20,063 \mathrm{cf}$, Depth= 6.38"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 100-Year Rainfall=8.80"

| Area (sf) | CN | Description |  |
| ---: | ---: | :--- | :--- |
| 14,923 | 83 | Brush, Poor, HSG D |  |
| 4,794 | 96 | Gravel surface, HSG D |  |
| 10,680 | 77 | Woods, Good, HSG D |  |
| 7,347 | 70 | Woods, Good, HSG C |  |

Subcatchment 4S:
Hydrograph


## Summary for Subcatchment 5S:

Runoff $=7.15$ cfs @ 12.13 hrs, Volume $=\quad 21,345 \mathrm{cf}$, Depth= 6.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 100-Year Rainfall=8.80"

|  | Area (sf) | CN D | Description |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | 23,878 | 80 | >75\% Grass cover, Good, HSG D |  |  |
|  | 7,772 |  | Woods, Good, HSG D |  |  |
|  | 2,272 | 70 | Woods, Good, HSG C |  |  |
|  | 5,480 | 96 | Gravel surface, HSG D |  |  |
|  | $\begin{aligned} & 39,402 \\ & 39,402 \end{aligned}$ |  | Weighted Average 100.00\% Pervious Area |  |  |
| $\begin{array}{r} \mathrm{Tc} \\ (\mathrm{~min}) \end{array}$ | Length (feet) | Slope (ft/ft) | Velocity (ft/sec) | Capacity (cfs) | Description |
| 2.7 | 50 | 0.1200 | 0.31 |  | Sheet Flow, Grass: Short $\mathrm{n}=0.150 \quad \mathrm{P} 2=3.44{ }^{\prime \prime}$ |
| 0.5 | 44 | 0.0520 | - 1.60 |  | Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps |
| 0.2 | 15 | 0.0530 | - 1.15 |  | Shallow Concentrated Flow, Woodland $\mathrm{Kv}=5.0 \mathrm{fps}$ |
| 3.4 | 109 | Total, | Increased | minimum | $\mathrm{Tc}=6.0 \mathrm{~min}$ |

## Subcatchment 5S:


$\square$ Runoff

## Summary for Subcatchment 6A:

Runoff $=13.07$ cfs @ 12.15 hrs, Volume $=\quad 41,774 \mathrm{cf}$, Depth= 6.13"
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$ NRCC 24-hr C 100-Year Rainfall=8.80"



## Summary for Subcatchment 6S:

Runoff $=\quad 2.62$ cfs @ 12.13 hrs, Volume $=\quad 7,590 \mathrm{cf}$, Depth= $5.1^{\prime \prime}$

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs NRCC 24-hr C 100-Year Rainfall=8.80"

| Area (sf) | CN | Description |  |
| ---: | ---: | ---: | :--- |
| 17,291 | 70 | Woods, Good, HSG C |  |
| 364 | 77 | Woods, Good, HSG D |  |

## Subcatchment 6S:


$\square$ Runoff

## Summary for Reach DP 1: Towards Offsite West

[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
39,843 sf, 7.57\% Impervious, Inflow Depth = 6.50" for 100-Year event
Outflow =
6.48 cfs @ 12.16 hrs, Volume=

21,583 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 1: Towards Offsite West



## Summary for Reach DP 2: Towards Offsite North

[40] Hint: Not Described (Outflow=Inflow)

| Inflow Area $=$ | $65,815 \mathrm{sf}$, | $1.23 \%$ Impervious, | Inflow Depth $=6.50 "$ | for $100-$ Year event |
| :--- | :--- | :--- | :--- | :--- |
| Inflow | $=$ | $6.07 \mathrm{cfs} @$ | 12.19 hrs , Volume $=$ | $35,651 \mathrm{cf}$ |
| Outflow | $=$ | $6.07 \mathrm{cfs} @$ | 12.19 hrs , Volume $=$ | $35,651 \mathrm{cf}$, Atten $=0 \%$, Lag $=0.0 \mathrm{~min}$ |

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 2: Towards Offsite North


## Summary for Reach DP 3: Towards West Wetland

[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
37,420 sf, 0.00\% Impervious, Inflow Depth = 6.87" for 100-Year event
Inflow =
6.88 cfs @ 12.14 hrs, Volume=

Outflow =
6.88 cfs @ 12.14 hrs, Volume=

Routing by Stor-Ind+Trans method, Time Span= $0.00-72.00 \mathrm{hrs}, \mathrm{dt}=0.01 \mathrm{hrs}$

## Reach DP 3: Towards West Wetland



Summary for Reach DP 4: Towards East Wetland
[40] Hint: Not Described (Outflow=Inflow)
Inflow Area =
Inflow =
37,744 sf, 0.00\% Impervious, Inflow Depth = 6.38" for 100-Year event
Outflow =
5.48 cfs @ 12.19 hrs , Volume= 20,063 cf

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 4: Towards East Wetland


## Summary for Reach DP 5: Towards Offsite Northwest

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = Inflow = Outflow =

39,402 sf, 0.00\% Impervious, Inflow Depth = 6.50" for 100-Year event

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs

## Reach DP 5: Towards Offsite Northwest



## Summary for Reach DP 6: Towards Offsite East

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area =
Inflow =
Outflow =

99,366 sf, 0.62\% Impervious, Inflow Depth = 5.96" for 100-Year event 7.96 cfs @ 12.26 hrs, Volume= 49,364 cf 7.96 cfs @ 12.26 hrs, Volume=

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Reach DP 6: Towards Offsite East


## Summary for Pond 2P: South West Basin

| Inflow Area = | 43,949 sf, | \% Impervious, | Inflow Depth = 6.50" for 100-Year event |
| :---: | :---: | :---: | :---: |
| Inflow | 7.35 cfs @ | 12.15 hrs , Volume= | 23,808 cf |
| Outflow | 2.98 cfs @ | 12.31 hrs , Volume= | $23,806 \mathrm{cf}$, Atten= 59\%, Lag= 9.6 min |
| Primary | 2.98 cfs @ | 12.31 hrs , Volume= | 23,806 cf |

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Peak Elev= 458.40' @ 12.31 hrs Surf.Area= 3,997 sf Storage= 5,473 cf
Plug-Flow detention time $=43.2$ min calculated for 23,806 cf ( $100 \%$ of inflow)
Center-of-Mass det. time $=43.1 \mathrm{~min}(848.5-805.5)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | $456.70^{\prime}$ | 6,701 cf | Custom Stage Data (Irregular)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Perim. <br> (feet) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> (sq-ft) |
| ---: | ---: | ---: | ---: | ---: | ---: |
| 456.70 | 2,487 | 270.0 | 0 | 0 | 2,487 |
| 458.70 | 4,296 | 320.0 | 6,701 | 6,701 | 4,908 |



Primary OutFlow Max=2.98 cfs @ 12.31 hrs HW=458.40' (Free Discharge)
L-1=ADS Round 10" (Inlet Controls 2.98 cfs @ 5.46 fps )

## Pond 2P: South West Basin



## Summary for Pond 6P: North East Basin

| Inflow Area = | 81,711 sf, | 0.76\% Impervious, | Inflow Depth = 6.13" for 100-Year event |
| :---: | :---: | :---: | :---: |
| Inflow | 13.07 cfs @ | 12.15 hrs , Volume= | 41,774 cf |
| Outflow | 6.86 cfs @ | 12.27 hrs , Volume= | 41,774 cf, Atten= 47\%, Lag= 6.9 min |
| Primary | 6.86 cfs @ | 12.27 hrs, Volume= | 41,774 cf |

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs
Peak Elev= 404.34' @ 12.27 hrs Surf.Area=4,946 sf Storage= 8,927 cf
Plug-Flow detention time $=22.6$ min calculated for $41,768 \mathrm{cf}$ ( $100 \%$ of inflow)
Center-of-Mass det. time $=22.6 \mathrm{~min}(835.5-812.9)$

| Volume | Invert | Avail.Storage | Storage Description |
| :---: | ---: | ---: | ---: |
| $\# 1$ | 401.00 | $12,521 \mathrm{cf}$ | Custom Stage Data (Irregular)Listed below (Recalc) |


| Elevation <br> (feet) | Surf.Area <br> (sq-ft) | Perim. <br> (feet) | Inc.Store <br> (cubic-feet) | Cum.Store <br> (cubic-feet) | Wet.Area <br> (sq-ft) |
| ---: | ---: | ---: | ---: | ---: | ---: |
| 401.00 | 454 | 159.0 | 0 | 0 | 454 |
| 402.00 | 1,868 | 254.0 | 1,081 | 1,081 | 3,583 |
| 403.00 | 3,130 | 251.0 | 2,472 | 3,553 | 3,863 |
| 404.00 | 4,462 | 289.0 | 3,776 | 7,329 | 5,518 |
| 405.00 | 5,957 | 326.0 | 5,192 | 12,521 | 7,355 |


| Device | Routing | Invert | Outlet Devices |
| :---: | :---: | :---: | :---: |
| \#1 | Primary | 401.00' | 8.0" Round ADS_Round 8" |
|  |  |  | L=22.0' CMP, end-section conforming to fill, $\mathrm{Ke}=0.500$ |
|  |  |  | Inlet / Outlet Invert= 401.00' / 400.78' S=0.0100 '/' Cc= 0.900 |
|  |  |  | $\mathrm{n}=0.013$, Flow Area $=0.35 \mathrm{sf}$ |
| \#2 | Primary | 404.00' | 8.0' long x 4.0' breadth Broad-Crested Rectangular Weir |
|  |  |  | Head (feet) 0.200 .400 .600 .801 .001 .201 .401 .601 .80 |
|  |  |  | 2.503 .003 .504 .004 .505 .005 .50 |
|  |  |  | Coef. (English) 2.382 .542 .692 .682 .672 .6712 .6512 .662 .66 |
|  |  |  |  |

Primary OutFlow Max=6.85 cfs @ 12.27 hrs HW=404.34' (Free Discharge)
-1=ADS_Round 8" (Barrel Controls 2.91 cfs @ 8.33 fps)
-2=Broad-Crested Rectangular Weir (Weir Controls 3.94 cfs @ 1.45 fps )

Pond 6P: North East Basin


APPENDIX C
Pre- and Post-Development Watershed Plans



APPENDIX D Miscellaneous Calculations

## Required Recharge Volume

| Design Engineer: | Atlantic Design Engineers, Inc | Job No.: | 3328.00 |
| :--- | :--- | :---: | :--- |
| Project Name: | Upper Union Solar Project | Calc'd By: | NCM |
| Location: | 0 Upper Union Street | Revised Date: | 6/19/2023 |

The groundwater recharge volume is required for the proposed asphalt impervious area.

$$
\begin{gathered}
\mathrm{Rv}=(\mathrm{F})(\text { Aimp }) \\
\mathrm{Rv}=\mathrm{Required} \mathrm{Recharge} \mathrm{Volume} \\
\text { Aimp }=\text { Impervious Area on site } \\
\mathrm{F}=\text { Target Depth Factor: } 0.1 \text { inch for } \mathrm{D} \text { soils }
\end{gathered}
$$

| Required Recharge |  |
| :---: | :---: |
| Total New Impervious Area $=$ |  |
| Required Recharge Volume $(\mathrm{Rv})=$ |  |
| $1,109 \mathrm{sf}$ |  |
| Recharge Volume Provided |  |
| Cultec 100 HD Subsurface System |  |
| * $0.1^{\prime \prime *}(1 / 12)=$ |  |
| (See HydroCAD Calcs) |  |
| Proposed Volume Provided in Sub- |  |
| Surface Systems= |  |

## Total Required Recharge Volume on Site= 9 cf

Proposed Recharge Volume Provided in Subsurface System=

## Required Recharge Volume

| Design Engineer: | Atlantic Design Engineers, Inc | Job No.: | 3328.00 |
| :--- | :--- | :---: | :--- |
| Project Name: | Upper Union Solar Project | Calc'd By: | NCM |
| Location: | 0 Upper Union Street | Revised Date: | 6/19/2023 |

The groundwater recharge volume is required for the proposed equipment pad impervious area.

$$
\begin{gathered}
R v=(F)(\text { Aimp }) \\
R v=\text { Required Recharge Volume } \\
\text { Aimp= Impervious Area on site } \\
F=\text { Target Depth Factor: } 0.1 \text { inch for D soils }
\end{gathered}
$$

| Infiltration Trench (50'Lx2'Wx1'D @ 40\% Voids) |  |  |
| :---: | :---: | :---: |
| Total New Impervious Area = | 640 sf |  |
| Required Recharge Volume (Rv)= | 640 *0.1"* (1/12)= | 5 cf |

Recharge Volume Provided
Infiltration Trench

$$
50 * 2 * 1 *(1-60 \%)=
$$

60 cf

Proposed Volume Provided in Infiltration Trench=

60 cf

## Total Required Recharge Volume on Site= <br> 5 cf

Proposed Recharge Volume Provided in Infiltration Trench=

60 cf
$60 \mathrm{cf}>5 \mathrm{cf}$

Standard is Met

## Water Quality Calulation Sheet*

| Design Engineer: | Atlantic Design Engineers, Inc | Job No.: | 3328.00 |
| :--- | :--- | :---: | :--- |
| Project Name: | Upper Union Solar Project | Calc'd By: | NCM |
| Location: | 0 Upper Union Street | Revised Date: | 6/19/2023 |

The required water quality treatment volume is calculated as follows:

> Vwq $=(\text { Dwq })^{\star}($ Aimp $)$
> Vwq $=$ Required Water Quality Volume

Dwq =Water Quality Depth * 1"
Aimp=Area of Impervious

Subcatchment Area: 1A
Total Impervious Area for the Subcatchment= 1,109 sf

| Water Quality Volume Required $(\mathrm{Vwq})$ | $=1,109$ | $* 1^{\prime \prime} *(1 / 12)=$ | 92 | cf |
| ---: | :--- | :---: | :---: | :---: |
| Volume Provided via Cultec 100 HD | $=$ | See Hydrocad | 126 | cf |

Volume Required $=92.4 \quad<\quad 126 \quad$ Water Quality Volume is met

## Subcatchment Area: 6A

Total Impervious Area for the Subcatchment= 640 sf

| Water Quality Volume Required (Vwq) $=$ | 640 | * 1" * $(1 / 12)=$ | 53 |
| :--- | :--- | :---: | :--- |


| Volume Required $=$ | 53.3 | $<$ | 60 |
| ---: | :--- | :---: | :--- |
| Total Impervious Area on the Site $=$ | 1,749 | sf |  |
| Total Volume Quality Required $=$ | 146 | cf |  |
| Total Volume Provided $=$ | 186 | cf |  |

* The purpose of these calculations is to show compliance with the Town of Franklin Stormwater Management Bylaw Chapter 153, specifically Section153-16.B.(1).(a)

APPENDIX E NRCS Soil Survey Maps and Soil Group Descriptions

United States Department of Agriculture


Natural
Resources
Conservation
Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Norfolk and Suffolk Counties, Massachusetts


## Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.
Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/ portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).
Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.
Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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## How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil
scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.
Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.
Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.
After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

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identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

## Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.


## MAP LEGEND

| Area of Interest (AOI) |  |
| :--- | :--- |
| $\square$ | Area of Interest (AOI) |
| Soils |  |
| $\square$ | Soil Map Unit Polygons |
| $\square$ | Soil Map Unit Lines |
| $\square$ | Soil Map Unit Points |

Special Point Features
(c) Blowout

B Borrow Pit
次 Clay Spot
$\diamond$ Closed Depression
Bravel Pit
$\therefore \quad$ Gravelly Spot
(4) Landfill
A. Lava Flow
A. Marsh or swamp
\& Mine or Quarry
(C) Miscellaneous Water

- Perennial Water
- Rock Outcrop
+ Saline Spot
$\because \quad$ Sandy Spot
을 Severely Eroded Spot
- Sinkhole

3) Slide or Slip
(6) Sodic Spot

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:25,000.

Warning: Soil Map may not be valid at this scale.
Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Norfolk and Suffolk Counties, Massachusetts Survey Area Data: Version 18, Sep 9, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 22, 2022—Jun 5, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background magery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

# Map Unit Legend 

| Map Unit Symbol | Map Unit Name | Acres in AOI | Percent of AOI |
| :---: | :---: | :---: | :---: |
| 51 | Swansea muck, 0 to 1 percent slopes | 0.6 | 2.0\% |
| 103B | Charlton-Hollis-Rock outcrop complex, 3 to 8 percent slopes | 1.8 | 5.8\% |
| 103C | Charlton-Hollis-Rock outcrop complex, 8 to 15 percent slopes | 9.9 | 32.4\% |
| 103D | Charlton-Hollis-Rock outcrop complex, 15 to 25 percent slopes | 8.6 | 28.0\% |
| 310B | Woodbridge fine sandy loam, 3 to 8 percent slopes | 6.8 | 22.3\% |
| 312B | Woodbridge fine sandy loam, 0 to 8 percent slopes, extremely stony | 2.9 | 9.5\% |
| Totals for Area of Interest |  | 30.6 | 100.0\% |

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.
Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a
given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.
Soils that have profiles that are almost alike make up a soil series. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into soil phases. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.
Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A complex consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.
An undifferentiated group is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include miscellaneous areas. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## Norfolk and Suffolk Counties, Massachusetts

## 51—Swansea muck, 0 to 1 percent slopes

## Map Unit Setting

National map unit symbol: 2 trl2
Elevation: 0 to 1,140 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees $F$
Frost-free period: 140 to 240 days
Farmland classification: Not prime farmland

## Map Unit Composition

Swansea and similar soils: 80 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Swansea

## Setting

Landform: Bogs, swamps
Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Concave
Parent material: Highly decomposed organic material over loose sandy and gravelly glaciofluvial deposits

## Typical profile

Oa1-0 to 24 inches: muck
Oa2-24 to 34 inches: muck
Cg-34 to 79 inches: coarse sand

## Properties and qualities

Slope: 0 to 1 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high
( 0.14 to $14.17 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: About 0 to 6 inches
Frequency of flooding: Rare
Frequency of ponding: Frequent
Available water supply, 0 to 60 inches: Very high (about 16.5 inches)
Interpretive groups
Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 8w
Hydrologic Soil Group: B/D
Ecological site: F144AY043MA - Acidic Organic Wetlands
Hydric soil rating: Yes

## Minor Components

Freetown
Percent of map unit: 10 percent
Landform: Bogs, swamps

Landform position (three-dimensional): Dip
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

## Whitman

Percent of map unit: 5 percent
Landform: Drainageways, depressions
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Base slope
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

## Scarboro

Percent of map unit: 5 percent
Landform: Drainageways, depressions
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Base slope, tread, dip
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

## 103B—Charlton-Hollis-Rock outcrop complex, 3 to 8 percent slopes

## Map Unit Setting

National map unit symbol: vktd
Elevation: 0 to 480 feet
Mean annual precipitation: 32 to 54 inches
Mean annual air temperature: 43 to 54 degrees F
Frost-free period: 120 to 240 days
Farmland classification: Not prime farmland

## Map Unit Composition

Charlton and similar soils: 40 percent
Hollis and similar soils: 25 percent
Rock outcrop: 20 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Charlton

Setting
Landform: Hills
Landform position (two-dimensional): Shoulder
Landform position (three-dimensional): Side slope
Down-slope shape: Convex
Across-slope shape: Convex
Parent material: Friable coarse-loamy ablation till derived from granite

## Typical profile

H1-0 to 6 inches: fine sandy loam
H2-6 to 36 inches: fine sandy loam
H3-36 to 60 inches: fine sandy loam

## Properties and qualities

## Slope: 3 to 8 percent

Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high ( 0.60 to $6.00 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Moderate (about 7.8 inches)

## Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s
Hydrologic Soil Group: A
Ecological site: F144AY034CT - Well Drained Till Uplands
Hydric soil rating: No

## Description of Hollis

## Setting

Landform: Hills
Landform position (two-dimensional): Shoulder
Landform position (three-dimensional): Side slope
Down-slope shape: Convex
Across-slope shape: Convex
Parent material: Shallow, friable loamy ablation till derived from igneous rock

## Typical profile

H1-0 to 3 inches: fine sandy loam
H2-3 to 14 inches: gravelly fine sandy loam
H3-14 to 18 inches: unweathered bedrock

## Properties and qualities

Slope: 3 to 8 percent
Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: 10 to 20 inches to lithic bedrock
Drainage class: Well drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low ( 0.00 to $0.14 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Very low (about 1.8 inches)
Interpretive groups
Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6s
Hydrologic Soil Group: D

Ecological site: F144AY033MA - Shallow Dry Till Uplands
Hydric soil rating: No

## Description of Rock Outcrop

## Setting

Parent material: Igneous and metamorphic rock
Properties and qualities
Slope: 3 to 8 percent
Depth to restrictive feature: 0 inches to lithic bedrock
Interpretive groups
Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 8s
Hydric soil rating: Unranked

## Minor Components

Canton
Percent of map unit: 7 percent
Hydric soil rating: No
Chatfield
Percent of map unit: 5 percent
Hydric soil rating: No

## Scituate

Percent of map unit: 2 percent
Hydric soil rating: No

## Whitman

Percent of map unit: 1 percent
Landform: Depressions
Hydric soil rating: Yes

## 103C—Charlton-Hollis-Rock outcrop complex, 8 to 15 percent slopes

## Map Unit Setting

National map unit symbol: 2wzp1
Elevation: 0 to 1,390 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Not prime farmland

## Map Unit Composition

Charlton, extremely stony, and similar soils: 50 percent
Hollis, extremely stony, and similar soils: 20 percent
Rock outcrop: 10 percent

Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Charlton, Extremely Stony

## Setting

Landform: Ridges, hills
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Convex, linear
Across-slope shape: Convex
Parent material: Coarse-loamy melt-out till derived from granite, gneiss, and/or schist

## Typical profile

Oe - 0 to 2 inches: moderately decomposed plant material
A - 2 to 4 inches: fine sandy loam
Bw - 4 to 27 inches: gravelly fine sandy loam
C-27 to 65 inches: gravelly fine sandy loam

## Properties and qualities

Slope: 8 to 15 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high
( 0.14 to $14.17 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline ( 0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 8.7 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7s
Hydrologic Soil Group: B
Ecological site: F144AY034CT - Well Drained Till Uplands
Hydric soil rating: No

## Description of Hollis, Extremely Stony

## Setting

Landform: Ridges, hills
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Nose slope, side slope, crest
Down-slope shape: Convex
Across-slope shape: Linear, convex
Parent material: Coarse-loamy melt-out till derived from granite, gneiss, and/or schist

## Typical profile

Oi-0 to 2 inches: slightly decomposed plant material
A - 2 to 7 inches: gravelly fine sandy loam
Bw-7 to 16 inches: gravelly fine sandy loam
$2 R$ - 16 to 26 inches: bedrock

## Properties and qualities

Slope: 8 to 15 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: 8 to 23 inches to lithic bedrock
Drainage class: Somewhat excessively drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Very low (0.00 to 0.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline ( 0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Very low (about 2.7 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7s
Hydrologic Soil Group: D
Ecological site: F144AY033MA - Shallow Dry Till Uplands
Hydric soil rating: No

## Description of Rock Outcrop

## Setting

Landform: Ridges, hills
Parent material: Igneous and metamorphic rock

## Typical profile

$R$ - 0 to 79 inches: bedrock

## Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: 0 inches to lithic bedrock
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Very low (0.00 to 0.00 in/hr)
Available water supply, 0 to 60 inches: Very low (about 0.0 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 8
Hydrologic Soil Group: D
Hydric soil rating: No

## Minor Components

Woodbridge, extremely stony
Percent of map unit: 8 percent
Landform: Ground moraines, hills, drumlins
Landform position (two-dimensional): Backslope, footslope
Landform position (three-dimensional): Side slope
Down-slope shape: Convex
Across-slope shape: Linear
Hydric soil rating: No
Chatfield, extremely stony
Percent of map unit: 5 percent

Landform: Ridges, hills
Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Nose slope, side slope, crest
Down-slope shape: Convex
Across-slope shape: Linear, convex
Hydric soil rating: No
Canton, extremely stony
Percent of map unit: 5 percent
Landform: Moraines, hills, ridges
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Convex, linear
Across-slope shape: Convex
Hydric soil rating: No
Ridgebury, extremely stony
Percent of map unit: 2 percent
Landform: Hills, drainageways, drumlins, depressions, ground moraines
Landform position (two-dimensional): Footslope, toeslope
Landform position (three-dimensional): Head slope, base slope
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

## 103D—Charlton-Hollis-Rock outcrop complex, 15 to 25 percent slopes

## Map Unit Setting

National map unit symbol: vktk
Elevation: 0 to 490 feet
Mean annual precipitation: 32 to 54 inches
Mean annual air temperature: 43 to 54 degrees F
Frost-free period: 120 to 240 days
Farmland classification: Not prime farmland

## Map Unit Composition

Charlton and similar soils: 35 percent
Hollis and similar soils: 25 percent
Rock outcrop: 20 percent
Minor components: 20 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Charlton

Setting
Landform: Hills
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope

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Down-slope shape: Linear
Across-slope shape: Convex
Parent material: Friable coarse-loamy ablation till derived from granite

## Typical profile

H1-0 to 6 inches: fine sandy loam
H2 - 6 to 36 inches: fine sandy loam
H3-36 to 60 inches: fine sandy loam

## Properties and qualities

Slope: 15 to 25 percent
Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high ( 0.60 to $6.00 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Moderate (about 7.8 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6s
Hydrologic Soil Group: A
Ecological site: F144AY034CT - Well Drained Till Uplands
Hydric soil rating: No

## Description of Hollis

## Setting

Landform: Hills
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Convex
Parent material: Shallow, friable loamy ablation till derived from igneous rock

## Typical profile

H1-0 to 3 inches: fine sandy loam
H2-3 to 14 inches: gravelly fine sandy loam
H3-14 to 18 inches: unweathered bedrock

## Properties and qualities

Slope: 15 to 25 percent
Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: 10 to 20 inches to lithic bedrock
Drainage class: Well drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low ( 0.00 to $0.14 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Very low (about 1.8 inches)

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6s
Hydrologic Soil Group: D
Ecological site: F144AY033MA - Shallow Dry Till Uplands
Hydric soil rating: No

## Description of Rock Outcrop

## Setting

Parent material: Igneous and metamorphic rock
Properties and qualities
Slope: 15 to 25 percent
Depth to restrictive feature: 0 inches to lithic bedrock

## Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 8s
Hydric soil rating: Unranked

## Minor Components

## Chatfield

Percent of map unit: 8 percent
Hydric soil rating: No

## Canton

Percent of map unit: 8 percent
Hydric soil rating: No

## Montauk

Percent of map unit: 4 percent
Hydric soil rating: No

## 310B-Woodbridge fine sandy loam, 3 to 8 percent slopes

## Map Unit Setting

National map unit symbol: 2t2ql
Elevation: 0 to 1,470 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: All areas are prime farmland

## Map Unit Composition

Woodbridge, fine sandy loam, and similar soils: 82 percent
Minor components: 18 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Woodbridge, Fine Sandy Loam

## Setting

Landform: Ground moraines, drumlins, hills
Landform position (two-dimensional): Summit, backslope, footslope
Landform position (three-dimensional): Side slope
Down-slope shape: Concave
Across-slope shape: Linear
Parent material: Coarse-loamy lodgment till derived from gneiss, granite, and/or schist

## Typical profile

Ap-0 to 7 inches: fine sandy loam
Bw1-7 to 18 inches: fine sandy loam
Bw2 - 18 to 30 inches: fine sandy loam
Cd - 30 to 65 inches: gravelly fine sandy loam

## Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: 20 to 39 inches to densic material
Drainage class: Moderately well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low ( 0.00 to $0.14 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: About 18 to 30 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline ( 0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 3.6 inches)
Interpretive groups
Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 2w
Hydrologic Soil Group: C/D
Ecological site: F144AY037MA - Moist Dense Till Uplands
Hydric soil rating: No

## Minor Components

## Paxton

Percent of map unit: 10 percent
Landform: Drumlins, ground moraines, hills
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Nose slope, side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Convex
Hydric soil rating: No
Ridgebury
Percent of map unit: 8 percent
Landform: Depressions, ground moraines, hills, drainageways
Landform position (two-dimensional): Toeslope, backslope, footslope
Landform position (three-dimensional): Base slope, head slope, dip
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

# 312B—Woodbridge fine sandy loam, 0 to 8 percent slopes, extremely stony 

## Map Unit Setting

National map unit symbol: 2t2qs
Elevation: 0 to 1,580 feet
Mean annual precipitation: 36 to 71 inches
Mean annual air temperature: 39 to 55 degrees F
Frost-free period: 140 to 240 days
Farmland classification: Not prime farmland

## Map Unit Composition

Woodbridge, extremely stony, and similar soils: 82 percent
Minor components: 18 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

## Description of Woodbridge, Extremely Stony

## Setting

Landform: Ground moraines, hills, drumlins
Landform position (two-dimensional): Summit, backslope, footslope
Landform position (three-dimensional): Side slope
Down-slope shape: Concave
Across-slope shape: Linear
Parent material: Coarse-loamy lodgment till derived from gneiss, granite, and/or schist

## Typical profile

Oe - 0 to 2 inches: moderately decomposed plant material
A - 2 to 9 inches: fine sandy loam
Bw1-9 to 20 inches: fine sandy loam
Bw2 - 20 to 32 inches: fine sandy loam
Cd - 32 to 67 inches: gravelly fine sandy loam

## Properties and qualities

Slope: 0 to 8 percent
Surface area covered with cobbles, stones or boulders: 9.0 percent
Depth to restrictive feature: 20 to 43 inches to densic material
Drainage class: Moderately well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low ( 0.00 to $0.14 \mathrm{in} / \mathrm{hr}$ )
Depth to water table: About 19 to 27 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline ( 0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Low (about 4.0 inches)
Interpretive groups
Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7s
Hydrologic Soil Group: C/D
Ecological site: F144AY037MA - Moist Dense Till Uplands
Hydric soil rating: No

## Minor Components

## Paxton, extremely stony

Percent of map unit: 10 percent
Landform: Ground moraines, hills, drumlins
Landform position (two-dimensional): Summit, shoulder, backslope
Landform position (three-dimensional): Side slope, crest
Down-slope shape: Convex, linear
Across-slope shape: Linear, convex
Hydric soil rating: No
Ridgebury, extremely stony
Percent of map unit: 8 percent
Landform: Hills, drainageways, drumlins, depressions, ground moraines
Landform position (two-dimensional): Toeslope
Landform position (three-dimensional): Head slope, base slope
Down-slope shape: Concave
Across-slope shape: Concave
Hydric soil rating: Yes

## APPENDIX F

 Post-Construction Long Term Stormwater Operation and Maintenance PlanUpper Union Solar Project
At
0 Upper Union Street - Franklin, MA
Post-Construction Long Term Stormwater Operation \& Maintenance Plan
June 19, 2023
ADE Job \#3328.00

## A. GENERAL NOTES

1. Upon completion of construction, the operation and maintenance of all components of the stormwater management system will be the responsibility (financially and otherwise) of the system owner (responsible party):

VS Union Solar Smart, LLC<br>24941 Dana Point Harbor<br>Dana Point, California 92629

## Signature

## Date

2. The responsible party shall file an inspection report with the Town of Franklin DPW following each site inspection as recommended in the Operation \& Maintenance (O\&M) Schedule. The inspection report shall identify the date of inspection, name, and contact number of responsible party, specific structures inspected, specific maintenance and/or repairs required and general observations. Any deficiencies noted in the inspection report shall be corrected to the Town of Franklin's DPW's satisfaction.
3. Disposal of accumulated sediment and hydrocarbons to be in accordance with the applicable local, state, and federal guidelines and regulations.
4. There shall be no illicit discharge of any waste or waste water into the stormwater management system. The maintenance of the facility shall be undertaken in such a manner as to prevent any discharge of waste or waste water into the stormwater management system. Any waste oil or other waste products generated during the maintenance shall be properly disposed of offsite.
5. The Town will be notified of changes in project ownership or assignment of operation and maintenance financial responsibility.
6. The maintenance schedule in this operation and maintenance (O\&M) Plan will only be amended by mutual agreement of the Town and the responsible party. Amendments will be made in writing and signed by the responsible party.

## B. STORMWATER SYSTEM/BMPS

## Erosion control barriers:

Until the site is fully stabilized, erosion control barriers (sediment log, straw wattles, silt fence, etc.) should be inspected immediately after major storm events (2" or greater). Sediment deposits must be removed when the level of deposition reaches approximately one-half the height of the barrier. Repair/replace any sections of erosion control barriers that are damaged and install additional rows of barriers if needed.

## Deep Sump Hooded Catch Basins:

Inspect after every major storm event (2" or greater) for the first few months after construction and at least twice per year thereafter. Inspect for clogged grates or pipes and excessive accumulation of sediment and trash. Remove accumulation of leaves or debris over grate inlets as needed throughout the year. Clean sumps when sediment reaches 24 ".

## Sub-surface Infiltration System:

Inspect after every major storm event (2" or greater) for the first few months after construction to ensure proper stabilization and function. Thereafter, inspect at least twice per year during wet weather to ensure the system is draining properly. Check for accumulation of sediment and ponding water. If ponding water is visible inside the system for several days after a storm event, notify the engineer for possible remedial measures. Remove sediment as necessary during construction, while the system is dry, and at least every five years after construction.

## Grassed swales:

Inspect after every major storm event (2" or greater) for the first few months after construction and at least twice per year thereafter. Repair eroded spots immediately after inspection. Additional inspections should be scheduled during the first few months to ensure that the vegetation in the channels is established adequately. Accumulated sediment shall be removed at least once a year or before it exceeds 0.5 ' in depth, whichever occurs first. Swales shall be mowed as needed. Clippings to be removed from swales, areas immediately up-gradient and properlyp disposed of.

## Street Sweeping:

All paved areas should be swept two times per year, once during the late spring and once during the late fall seasons after construction.

## Stone Infiltration Trench:

Inspect after every major storm event (2" or greater) for the first few months after construction and at least twice per year thereafter during wet weather to ensure the system is working properly. Check for accumulation of sediment, debris, weed growth and leaf litter and clean out as required, including replacement of top layer of stone.

## Detention Basins:

Inspect after every major storm event ( 2 " or greater) for the first few months after construction to ensure proper stabilization and function. Thereafter inspect at least twice per year during wet
weather to ensure the system is draining properly. Examine the outlet structure or outlet pipes for evidence of clogging or excessive outlet velocities. Check for accumulation of sediment and ponding of water. If ponding water above the outlet pipes is visible inside the basin for several days after a storm event, notify the engineer for possible remedial measures. Mow the berm at least twice per year. Remove sediment while the system is dry, and at least every 5 years after construction.

## Outlet Pipes and Flared End Sections:

Inspect after every major storm event (2" or greater) for the first few months after construction to ensure proper stabilization and function. Thereater inspect twice a year for erosion, clogging, settling, and excessive accumulation of leaves, trash, debris or sediment and channelization of stormwater discharge.

## Rip-rap Aprons/Spillways/Level Spreaders:

Inspect after every major storm event ( 2 " or greater) for the first few months after construction to ensure proper stabilization and function. Thereafter inspect twice per year at a minimum, for erosion, excessive accumulation of sediment, signs of failure, excessive weed/vegetation growth, and trash. Repair eroded spots immediately after inspection. Accumulated sediment shall be removed at least once a year or before it exceeds 0.5 ft . in depth, whichever occurs first.

## C. ESTIMATED ANNUAL BUDGET

The estimated annual budget for the activities required in this Post-Construction Long Term Stormwater Operation and Maintenance Plan is $\$ 2,000.00$.
D. SAMPLE OPERATION AND MAINTENANCE LOG (Next Page)

## SAMPLE OPERATION AND MAINTENANCE LOG

UPPER UNION SOLAR PROJECT - FRANKLIN, MASSACHUSETTS
POST-CONSTRUCTION
LONG TERM STORMWATER OPERATION \& MAINTENANCE PLAN

| Date: | Personnel Present: |
| :---: | :---: |
| Inspectors Name: |  |
| Inspectors Contact Information: |  |
|  |  |
| O\&M ITEM: | COMMENTS, CORRECTIVE ACTION NEEDED, AND NOTES: |
| Erosion Control Barriers |  |
| Deep Sump Hooded Catch Basins |  |
| Sub-Surface Infiltration System |  |
| Grassed Swales |  |
| Street Sweeping |  |
| Stone Infiltration Trench |  |
| Detention Basins |  |
| Outlet Pipes and Flared End Sections |  |
| Rip-rap Aprons/Spillways/Level Spreaders |  |


[^0]:    ${ }^{1}$ The Stormwater Report may also include the Illicit Discharge Compliance Statement required by Standard 10. If not included in the Stormwater Report, the Illicit Discharge Compliance Statement must be submitted prior to the discharge of stormwater runoff to the post-construction best management practices.
    ${ }^{2}$ For some complex projects, it may not be possible to include the Construction Period Erosion and Sedimentation Control Plan in the Stormwater Report. In that event, the issuing authority has the discretion to issue an Order of Conditions that approves the project and includes a condition requiring the proponent to submit the Construction Period Erosion and Sedimentation Control Plan before commencing any land disturbance activity on the site.

[^1]:    ${ }^{1} 80 \%$ TSS removal is required prior to discharge to infiltration BMP if Dynamic Field method is used.

